

Phase Two Environmental Site Assessment

2670 & 2690 Erin Center Boulevard, Mississauga, ON

Client:

EMTC Holdings Inc. c/o The Muzzo Group of Companies 50 Confederation Parkway Concord, ON, L4K 4T8

Attention:

Mr. Barry Stern

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1 Executive Summary

EXP Services Inc. (EXP) was retained by EMTC Holdings Inc. c/o EMTC Holdings Inc. c/o The Muzzo Group of Companies (the "Client") to complete a Phase One Environmental Site Assessment (ESA) of the property that encompasses the municipal addresses 2670 & 2690 Erin Center Boulevard, Mississauga, Ontario, which is hereinafter referred to as the "Site" (Figure 1). It is understood that a Record of Site Condition (RSC) