



FUNCTIONAL SERVICING REPORT

PROPOSED TOWNHOUSE DEVELOPMENT

**2463-2464 MIMOSA ROW
FOXMAR LTD.**

**CITY OF MISSISSAUGA
REGIONAL MUNICIPALITY OF PEEL**

FILE NO. 220-M130

APRIL 05, 2022



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1.0 INTRODUCTION

Skira & Associates Ltd. has been retained by Foxmar Ltd. to investigate and prepare a Functional Servicing Report (FSR) in support of an Official Plan Amendment and Zoning By-law Amendment for a proposed residential townhouse development at 2463-2462 Mimosa Row, in the City of Mississauga, Region of Peel.

The purpose of this report is to define the existing municipal services to the subject parcel of land and the proposed servicing details in support of the proposed residential townhouse development.

It is intended this FSR will result in ‘approval in principal’ of the design proposal by the City of Mississauga, Regional Municipality of Peel and any other relevant authorities. Detailed design will be provided during the Site Plan Application process.

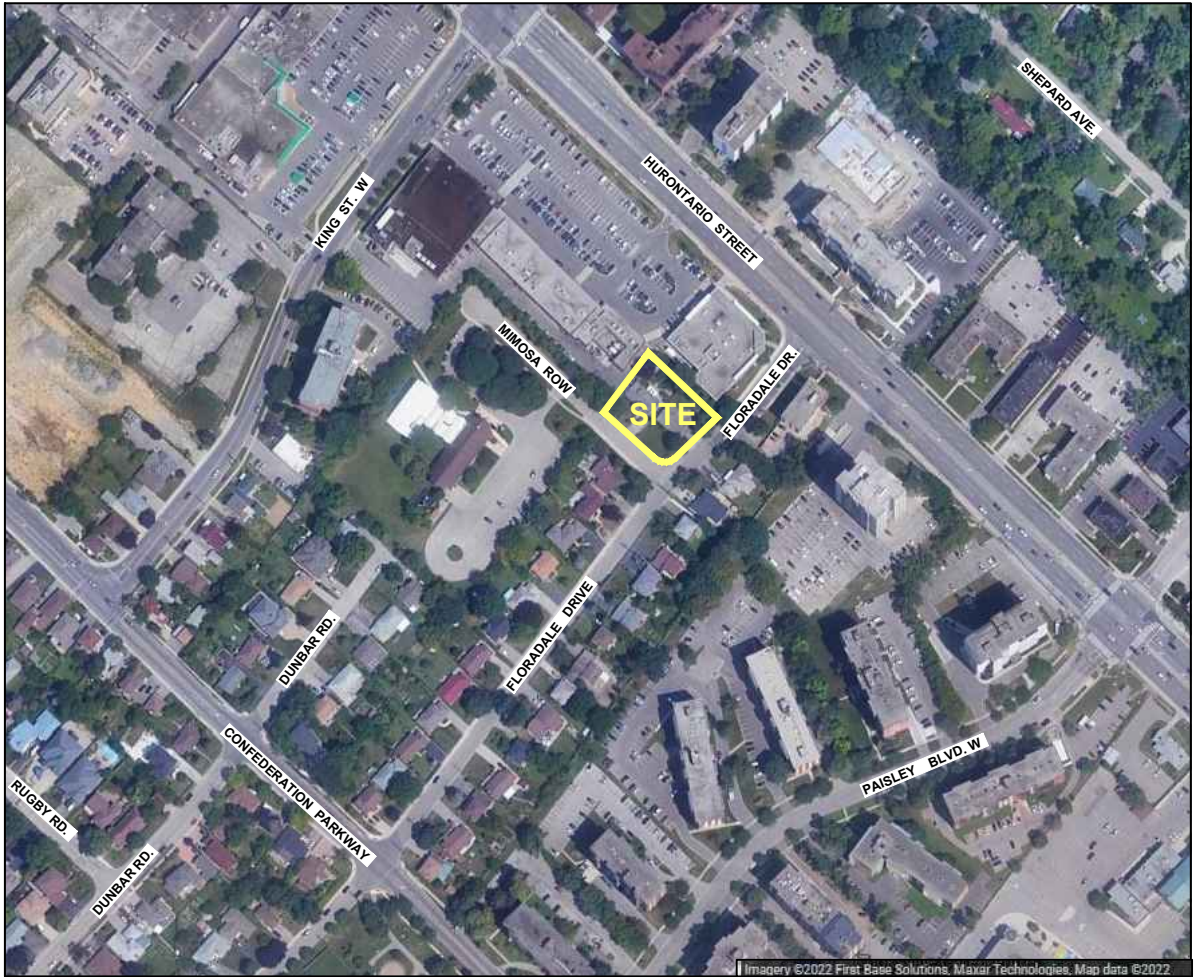
2.0 SITE AREA INFORMATION

The subject site is part of Lots 1 and 2, Registered Plan 500, City of Mississauga, Regional Municipality of Peel, and covers an area of approximately 0.14 Ha.

The subject site is bounded by Mimosa Row to the west, Floradale Drive to the south, and commercial properties to the north and east. Refer to *Figure 1 Key Plan*.

Currently, the site is comprised of two existing single-family residential homes fronting Mimosa Row. The existing buildings will be demolished prior to the start of construction.

The proposed residential development will consist of six (6) 3-storey townhouse units with basement.



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KEY PLAN

PROJECT No. 220-M130

DATE - MARCH 2022

SCALE - N.T.S.

DRAWN BY - D.M.

**FIGURE
No. 1**

3.0 SITE ACCESS

The site is in a good location to be serviced by existing major arterial roads, Hurontario Street, Confederation Parkway and Queensway West.

Currently, the existing house at 2469 has a driveway off Mimosa Row, while the existing house at 2463 has a driveway off Floradale Drive. The existing driveways and curb depressions will be removed. The boulevard will be reinstated with topsoil and sod to the satisfaction of the city, and curb depressions replaced with concrete curb and gutter as per OPSD 600.040.

Each unit of the proposed townhouse block will be provided with a 3.0m wide driveway off Mimosa Row. Refer to *Dwg. 220-M130-2 Site Grading Plan*.

4.0 WATER DISTRIBUTION SYSTEM

According to available records, there is an existing 150mm diameter watermain on Mimosa Row and an existing 200mm diameter watermain on Floradale Drive.

Currently, the existing house at 2469 has an existing 20mm water service to the existing 150mm diameter watermain on Mimosa Row. The existing house at 2463 has an existing 20mm water service to the existing 200mm diameter watermain on Floradale Drive. The water services will be disconnected as per Region of Peel standards.

The proposed townhouse units will be provided with new individual 25mm water service connections to the existing 150mm watermain on Mimosa Row.

The existing watermain will provide sufficient water supply to service the residential homes. The existing hydrant on Mimosa Row will provide fire coverage for the townhouse block. Refer to *Dwg. 220-M130-1 Site Servicing Plan*.

Water Demand Calculations

The estimated domestic water demand from the development was calculated as follows:

$$\text{Proposed population} - 6 \times 3.5 \text{ (townhouse)} = 21$$

$$\begin{aligned} \text{Site Average Flow} &= 280 \text{ Litres/capita/day} \\ &= 280 \times 21 \\ &= 5,880 \text{ L/day} \qquad = 0.068 \text{ L/s} \end{aligned}$$

$$\begin{aligned} \text{Total Expected Peak Flow Rate} &= \text{Site Average Flow} \times \text{Peak Hour Factor} \\ &= 5,880 \times 3.0 \\ &= 17,640 \text{ L/day} \qquad = 0.204 \text{ L/s} \end{aligned}$$

$$\begin{aligned} \text{Total Expected Maximum Daily Flow} &= \text{Site Average Flow} \times \text{Maximum Day Factor} \\ &= 5,880 \times 2.0 \\ &= 11,760 \text{ L/day} \qquad = 0.136 \text{ L/s} \end{aligned}$$

Based on Fire Underwriter Survey 1999, the fire flow is calculated on the total floor area:

$$F = 220 C\sqrt{A}$$

Where, C = coefficient of ordinary construction, 1.0
A = total floor area (including all storeys, but excluding basements), 1,796m²
F = fire flow in L/min

$$\begin{aligned} F &= 220 \times 1.0 \times \sqrt{1,796} \\ &= 9,323 \text{ L/min} \qquad \approx 9,000 \text{ L/min} \\ &= \mathbf{155.4 \text{ L/s}} \end{aligned}$$

A decrease can be applied for occupancy having a low contents fire hazard:

$$F = 9,000 \text{ L/min} - 25\% = 6,750 \text{ L/min}$$

The neighbouring properties have a 6.0m – 30.0m separation. A charge of 60% is applied for the exposures:

$$F = 6,750 \text{ L/min} \times 60\% = 4,050 \text{ L/min}$$

Therefore:

$$\begin{aligned} F &= 6,750 + 4,050 \\ &= 10,800 \text{ L/min} \quad \approx 11,000 \text{ L/min} = \mathbf{183.33 \text{ L/s}} \end{aligned}$$

$$\begin{aligned} \mathbf{\text{Maximum Peak Flow}} &= 0.204 \text{ (Res.)} + 183.33 \text{ (Fire)} \\ &= \mathbf{183.5 \text{ L/s}} \end{aligned}$$

$$\mathbf{\text{Maximum Daily Flow}} = \mathbf{0.136 \text{ L/s}}$$

5.0 SANITARY DRAINAGE SYSTEM

According to available records, there is an existing 250mm diameter sanitary sewer on Mimosa Row connecting to an existing 300mm diameter sanitary sewer on Floradale Drive.

Currently, the existing house at 2469 has an existing 125mm sanitary connection to the existing 250mm diameter sanitary sewer on Mimosa Row. The existing house at 2463 has an existing 125mm sanitary connection to the existing 300mm diameter sanitary sewer on Floradale Drive. The sanitary connections will be disconnected as per Region of Peel standards.

The proposed townhouse units will be provided with new 125mm sanitary connections to the existing 250mm diameter sanitary sewer on Mimosa Row.

The proposed tentative basement floor elevation is approximately 105.80m. The proposed sanitary connection invert elevations are approximately 104.45m – 104.70m at the property line. Based on these inverts and the tentative finished floor elevation, each home will have sufficient depth to be serviced by gravity flow. Refer to *Dwg. 220-M130-1 Site Servicing Plan*.

Refer to *Figure 2 Sanitary Drainage Plan* and *Appendix A* for the Sanitary Sewer Design Chart.

Sanitary Flow Calculations

The average flow from the development to the 250mm sanitary sewer on Mimosa Row:

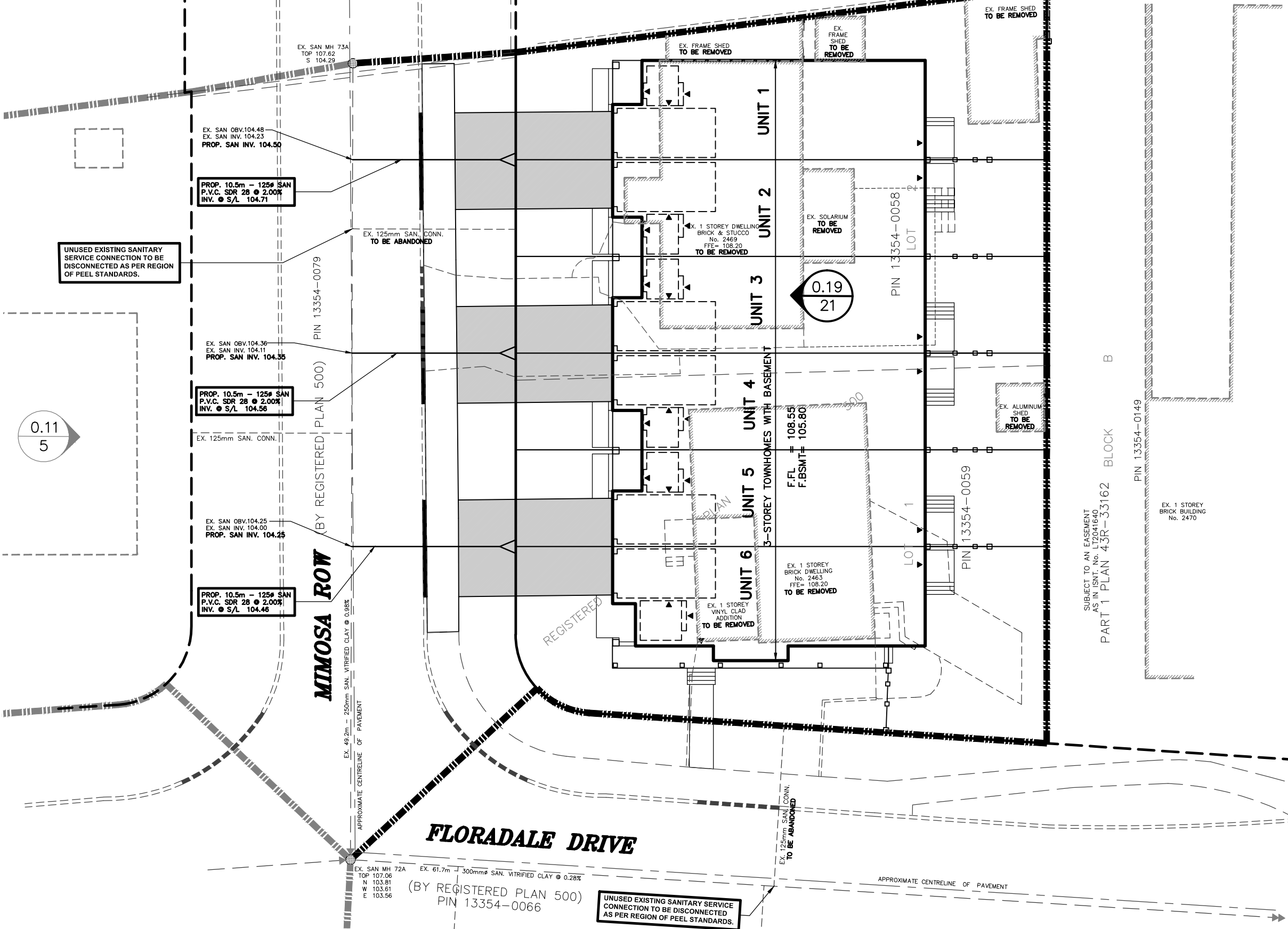
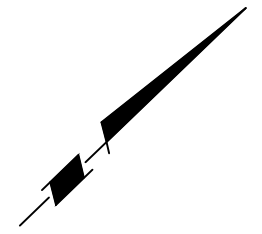
Proposed population – 6 x 3.5 (townhouse) = 21 persons

$$\begin{aligned}\text{Average Daily Flow} &= 302.8 \text{ L/cap/day} \times 21 \\ &= 6,358.8 \text{ L/day} \\ &= 0.074 \text{ L/s}\end{aligned}$$

$$\begin{aligned}\text{Peak Factor} &= 1 + \frac{14}{4 + P^{0.5}} && \text{Where, P = population in thousands} \\ &= 1 + \frac{14}{4 + 0.021^{0.5}} \\ &= 1 + 3.38 \\ &= 4.38 && \text{Maximum Peak Factor is 4.0.}\end{aligned}$$

$$\begin{aligned}\text{Peak Flow Rate} &= \text{Average Daily Flow} \times \text{Peak Factor} \\ &= 6,358.8 \times 4.0 \\ &= 25,435.2 \text{ L/day} \\ &= \mathbf{0.294 \text{ L/s}}\end{aligned}$$

LOT 16 CONCESSION 1 SOUTH OF DUNDAS STREET
 EX. ASPHALT
 PIN 13354-0030



UNUSED EXISTING SANITARY SERVICE CONNECTION TO BE DISCONNECTED AS PER REGION OF PEEL STANDARDS.

0.11
5

PROP. 10.5m - 125# SAN P.V.C. SDR 28 @ 2.00% INV. @ S/L 104.71

PROP. 10.5m - 125# SAN P.V.C. SDR 28 @ 2.00% INV. @ S/L 104.56

PROP. 10.5m - 125# SAN P.V.C. SDR 28 @ 2.00% INV. @ S/L 104.46

MIMOSA ROW
 (BY REGISTERED PLAN 500)

FLORADALE DRIVE

(BY REGISTERED PLAN 500)
 PIN 13354-0066

UNUSED EXISTING SANITARY SERVICE CONNECTION TO BE DISCONNECTED AS PER REGION OF PEEL STANDARDS.

LEGEND

- EXISTING SANITARY SEWER
- PROPOSED SANITARY CONNECTION
- EXISTING SANITARY MANHOLE
- PROPOSED DRAINAGE AREA BOUNDARY
- EXISTING DRAINAGE AREA BOUNDARY
- AREA (HECTARES)
 3.19 / 12 - POPULATION
- LIMIT OF PROPERTY

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SANITARY DRAINAGE

PROJECT No. 220-M130

DATE - MARCH 2022

SCALE - 1 : 250 DRAWN BY - D.M.

**FIGURE
 No. 2**

6.0 STORM DRAINAGE SYSTEM

According to available records, there is an existing 675mm diameter storm sewer on Mimosa Row connecting to an existing 900mm diameter storm sewer on Floradale Drive.

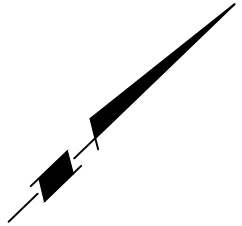
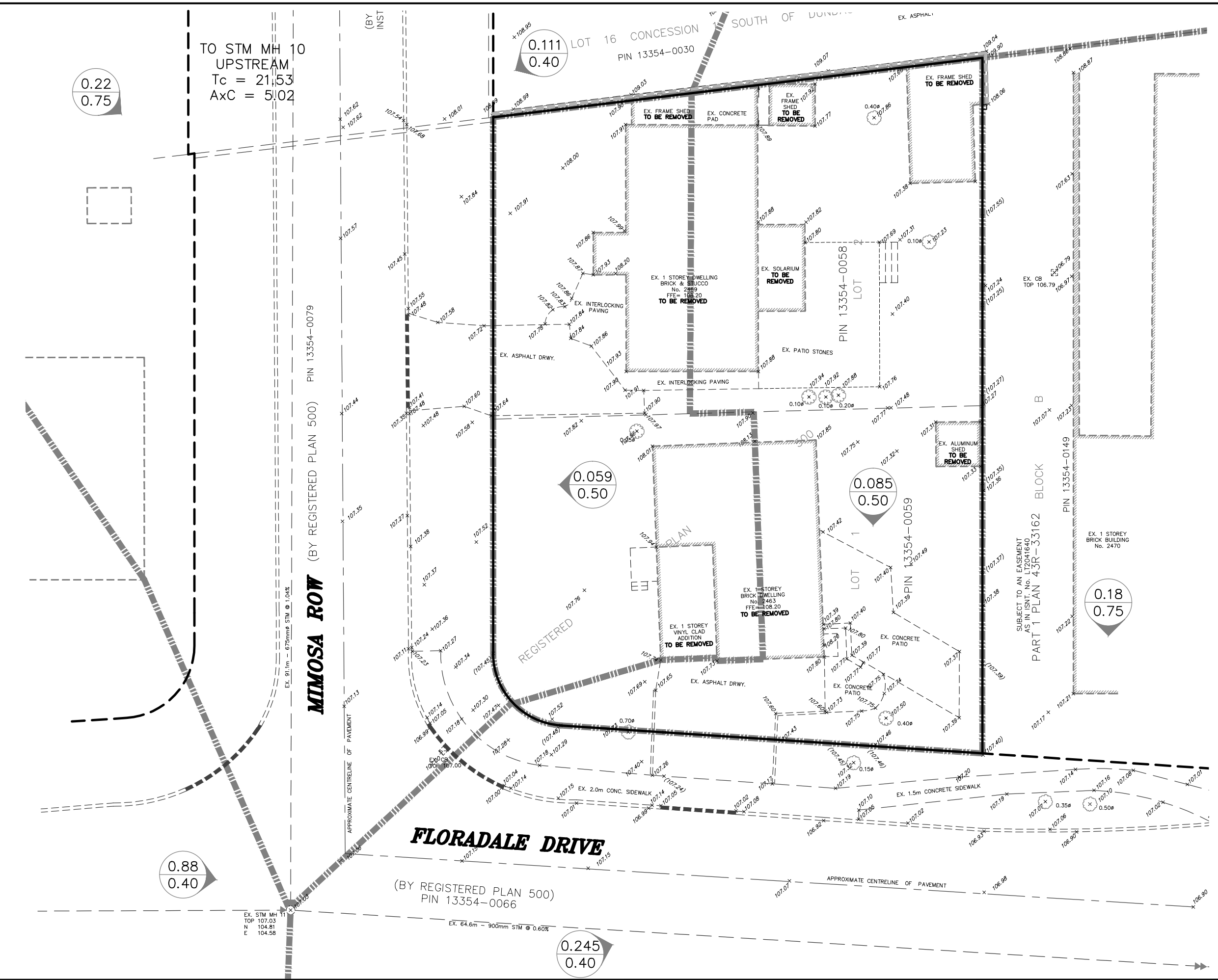
Currently, an area of 0.059Ha from the site drains towards Mimosa Row and an area of 0.085Ha drains towards Floradale Drive. Refer to ***Figure 3 Existing Storm Drainage Plan.***

For the proposed development, roof downspouts will discharge onto surface via splash pad and directed towards the proposed side and back yard swales.

A rear yard catchbasin and a 250mm diameter storm sewer will be provided to capture drainage and will be connected to the existing 900mm diameter storm sewer on Floradale Drive. A 3.0m municipal storm sewer easement will be required for access and maintenance of the catchbasin and storm sewer.

Each unit will be provided with a sump pump to pump basement weeping tiles to surface. Basements will be constructed a minimum 1.0m above the groundwater level.

Refer to ***Figure 4 Proposed Storm Drainage Plan*** and ***Appendix B*** for the existing storm sewer drainage record and Storm Sewer Design Chart.



- ### LEGEND
- > - EXISTING STORM SEWER
 - - EXISTING STORM MANHOLE
 - - - - - EXISTING DRAINAGE AREA BOUNDARY
 - 0.36 - AREA (HECTARES)
 - 0.70 - RUN-OFF COEFFICIENT
 - LIMIT OF PROPERTY

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EXISTING STORM DRAINAGE PLAN

PROJECT No. 220-M130		FIGURE No. 3
DATE - MARCH 2022		
SCALE - 1 : 250	DRAWN BY - D.M.	

TO STM MH 10
 UPSTREAM
 $T_c = 21.53$
 $AxC = 5.02$

0.22
 0.75

0.111
 0.40

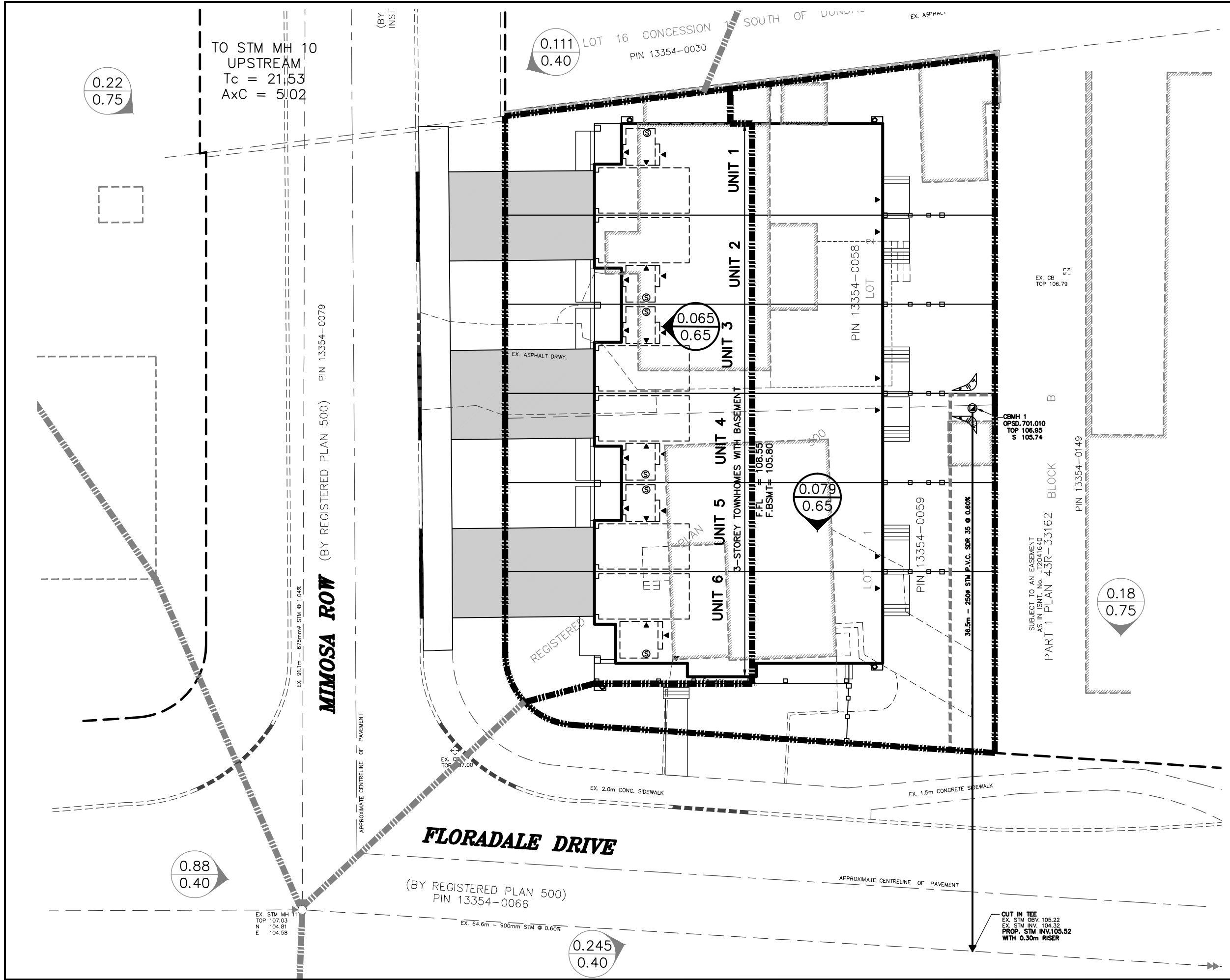
0.059
 0.50

0.085
 0.50

0.18
 0.75

0.88
 0.40

0.245
 0.40



- LEGEND**
- - - - - EXISTING STORM SEWER
 - PROPOSED STORM CONNECTION
 - EXISTING STORM MANHOLE
 - PROPOSED STORM MANHOLE
 - ▬▬▬▬▬ PROPOSED DRAINAGE AREA BOUNDARY
 - ▬▬▬▬▬ EXISTING DRAINAGE AREA BOUNDARY
 - 0.36 - AREA (HECTARES)
 - 0.70 - RUN-OFF COEFFICIENT
 - ▬▬▬▬▬ LIMIT OF PROPERTY

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PROPOSED STORM DRAINAGE PLAN

PROJECT No. 220-M130
 DATE - MARCH 2022
 SCALE - 1 : 250 DRAWN BY - D.M.

FIGURE No. 4

CUT IN TEE
 EX. STM. INV. 105.22
 EX. STM. INV. 104.32
 PROP. STM. INV. 105.52
 WITH 0.30m RISER

TO STM MH 10
 UPSTREAM
 Tc = 21.53
 AxC = 5.02

MIMOSA ROW (BY REGISTERED PLAN 500) PIN 13354-0079

FLORADALE DRIVE (BY REGISTERED PLAN 500) PIN 13354-0066

3-STORY TOWNHOMES WITH BASEMENT

SUBJECT TO AN EASEMENT AS IN ISNT. No. L22041640 PART 1 PLAN 43R-33162 BLOCK B

0.111
0.40

0.065
0.65

0.079
0.65

0.18
0.75

0.88
0.40

0.245
0.40

EX. STM. MH 11
 TOP 107.03
 N 104.81
 E 104.58

EX. 64.6m - 900mm STM @ 0.60%

36.5m - 250# STM P.V.C. SDR 35 @ 0.60%

F.F.L. = 108.55
 F.B.S.M.T. = 105.80

EX. CB TOP 106.79

PIN 13354-0149

EX. 1.5m CONCRETE SIDEWALK

EX. 2.0m CONC. SIDEWALK

EX. 91.1m - 675mm STM @ 1.04%

APPROXIMATE CENTRELINE OF PAVEMENT

APPROXIMATE CENTRELINE OF PAVEMENT

REGISTERED

(BY INST)

EX. ASPHALT DRWY.

SOUTH OF DUNDAS

EX. ASPHALT

LOT 16 CONCESSION
 PIN 13354-0030

PIN 13354-0058

PIN 13354-0059

EX. CB TOP 106.79

B

PIN 13354-0149

PIN 13354-0149

PIN 13354-0149

7.0 SUMMARY

Our findings reveal the proposed residential townhouse development of six (6) 3-storey townhouse units with basement can be fully serviced to the existing available services on Mimosa Row and Floradale Drive. The findings of this report are global and are related to the servicing functionality of this application. These findings by no means are final and are not to replace the detailed review of this application.

The conclusion is as follows:

- Each unit will be provided with individual driveways to Mimosa Row.
- Each unit will be serviced by individual proposed **25mm diameter** water service connections to the existing 150mm diameter watermain on Mimosa Row.
- Each unit will be serviced by individual proposed **125mm diameter** sanitary connections to the existing 250mm diameter sanitary sewer on Mimosa Row.
- A **250mm diameter** storm sewer will be provided to drain the proposed side yard and backyard swales and connected to the existing 900mm diameter storm sewer on Floradale Drive. A 3.0m municipal storm sewer easement will be required for access and maintenance of the catchbasin and storm sewer.
- Roof downspouts will discharge onto surface via splash pad and directed towards the proposed side and back yard swales.
- Basement weeping tiles for each unit will be provided with sump pumps and pumped to surface.

We respectively submit this report with the intention of obtaining approval in principal of the recommendations herein, and trust the information provided meets with the requirements. The report's recommendations will be implemented in detail design during the site plan and building permit process.

Yours truly,

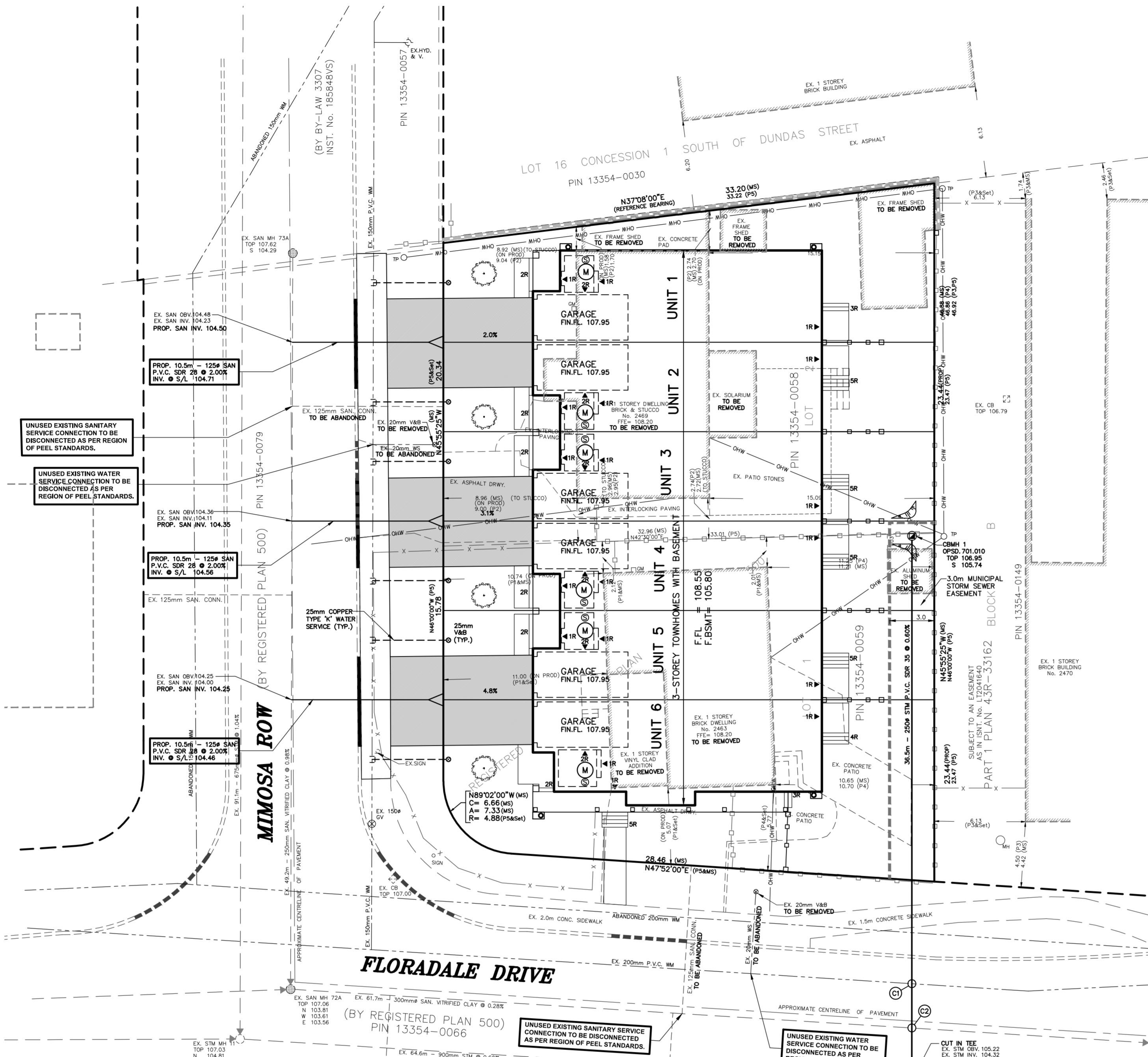
SKIRA & ASSOCIATES LTD.

Roman T. Kerkuszk, P. Eng.

NOTE: **Limitation of Report**

This report was prepared by Skira & Associates Ltd. for Foxmar Ltd. for review and approval by government agencies only.

In light of the information available at the time of preparation of this report, any use by a Third Party of this report are solely the responsibility of such Third Party and Skira & Associates Ltd. accepts no responsibility for any damages, if any, suffered by the Third Party



- ### STORM SEWERS
1. ALL STORM SEWER MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO CURRENT MUNICIPAL STANDARDS AND SPECIFICATIONS.
 2. RECORDING TO BE TYPE W AS PER C.M. STD. 212.08 UNLESS OTHERWISE NOTED.
 3. SEWER BEDDING AND COVER MATERIAL SHALL CONFORM TO C.M. STD. 212.111 AND 212.190 UNLESS OTHERWISE NOTED.
 4. IF WATER IS PRESENT IN THE TRENCH EXCAVATION, THEN 19mm CLEAR STONE OR 6mm WASHED CRUSHED GRAVEL IS TO BE USED FOR BEDDING IN ACCORDANCE WITH C.M. STD. 212.119 AND 212.148 RESPECTIVELY. GEOTECHNICAL ASSESSMENT MAY BE REQUIRED TO DETERMINE APPROPRIATE BEDDING IN ORDER TO STABILIZE THE SUBGRADE FOR SEWER CONSTRUCTION.
 5. STORM SEWERS AND CONNECTIONS 150mm AND SMALLER TO BE CONCRETE CL 3 OR PVC SDR 26 PIPE UNLESS OTHERWISE NOTED.
 6. STORM SEWERS AND CONNECTIONS 200mm AND LARGER TO BE CONCRETE CL 3, CONCRETE CL 6.0, PVC SDR 35, WITH TYPE B BEDDING THROUGHOUT EXCEPT AT RISERS UNLESS OTHERWISE NOTED.
 7. ALL MANHOLES OR CATCHBASIN MANHOLES TO BE SUMPLESS AS PER OPSD. 701.010 UNLESS OTHERWISE NOTED.
 8. CATCHBASIN LEADS TO BE SINGLE 300mm, DOUBLE 300mm UNLESS OTHERWISE NOTED.

- ### SANITARY SEWERS
1. ALL SANITARY SEWER MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO CURRENT REGION OF PEEL STD. 4.8.16C.
 2. SANITARY CONNECTIONS 150mm AND LESS TO BE PVC SDR 26.
 3. SANITARY SEWERS AND CONNECTIONS 200mm AND LARGER TO BE PVC SDR 26 ASTM D3034 BY 11 WITH TYPE B BEDDING THROUGHOUT EXCEPT AT RISERS UNLESS OTHERWISE NOTED.
 4. ALL MANHOLES TO BE R.P. STD 2-3.3 UNLESS OTHERWISE NOTED.

- ### CONNECTIONS
1. **SANITARY:**
 - A. SINGLE AND DOUBLE MIN. 125mm DIA PVC SDR 26.
 - B. CONNECTIONS TO SEWER TO BE MADE WITH MANUFACTURED TEE OR WYE WHERE APPLICABLE AND SHALL BE COLOUR CODED AS NON-WHITE OR AS PER C.M. STD. 212.095 & R.P. STD. 2-4.1 TO 2-4.4.
 - C. SANITARY SERVICE SHALL BE LOWER THAN AND TO THE RIGHT OF THE STORM SERVICE AT THE PROPERTY LINE WHEN PASSING THE LOT FROM THE STREET.
 - D. SERVICE CONNECTION TO LOT LINE SHALL BE VISIBLY MARKED BY A 1.8m x 100mm x 100mm WOOD STAKE BURIED 100mm AND PAINTED RED.
 2. **STORM:**
 - A. SINGLE AND DOUBLE MIN. 150mm DIA CONC. CLASS 1 OR PVC SDR 35.
 - B. CONNECTIONS TO SEWER 450mm DIA AND LESS TO BE MADE WITH MANUFACTURED TEE OR WYE WHERE APPLICABLE AND SHALL BE COLOUR CODED AS WHITE OR AS PER LOCAL STANDARDS.
 - C. STORM SERVICE SHALL BE ON THE LEFT SIDE OF THE SANITARY CONNECTION WHEN PASSING THE LOT FROM THE STREET.
 - D. SERVICE CONNECTION TO LOT LINE SHALL BE VISIBLY MARKED BY A 1.8m x 100mm x 100mm WOOD STAKE BURIED 100mm AND PAINTED GREEN AND/OR WHITE.
 - E. ALL 'BOOT JACKS' AND 'Y'S' ARE TO BE CAST IRON FOR STORM HOUSE CONNECTIONS.
 3. **WATER:**
 - A. SERVICE CONNECTIONS TO BE 25mm DIA TYPE 'K' SOFT COPPER TUBING UNLESS OTHERWISE NOTED AND AS PER R.P. STD. 4-7.1 & C.M. STD. 218.194 TO 218.198.
 - B. SERVICE CONNECTION TO BE VISIBLY MARKED BY 1.8m x 100mm x 100mm WOOD STAKE BURIED 100mm AND PAINTED BLUE.

- ### GENERAL NOTES
1. THE CONTRACTOR IS TO CHECK AND VERIFY ALL DIMENSIONS. IF ANY DISCREPANCIES, THEY MUST BE REPORTED TO THE ENGINEER IMMEDIATELY PRIOR TO CONSTRUCTION.
 2. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES DURING CONSTRUCTION. GAS, HYDRO, TELEPHONE OR ANY OTHER UTILITIES THAT MAY EXIST ON THE SITE OR WITHIN THE STREETLINES MUST BE LOCATED BY ITS OWN UTILITIES AND VERIFIED PRIOR TO CONSTRUCTION.
 3. ALL CONNECTIONS SHALL BE INSTALLED AS PER MUNICIPAL STANDARDS AND SPECIFICATIONS.
 4. BUILDERS TO REFER TO THE ENGINEER THAT THE FINAL FOOTING ELEVATION AND TOP OF FOUNDATION WALL ELEVATION ARE IN CONFORMANCE WITH THE BUILDING CODE AND THE CERTIFIED GRADING PLAN PRIOR TO PROCEEDING.
 5. THE ELEVATION OF THE SEE SWALE AT THE BUILDING LINE SHALL BE A MINIMUM OF 150mm BELOW THE BUILDING LINE AT THE CENTRE OF THE SWALE.
 6. OUTSIDE FINISHED GRADE TO BE A MINIMUM OF 150mm BELOW BRICK VENEER ELEVATION.
 7. PRIOR TO ANY SOODING, THE BUILDER IS TO ENSURE TO THE SOIL CONSTANT AND/OR THE ENGINEER THAT THE LOT TO BE SEWER GRAZED AND TOPSOIL TO BE COMPLETED WITH A MINIMUM DEPTH OF 100mm OF TOPSOIL AND NO 4 NURSERY SOIL AND A MINIMUM DEPTH OF 150mm OF CRUSHED STONE TO BE PROVIDED ON THE ENTIRE LENGTH OF EACH DRIVEWAY OR DRIVEWAY AND DRIVEWAY TO BE PAVED WITH A MINIMUM COMPACTED DEPTH OF 75mm OF ASPHALT BETWEEN THE CURB AND THE GARAGE.
 8. NO SOODING ON ANY LOTS IS PERMITTED UNTIL PRELIMINARY INSPECTIONS DONE BY THE ENGINEER AND THE BUILDER.
 9. ALL ENTRANCES TO THE SITE, ROAD CURBS AND SIDEWALKS WILL BE CONTINUOUS THROUGH THE DRIVEWAY. THE DRIVEWAY GRADE WILL BE COMPATIBLE WITH THE EXISTING OR FUTURE SIDEWALK AND CURB DEPRESSION SHALL BE PROVIDED FOR EACH ENTRANCE.
 10. DRIVEWAY GRADES SHOULD NOT BE LESS THAN 2% AND NOT GREATER THAN 8%.
 11. LAWN AND SWALES SHALL HAVE A MINIMUM SLOPE OF 1.5% (PREFERRED 2%) AND A MAXIMUM SLOPE OF 8%.
 12. WHERE GRADES IN EXCESS OF 8% ARE REQUIRED, THE MAXIMUM SLOPE SHALL BE 3% GRADE CHANGES IN EXCESS OF 1.0m ARE TO BE ACCOMPLISHED BY USE OF A RETAINING WALL. RETAINING WALLS HIGHER THAN 0.6m SHALL HAVE A FENCE NOTICABLE ON THE INSIDE.
 13. THE SERVICE CONNECTION TRENCH WITHIN THE TRAVELLED PORTION OF THE ROAD ALLOWANCE SHALL BE BACKFILLED WITH IMPROVABLE SANDFILL MATERIAL AS PER C.M. STD. 228.019 AND 228.022 UNLESS OTHERWISE SPECIFIED PRIOR APPROVAL FOR OTHER SANDFILL MATERIAL HAS BEEN OBTAINED.
 14. ALL WATERMAIN AND WATER SERVICE MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO CURRENT MUNICIPAL STANDARDS AND SPECIFICATIONS.
 15. WATERMAIN AND WATER SERVICE ARE TO HAVE A MIN. DEPTH OF 1.7m WITH A MIN. HORIZONTAL SPACING OF 1.2m FROM THEMSELVES AND OTHER UTILITIES.
 16. SEDIMENT CONTROL FENCE TO BE INSTALLED AS PER C.M. STD. 204.010.
 17. ALL DAMAGED AND DISTURBED AREAS TO BE RENOVATED WITH TOPSOIL AND SOO.

- ### FIRE DEPARTMENT
1. FIRE ROUTE WILL BE DESIGNATED AS PER CITY OF MISSISSAUGA BY-LAW (108-01) AS AMENDED PRIOR TO OCCUPANCY OF THE BUILDING.
 2. FIRE ROUTES TO BE DESIGNED TO WITH STAND A LOAD NOT LESS THAN 11.3kN/m² PER AXLE AND HAVE A CHANGE IN GRADIENT OF NOT MORE THAN 1% IN 3.0m OVER A DISTANCE 15.0m AS PER BY-LAW 108-01.
 3. ALL 90° TURNING RADIUSES SHALL BE CLEARANCE OF 3.0m FROM THE CENTRE LINE OF TURNING RADIUS AND ANY CURBS OR PART OF BUILDING.
 4. PRIVATE FIRE HYDRANTS SHALL BE LOW TESTED AND COLOUR CODED IN CONFORMANCE WITH THE REGION OF PEEL 'UNIFORM MARKING OF HYDRANTS'.

- ### WATERMANS
1. ALL MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO THE CURRENT PEEL PUBLIC WORKS STANDARDS AND SPECIFICATIONS.
 2. WATERMAIN AND/OR WATER SERVICE MATERIALS 150mm AND LARGER MUST BE V.C. OR 18 TO 18MM SPEC C900-H, SDR 300a AND SMALLER MUST BE SOFT COPPER TYPE 'K' SPEC ASTM A818-04.
 3. WATERMAIN AND/OR WATER SERVICES ARE TO HAVE A MIN. DEPTH OF 1.7m WITH A MIN. HORIZONTAL SPACING OF 1.2m FROM THEMSELVES AND OTHER UTILITIES.
 4. PROVISIONS FOR FLOODING WATER LINE PRIOR TO TESTING, ETC. MUST BE PROVIDED WITH AT LEAST A 50mm OUTLET OR 100mm AND LARGER TO ALLOW THE WATER TO DRAIN OFF TO A PARKING LOT OR DOWN A DRAIN. ON FIRE LINES, FLOODING OUTLET TO BE 100mm MINIMUM ON HYDRANT.
 5. ALL CURB STOPS TO BE 3.0m OFF THE FACE OF THE BUILDING UNLESS OTHERWISE NOTED.
 6. HYDRANT AND VALVE SET TO R.P. STD. 1-4-1. DIMENSION A AND B 5.0m AND 5.0m AND TO HAVE PUMPER NOZZLE.
 7. WATERMAIN TO BE RETIRED TO GRADE AS SHOWN ON APPROVED SITE PLAN. COPY OF GRADE SHEET MUST BE SUPPLIED TO INSPECTOR PRIOR TO COMMENCEMENT OF WORK, WHERE REQUESTED BY INSPECTOR.
 8. WATERMAIN MUST HAVE A MIN. VERTICAL CLEARANCE OF 3.0m OVER OR 0.9m UNDER SEWERS AND ALL OTHER UTILITIES WHEN CROSSING.
 9. ALL PROPOSED WATER FIRING MUST BE ISOLATED FROM EXISTING LINES IN ORDER TO ALLOW INDEPENDENT PRESSURE TESTING AND CLEANING FROM EXISTING SYSTEMS.
 10. ALL LIVE TAPPING AND OPERATION OF REGION WATER VALVES SHALL BE ARRANGED THROUGH THE REGIONAL INSPECTOR (ASSISTED BY CONTRACTOR) OPERATIONS AND MAINTENANCE DIVISION.
 11. MECHANICAL RESTRAINTS MUST BE INSTALLED ON ALL BENDS, TEES AND REDUCERS.
 12. LOCATION OF ALL EXISTING UTILITIES IN THE FIELD TO BE ESTABLISHED BY THE CONTRACTOR.
 13. THE CONTRACTORS SHALL BE SOLELY RESPONSIBLE FOR LOCATES, EXPOSING, SUPPORTING AND PROTECTING OF ALL UNDERGROUND AND OVERHEAD UTILITIES AND STRUCTURES EXISTING AT THE TIME OF CONSTRUCTION IN THE AREA OF THE WORK, WHETHER SHOWN ON THE PLANS OR NOT AND FOR ALL REPAIRS AND CONSEQUENCES RESULTING FROM DAMAGE TO SAME.
 14. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE TO GIVE WRITTEN NOTICE TO THE UTILITIES PRIOR TO CROSSING SUCH UTILITIES FOR THE PURPOSE OF INSPECTION BY THE CONCERNED UTILITY. THIS INSPECTION WILL BE FOR THE DURATION OF THE CONSTRUCTION WITH THE CONTRACTOR RESPONSIBLE FOR ALL COSTS ARISING FROM SUCH INSPECTION.
 15. ALL PROPOSED WATER FIRING MUST BE ISOLATED THROUGH A TEMPORARY CONNECTION THAT SHALL INCLUDE AN APPROPRIATE CROSS-CONNECTION CONTROL DEVICE. CONSISTENT WITH THE REQUIRE OF HAZOP, FOR BACKFLOW PREVENTION OF THE ACTIVE DISTRIBUTION SYSTEM, CONFORMING TO R.P. STD. 1-5-7 AND 1-5-8.

C.M. BENCHMARK No. 337 ELEVATION: 108.73
 DESCRIPTION: ON THE SOUTH FACE, JUST EAST OF THE GARAGE DOOR OF A GREY AND YELLOW BRICK BUNGALOW NO. 111 AT THE NORTHEAST CORNER OF FLORADALE DRIVE AND CONFESSIONARY PARKWAY.

NOTE:
 WATER METERS ARE TO BE LOCATED IN THE BUILDINGS IN A HEATED AND EASY ACCESS LOCATION. (GROUND ACCESS)

WATER METER
 METER ROOM
 200mm METER
 R.P. STD. 1-4-7
 MIN. 0.2m
 MAX. 24.0m

CROSSINGS

C1	STM INV.	105.56
	WM OBV	105.20
C2	STM INV.	105.54
	SAN OBV	103.75

SITE DATA

ZONING	INDUSTRIAL
LOT AREA	10,117
BUILDING AREA	1,017
LANDSCAPED AREA	1,017
TOTAL PAVED AREA	1,017
PARKING REQUIRED	1,017
PARKING PROVIDED INCL. HDOP	1,017
LOADING SPACE REQUIRED	1,017
LOADING SPACE PROVIDED	1,017

FOR SERVICE CONNECTION WITH LESS THAN 0.50m CLEAR DISTANCE BETWEEN PIPES, PIPE INSULATION IS REQUIRED AS PER R.P. STD. 1-5-4.

SLOPE OF SANITARY LATERAL MUST BE BETWEEN 1% AND 2% AS PER REGION OF PEEL STANDARDS.

PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR MUST VERIFY THE EXISTING SANITARY, STORM SEWER AND WATERMAIN ELEVATION AND LOCATION AND ADVISE THE ENGINEER OF ANY DISCREPANCIES.

THE APPLICANT WILL BE REQUIRED TO CONTACT ALL UTILITY COMPANIES TO OBTAIN ALL REQUIRED LOCATES PRIOR TO THE INSTALLATION OF HOARDINGS WITHIN MUNICIPAL RIGHT-OF-WAY.

ALL ROOF DOWNSPOUTS FROM EAVESTROUGH ARE TO DISCHARGE ONTO THE SURFACE VIA CONCRETE SPLASH PADS AND THE RUNOFF DIRECTED TOWARDS THE REAR WHERE POSSIBLE AND TO THE ROAD.

NOTE:
 FOR ADDITIONAL INFORMATION, DETAILS, DIMENSIONS AND CONFORMITY TO THE SITE PLAN, THE CONTRACTOR MUST REFER TO THE ARCHITECTURAL SITE PLAN.

ALL INTERNAL EXISTING SERVICES AND APPURTENANCES NOT UTILIZED FOR SERVICING OF THIS PROJECT ARE TO BE REMOVED OFF SITE UNLESS OTHERWISE DIRECTED BY THE ENGINEER.

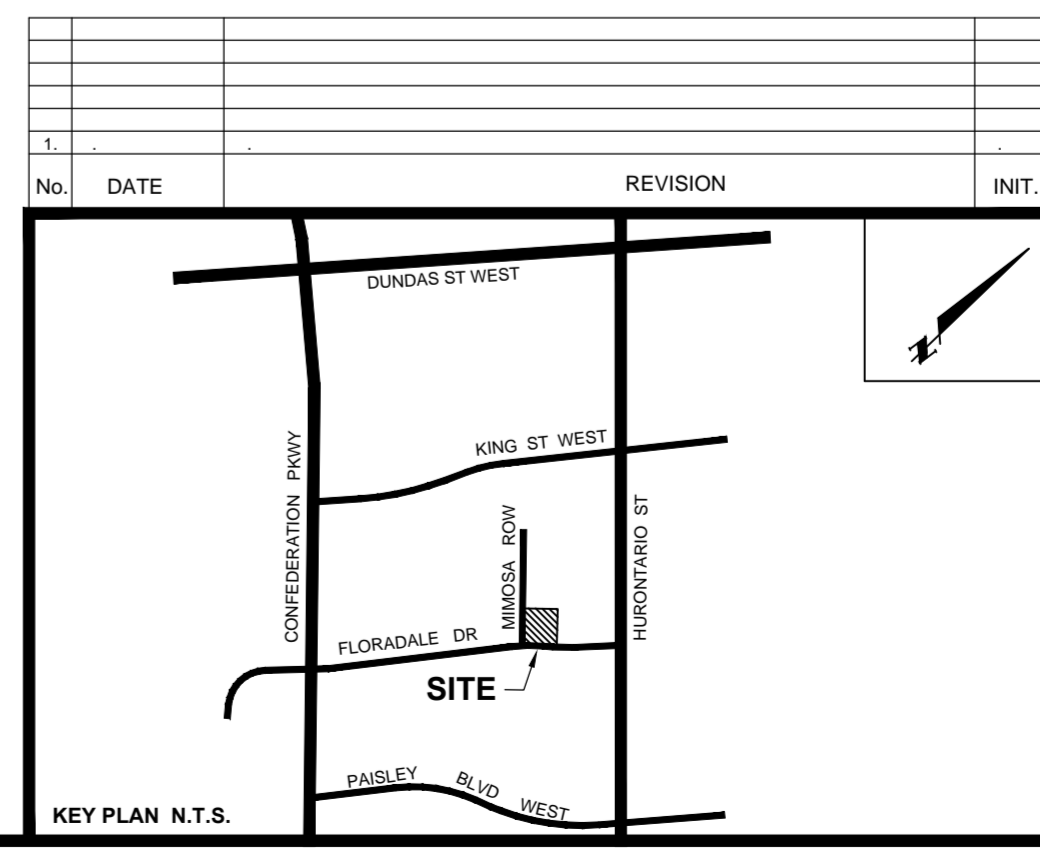
INFORMATION SHOWN HEREON REGARDING THE SIZE AND LOCATION OF EXISTING SERVICES AND/OR UTILITIES IS FURNISHED AS THE BEST AVAILABLE INFORMATION AND SHALL BE INTERPRETED AS THE CONTRACTOR SEES FIT WITH THE UNDERSTANDING THAT THE OWNER DISCLAIMS ALL RESPONSIBILITY FOR ITS SUFFICIENCY AND/OR ACCURACY.

WORKS IN THE MUNICIPAL RIGHT OF WAY BEING PERFORMED BY THE CITY'S CONTRACTOR WILL REQUIRE 4 TO 6 WEEKS NOTICE PRIOR TO COMMENCEMENT OF CONSTRUCTION AFTER ALL DRAWINGS HAVE BEEN APPROVED AND SECURITIES HAVE BEEN RECEIVED.

PRIOR TO ANY CONSTRUCTION, THE CONTRACTOR IS TO REFER TO THE TREE PRESERVATION PLAN PREPARED BY THE LANDSCAPE ARCHITECT.

PRIOR TO ANY CONSTRUCTION, THE BUILDER IS TO CONFIRM WITH ARCHITECT OR THE OWNER THAT THE PROPOSED FINISHED FLOOR AND TOP OF WALL ELEVATIONS ARE IN CONFORMANCE WITH THE ARCHITECTURAL DRAWINGS.

THE CONTRACTOR/BUILDER IS RESPONSIBLE FOR CONFIRMING ALL ABOVEGROUND AND UNDERGROUND UTILITY LOCATIONS AND IS TO IMMEDIATELY ADVISE THE ENGINEER OF ANY DISCREPANCIES.



SKIRA & ASSOCIATES LTD.
 CONSULTING ENGINEERS
 3464 Seminary Court, Suite 100, Mississauga, Ontario L5C 4P8
 Tel. (905) 276-5100 Fax. (905) 270-1536 Email - info@skiraconsult.ca

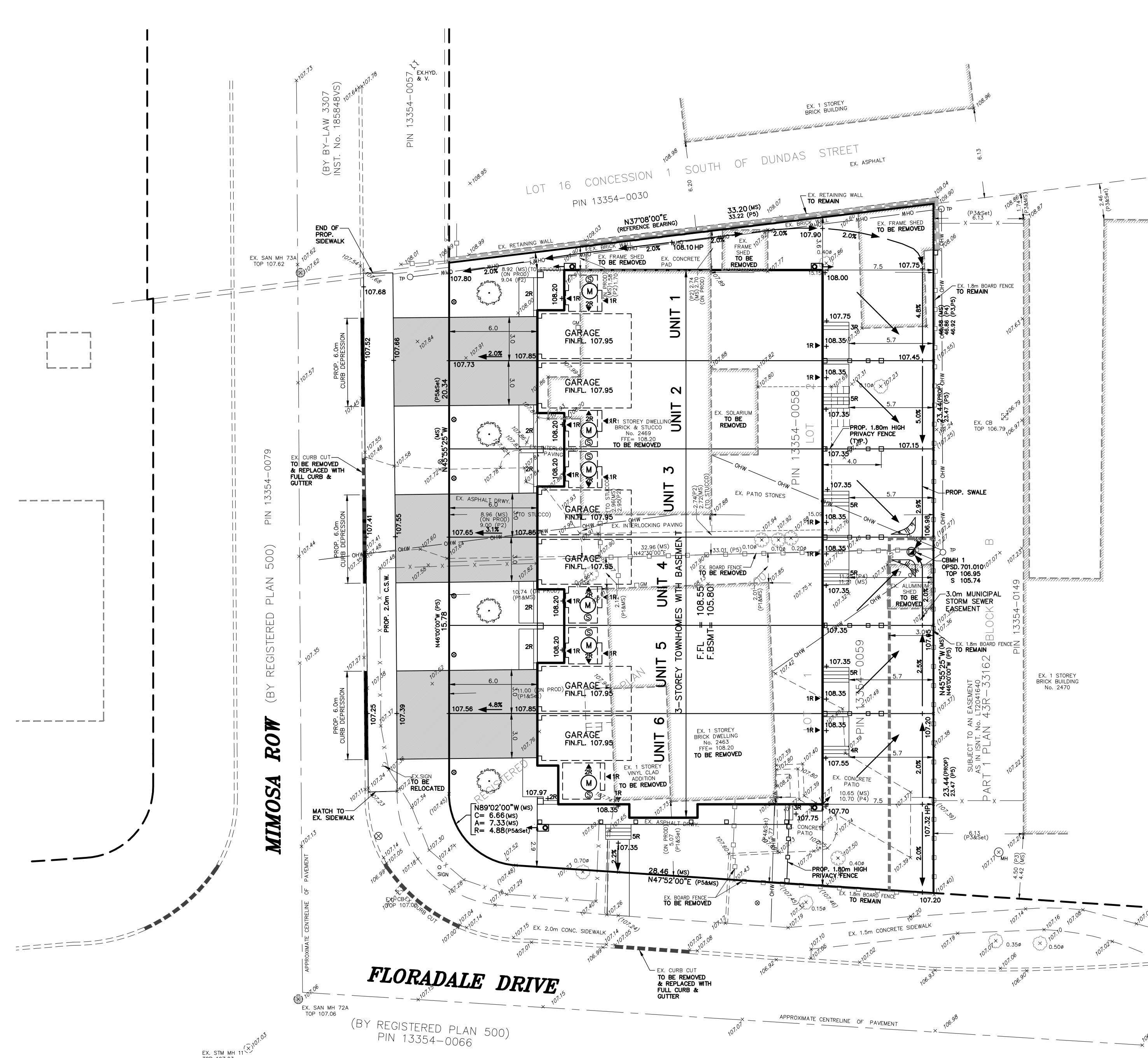
PROPOSED RESIDENTIAL TOWNHOUSES
 LOTS 1 AND 2, REGISTERED PLAN 500

2463-2469 MIMOSA ROW
FOXMAR LTD.
 2533 BURLSLEM ROAD MISSISSAUGA ON L5A 2R5

MISSISSAUGA

SITE SERVICING PLAN

DATE: MAR. 2022	AREA: Z-15	DWG No.:
SCALE: 1:200	DRAWN BY: DM	C101
CITY FILE:	REGION FILE:	PROJECT No. 220-M130-1



- GENERAL NOTES**
1. THE CONTRACTOR IS TO CHECK AND VERIFY ALL DIMENSIONS. IF ANY DISCREPANCIES, THEY MUST BE REPORTED TO THE ENGINEER IMMEDIATELY PRIOR TO CONSTRUCTION.
 2. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES DURING CONSTRUCTION. GAS, HYDRO, TELEPHONE OR ANY OTHER UTILITIES THAT MAY EXIST ON THE SITE OR WITHIN THE STREETS LINES MUST BE LOCATED BY THE CONTRACTOR AND VERIFIED PRIOR TO CONSTRUCTION.
 3. ALL CONNECTIONS SHALL BE INSTALLED AS PER MUNICIPAL STANDARDS AND SPECIFICATIONS.
 4. BUILDERS TO VERIFY TO THE ENGINEER THAT THE FINAL FOOTING ELEVATION AND TOP OF FOUNDATION WALL ELEVATION ARE IN CONFORMANCE WITH THE BUILDING CODE AND THE CERTIFIED GRADING PLAN PRIOR TO PROCEEDING.
 5. THE ELEVATION OF THE SIDE SWALE AT THE BUILDING LINE SHALL BE A MINIMUM OF 150mm BELOW THE BUILDING LINE AT THE CENTRE OF THE SWALE.
 6. OUTSIDE FINISHED GRADE TO BE A MINIMUM OF 150mm BELOW BRICK VENEER ELEVATION.
 7. PRIOR TO ANY SOILING, THE BUILDER IS TO ENSURE THE SOILS CONSULTANT AND/OR THE ENGINEER THAT THE LOT HAS BEEN GRADDED AND TOPSOILED AND SOILS COMPACTED WITH A MINIMUM DEPTH OF 100mm OF TOPSOIL AND NO. 1 NURSERY SOIL AND A MINIMUM DEPTH OF 180mm OF COARSELY GRADED STONE TO BE PROVIDED ON THE ENTIRE LENGTH OF EACH DRIVEWAY OR FIRE SUBGRADE AND THE DRIVEWAY TO BE PAVED WITH A MINIMUM COMPACTED DEPTH OF 75mm OF ASPHALT BETWEEN THE CURBS AND THE GARAGE.
 8. NO DOODING ON ANY LOTS IS PERMITTED UNTIL PRELIMINARY INSPECTION IS DONE BY THE ENGINEER AND THE BUILDER.
 9. AT ALL ENTRANCES TO THE SITE THE ROAD CURBS AND SIDEWALKS WILL BE CONTINUOUS THROUGH THE DRIVEWAY. THE DRIVEWAY GRADE WILL BE COMPATIBLE WITH THE EXISTING OR FUTURE SIDEWALK AND CURB DEPRESSION WILL BE PROVIDED FOR EACH ENTRANCE.
 10. DRIVEWAY GRADES SHOULD NOT BE LESS THAN 2.0% AND NOT GREATER THAN 8%.
 11. LAWN AND SWALES SHALL HAVE A MINIMUM SLOPE OF 1.5% (PREFERRED 2%) AND A MAXIMUM SLOPE OF 4%.
 12. WHERE GRADES IN EXCESS OF 8% ARE REQUIRED, THE MAXIMUM SLOPE SHALL BE 3:1. GRADE CHANGES IN EXCESS OF 100mm ARE TO BE ACCOMMODATED BY USE OF A RETAINING WALL. RETAINING WALLS HIGHER THAN 1.8m SHALL HAVE A FENCE INSTALLED ON THE HIGH SIDE.
 13. THE SERVICE CONNECTION TRENCH WITHIN THE TRAVELLED PORTION OF THE ROAD ALLOWANCE SHALL BE BACKFILLED WITH UNSHRINKABLE BACKFILL MATERIAL AS PER C.M. STDS. 2228.039, 2228.031 AND 2228.032 UNLESS OTHERWISE SPECIFIED PRIOR APPROVAL FOR OTHER BACKFILL MATERIAL HAS BEEN OBTAINED.
 14. ALL WATERMANS AND WATER SERVICE MATERIALS AND CONSTRUCTION METHODS MUST CORRESPOND TO CURRENT MUNICIPAL STANDARDS & SPECIFICATIONS.
 15. WATERMANS AND/OR WATER SERVICES ARE TO HAVE A MIN. DEPTH OF 1.5m WITH A MIN. HORIZONTAL SPACING OF 1.2m FROM THEMSELVES AND OTHER UTILITIES.
 16. SEDIMENT CONTROL FENCE TO BE INSTALLED AS PER C.M. STS. 2248.018.
 17. ALL DAMAGED AND DISTURBED AREAS TO BE RESEATED WITH TOPSOIL AND SOIL.

- FIRE DEPARTMENT**
1. FIRE ROUTE WILL BE DESIGNATED AS PER CITY OF MISSISSAUGA BYLAW (1036-01) AS AMENDED PRIOR TO OCCUPANCY OF THE BUILDING.
 2. FIRE ROUTES TO BE DESIGNED TO WITH STAND A LOAD NOT LESS THAN 1.35KN/m PER AXLE AND HAVE A CHANGE IN GRADIENT OF NOT MORE THAN 1% OVER A DISTANCE 15.0m AS PER BYLAW 1036-01.
 3. ALL 12.0m TURNING RADI SHALL HAVE MIN. CLEARANCE OF 3.0m BETWEEN THE CENTRE LINE OF TURNING RADI AND ANY CURB OR PART OF BUILDING.
 4. PRIVATE FIRE HYDRANTS SHALL BE FLOW TESTED AND COLOUR CODED IN CONFORMANCE WITH THE REGION OF PEELE UNIFORM MARKING OF HYDRANTS.

- ROADS**
1. ALL FILL WITH ROAD ALLOWANCE AND EASEMENTS TO BE COMPACTED TO MIN 98% STANDARD PROCTOR DENSITY. THE SUITABILITY AND COMPOSITION OF ALL FILL MATERIALS TO BE CONFIRMED BY A REGISTERED SOIL CONSULTANT TO THE CITY ENGINEER AND THE SUBGRADE OF ALL ROADWAYS SHALL BE PROOF ROLLED UNDER THE SUPERVISION OF THE SOILS CONSULTANT PRIOR TO THE INSTALLATION OF ANY ROAD BASE MATERIALS.
 2. THE DEVELOPER/CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES PRIOR TO AND DURING CONSTRUCTION. LOCATION OF EXISTING UTILITIES, WATERMANS AND OTHER STRUCTURES OF ANY KIND OR UTILITIES AND STRUCTURES ARE NOT NECESSARILY SHOWN ON THE DRAWINGS. PRIOR TO COMMENCEMENT OF WORK CONTRACTOR MUST EXAMINE THE ACCURACY OF SUCH EXISTING UTILITIES AND STRUCTURES WHETHER SHOWN OR NOT AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM. ANY DISCREPANCIES TO LOCATION OF EXISTING WATERMANS AND SEWERS TO BE RECTIFIED AT DEVELOPER/ CONTRACTOR'S EXPENSE.
 3. THE DEVELOPER/CONTRACTOR MUST ENSURE THAT A SUBGRADE CERTIFICATE IS ISSUED BY THE GEOTECHNICAL SOILS CONSULTANT TO THE ENGINEER. ONLY UPON VERIFICATION AND APPROVAL OF THE LOCAL VARIOUS INSPECTOR DEPARTMENT WILL THE COMMENCEMENT OF ANY ROAD BASE MATERIALS BE PLACED. FAILURE TO FOLLOW THIS PROCEDURE WILL MEAN THE REMOVAL OF ROAD BASE MATERIALS AND ADDITIONAL TESTING THAT PROPER COMPACTION HAS BEEN ACHIEVED AT THE SUBGRADE AT DEVELOPER/CONTRACTOR'S EXPENSE.
 4. TRENCH BACKFILL ON PROPOSED ROADS SHALL COMPLY WITH CITY OF MISSISSAUGA SECTION 4.02.04. TRENCH BACKFILLING ON ROADS AS PROVIDED IN THE CITY'S DEVELOPER REQUIREMENT MANUAL.
 - a) THE TOP 100mm OF THE SUBGRADE IS TO BE COMPACTED TO A MINIMUM 98% OF SPD WITHIN 2% OF THE OPTIMUM MOISTURE CONTENT.
 5. ALL CONNECTIONS WITHIN ROAD RIGHTWAY ANY EXISTING ROAD TO BE BACKFILLED WITH UNSHRINKABLE BACKFILL MATERIAL AS PER C.M. STDS. 2228.039, 2228.031 AND 2228.032 UNLESS OTHERWISE SPECIFIED PRIOR APPROVAL FOR OTHER BACKFILL MATERIAL HAS BEEN OBTAINED.
 6. ALL OTHER EXCAVATIONS WITHIN EXISTING ROAD ALLOWANCE SHALL BE BACKFILLED TO SUBGRADE WITH UNSHRINKABLE MATERIAL AND COMPACTED TO A MINIMUM OF 98% STANDARD PROCTOR DENSITY. SURFACE RESTORATION SHALL BE EQUAL TO OR BETTER THAN EXISTING CONDITION IN ACCORDANCE WITH P.S.S. 807.
 7. CURBS TO BE AS PER O.P.S.D. 606.118 UNLESS OTHERWISE NOTED.
 8. SUBURBAN UNDERNEATH ALL CURBS TO BE MINIMUM 100mm AS PER C.M. STDS. 2228.040 AND 2228.045 AND ON EXISTING ROADS.
 9. ALL DISTURBED AREAS WITHIN EXISTING ROAD ALLOWANCE TO BE RESEATED WITH TOPSOIL AND SOIL TO THE SATISFACTION OF CITY OF MISSISSAUGA.
 10. SIDEWALKS TO BE AS PER C.M. STS. 2248.018 AND 2248.019 AND RESTRAIN RAMP TO BE PROVIDED AT ALL INTERSECTIONS AS PER C.M. STDS. 2248.039 AND 2248.040.

MIMOSA ROW (BY REGISTERED PLAN 500) PIN 13354-0079

FLORADALE DRIVE (BY REGISTERED PLAN 500) PIN 13354-0066

SITE DATA

	INDUSTRIAL
ZONING	
LOT AREA	m ²
BUILDING AREA	m ²
LANDSCAPED AREA	m ²
TOTAL PAVED AREA	m ²
PARKING REQUIRED	
PARKING PROVIDED INCL. HOCP	
LOADING SPACE REQUIRED	
LOADING SPACE PROVIDED	

PRIOR TO ANY CONSTRUCTION, THE BUILDER IS TO CONFIRM WITH ARCHITECT OR THE OWNER, THAT THE PROPOSED FINISHED FLOOR AND TOP OF WALL ELEVATIONS ARE IN CONFORMANCE WITH THE ARCHITECTURAL DRAWINGS.

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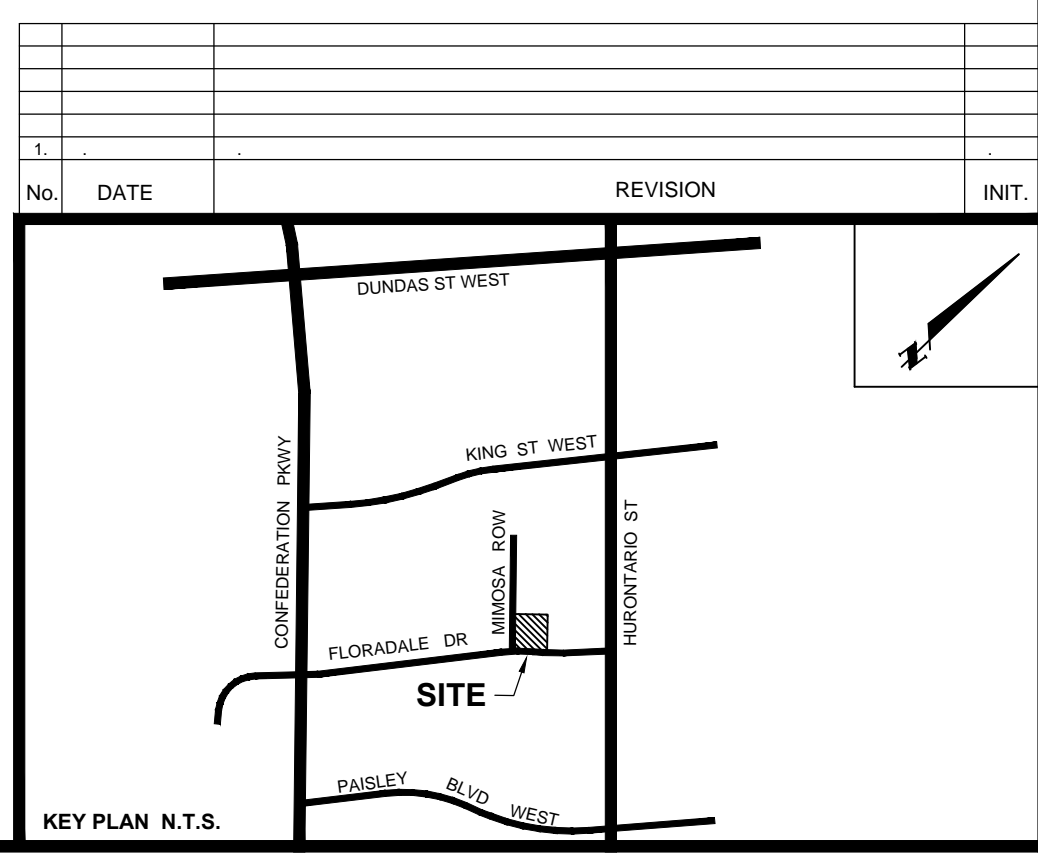
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- LEGEND**
- ×(000.00) - EXISTING ELEVATION TO REMAIN
 - × 000.00 - EXISTING ELEVATION
 - - DIRECTION OF SURFACE FLOW
 - +000.00 - PROPOSED ELEVATION
 - ▭ - CATCH-BASIN WITH TEMPORARY SEDIMENT CONTROL
 - ⊗ - EX. TREE TO REMAIN
 - ⊘ - EX. TREE TO BE REMOVED
 - - EASEMENT
 - ⊕ - FIRE HYDRANT
 - ⊙ - SUMP PUMP
 - ⊖ - HYDRO METER
 - ⊗ - GAS METER
 - ⊙ - METER ROOM
 - ⊕ - ROOF DOWNSPOUTS
 - ⊕ - PROPERTY LINE



C.M. BENCHMARK No. 337 ELEVATION: 108.73
DESCRIPTION: ON THE SOUTH FACE JUST EAST OF THE GARAGE DOOR OF A GREY AND YELLOW BRICK BUNGALOW NO. 111 AT THE NORTHEAST CORNER OF FLORADALE DRIVE AND CONFEDERATION PARKWAY.

SKIRA & ASSOCIATES LTD.
CONSULTING ENGINEERS
3464 Samenyk Court, Suite 100, Mississauga, Ontario L5C 4P8
Tel: (905) 276-5100 Fax: (905) 270-1936 Email: info@skiraconsult.ca

PROPOSED RESIDENTIAL TOWNHOUSES
LOTS 1 AND 2, REGISTERED PLAN 500

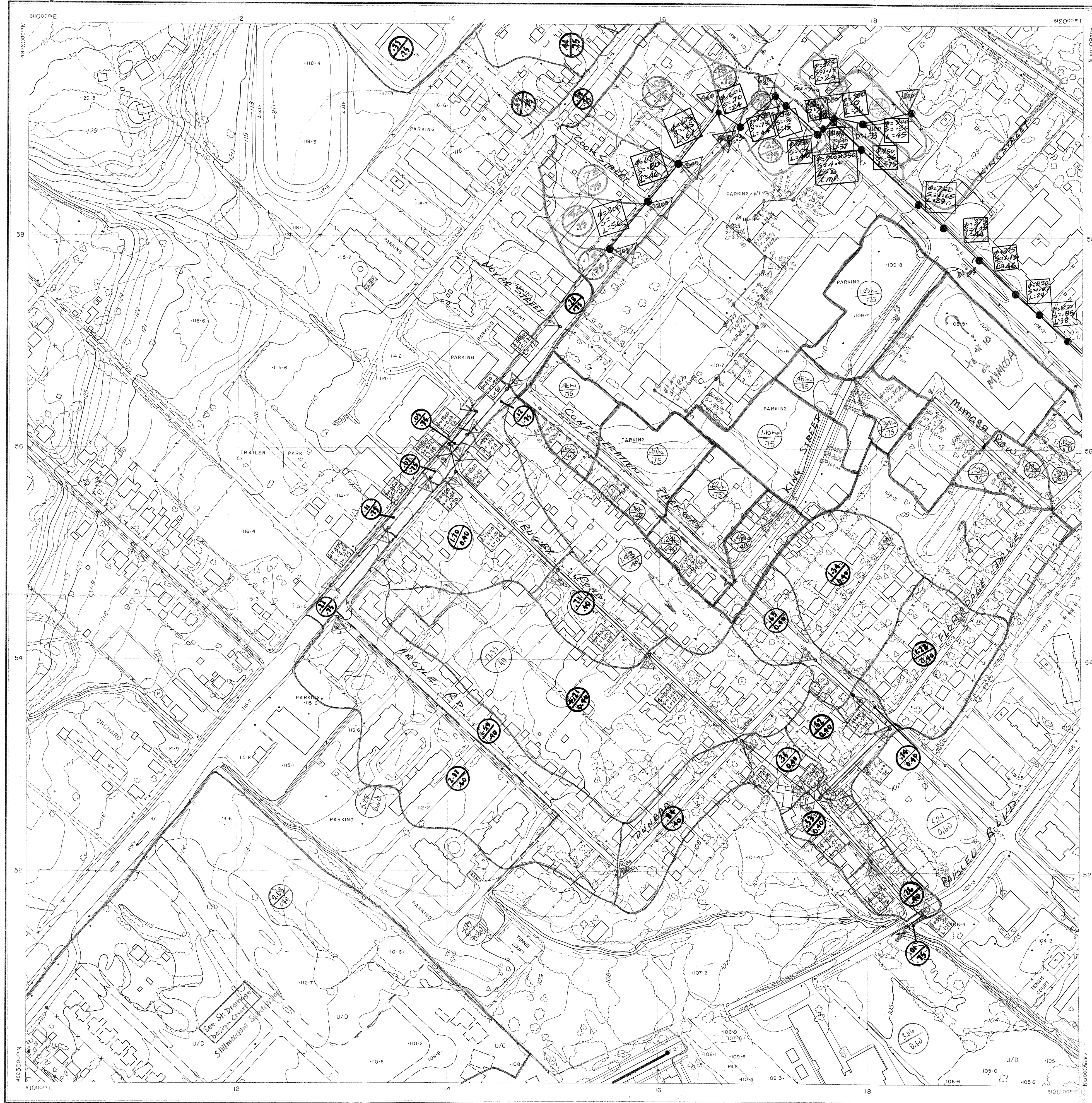
2463-2469 MIMOSA ROW
FOXMAR LTD.
2533 BURSLEM ROAD MISSISSAUGA ON L5A 2R5

MISSISSAUGA
SITE GRADNG PLAN

DATE: MAR 2022	AREA: 2-15	DWG No.:
SCALE: 1:200	DRAWN BY: DM	C102
CITY FILE:	REGION FILE:	PROJECT No. 220-M130-2

Appendix A
Sanitary Sewer Design Chart

Appendix B
Existing Storm Sewer Drainage Record &
Storm Sewer Design Chart

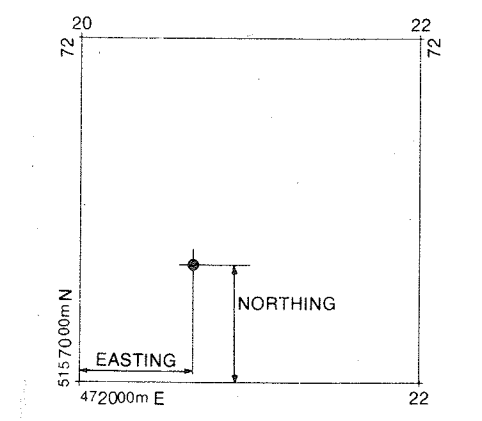


Horizontal Control Point	△ 2 027 76 0001
Vertical Control point, with Elevation	○ 359 16 1 8 027 76 0001
Bench Mark	+ 10
Photo Centre	+
Railroad	—+—+—+—
Narrow Gauge	—+—+—+—
Abandoned	—+—+—+—
Street Car Line	—+—+—+—
Tunnel	—+—+—+—
Turntable	—+—+—+—
Road, Hard Surface, with Median	—+—+—+—
Loose Surface	—+—+—+—
Driveway	—+—+—+—
Farm Lane, Cart Track, Wagon Road	—+—+—+—
Footpath, Trail	—+—+—+—
Building, Garage, Shed, with Property Division	—+—+—+—
Under Construction, Foundation	—+—+—+—
Ruins	—+—+—+—
River, Stream, Canal	—+—+—+—
Approximate Alignment	—+—+—+—
Disappearing	—+—+—+—
Split	—+—+—+—
Flow Arrow	—+—+—+—
Shoreline, Lake	—+—+—+—
Approximate Alignment	—+—+—+—
Flooded Land	—+—+—+—
Marsh	—+—+—+—
Swamp	—+—+—+—
Ditch, Drain (with culvert)	—+—+—+—
Airport Runway	—+—+—+—
Area Outline, eg. Auto Wrecker, Cemetery, Nursery, Pile	—+—+—+—
Area Outline, eg. Under Development, Orchard, Park	—+—+—+—
Beacon	○ R4010
Billboard, Bleachers	—+—+—+—
Bridge, Footbridge	—+—+—+—
Chimney	—+—+—+—
Cliff, Cut and Fill	—+—+—+—
Conveyor, Crane (Moveable)	—+—+—+—
Crib	—+—+—+—
Culvert	—+—+—+—
Dam, Beaver Dam	—+—+—+—
Dike	—+—+—+—
Falls	—+—+—+—
Fence	—+—+—+—
Fire Tower, Flag Pole	—+—+—+—
Gate, Guard Rail	—+—+—+—
Golf Bunker, Green, Tee	—+—+—+—
Hedge	—+—+—+—
Locks	—+—+—+—
Logged Area, Reforested Area	—+—+—+—
Monument, Shrine, Fountain etc.	—+—+—+—
Parking Area, Hard and Loose Surface	—+—+—+—
Pipeline	—+—+—+—
Pit	—+—+—+—
Pole, Light Standard, Flood Light	—+—+—+—
Power Transmission Line, with Poles, with Pylons	—+—+—+—
Rapids	—+—+—+—
Reservoir	—+—+—+—
Rock, Rock Outcrop	—+—+—+—
Scrub	—+—+—+—
Sidewalk	—+—+—+—
Sign, Overhead	—+—+—+—
Steps	—+—+—+—
Tower, Wind Pump	—+—+—+—
Tree	—+—+—+—
Wall	—+—+—+—
Wooded Area	—+—+—+—
Contour, Index	5.0
Intermediate	—+—+—+—
Depression	—+—+—+—
Spot Elevation, Water Level	101.5 W/L 971

ONTARIO GRID REFERENCE

The grid lines form part of the Ontario Grid and are at 200 metre intervals. To give a reference defining the position of a point to within 1 metre proceed as follows:

EXAMPLE:	
EASTING	
Take west edge of 200 metre square in which the point lies and read the figures opposite this line on the north or south margin.	4720
Estimate millimetres from the same grid line to the point and multiply by 2.	70
	472070
NORTHING	
Take south edge of 200 metre square in which the point lies and read the figures opposite this line on the east or west margin.	51570
Estimate millimetres from the same grid line to the point and multiply by 2.	64
	5157064
1 METRE REFERENCE	4720705157064

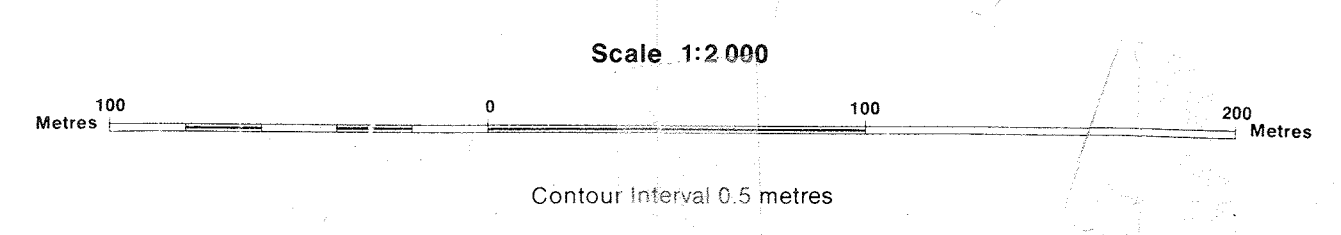


GENERAL INFORMATION

North American Datum 1927.
 Universal Transverse Mercator (6°) projection.
 Zone 17 Central meridian of this sheet is 81°W.
 The approximate geographical position for the centre of this sheet is 43°35'N
 Full grid values in meters are given at the sheet corners.
 Grid interval on this sheet 200 metres.
 Full details of the grid are published separately and may be obtained from the Surveys and Mapping Branch, Ministry of Natural Resources, Province of Ontario.
 Contour interval 0.5 metres.
 Note: one grid square on this map represents 4 hectares on the ground.
 Map base produced in 1977 from photography flown in April 1977.
 Reproduction of this map is prohibited without the authority of the City of Mississauga Engineering, Works and Building Department.

INDEX TO ADJOINING SHEETS

	02 17 6110 48250	
02 17 6110 48250	02 17 6110 48250	02 17 6110 48250
	02 17 6110 48250	



18 2-15

CITY OF MISSISSAUGA

STORM SEWER DESIGN CHART

SUBDIVISION : 2463-2469 Mimosa Row

MAJOR DRAINAGE AREA: _____

REGION FILE: _____

CONSULTANT : SKIRA & ASSOCIATES LTD.

SHEET No. 1 of 1

PROJECT No. : 220-M130

DESIGNED BY : D.M.

DATE : Mar - '22

$I_{(10YR)} = 1010/(Tc+4.6)^{0.78}$

MANNING'S ROUGHNESS COEFF. $n = 0.013$

LOCATION	FROM MH	TO MH	AREA	RUNOFF COEFF.		ACCUM. AREA	ACCUM. AaxCa	Tc	INTENSITY	EXPECTED FLOW	TYPE OF PIPE	LENGTH	SLOPE	PIPE SIZE NOMINAL	CAPACITY	VELOCITY	TIME OF FLOW	VELOCITY	% FULL	INVERT ELEV.	
			Aa	Ca	AaxCa	A=∑Aa	C=∑AaxCa		I	$Q = \frac{I \cdot A \cdot C}{360}$		L	S	D	e	V	$T = \frac{L}{V \times 60}$	n = 0.009		UPPER	LOWER
	MH#	MH#	ha			ha		min	mm/hr	m ³ /s		m	%	mm	m ³ /s	m/s	min	m/s		MH	MH
PRE DEVELOPMENT																					
EXTERNAL DRAINAGE	TO	10				7.16	5.02	21.53	79.24	1.105	CONC	65.8	0.51	750	0.829	1.82	0.60	2.63	133.2%		
MIMOSA ROW	10	11	0.22	0.75	0.17																
SITE			0.059	0.50	0.03																
			0.111	0.40	0.04	7.55	5.26	22.13	77.85	1.137	CONC	91.1	1.04	675	0.895	2.42	0.63	3.50	127.1%		
EXTERNAL DRAINAGE	TO	11	0.88	0.40	0.35	0.88	0.35														
FLORADALE RD.	11	12	0.245	0.40	0.10																
SITE			0.085	0.50	0.04																
			0.18	0.75	0.14	8.94	5.89	22.76	76.45	1.251	CONC	64.6	0.60	900	1.461	2.23	0.48	3.22	85.6%		
POST DEVELOPMENT																					
EXTERNAL DRAINAGE	TO	10				7.16	5.02	21.53	79.24	1.105	CONC	65.8	0.51	750	0.829	1.82	0.60	2.63	133.2%		
MIMOSA ROW	10	11	0.22	0.75	0.17																
SITE			0.065	0.65	0.04																
			0.111	0.40	0.04	7.56	5.27	22.13	77.85	1.140	CONC	91.1	1.04	675	0.895	2.42	0.63	3.50	127.3%		
EXTERNAL DRAINAGE	TO	11	0.88	0.40	0.35	0.88	0.35														
FLORADALE RD.	11	12	0.245	0.40	0.10																
SITE			0.079	0.65	0.05																
			0.18	0.75	0.14	8.94	5.91	22.76	76.45	1.255	CONC	64.6	0.60	900	1.461	2.23	0.48	3.22	85.9%		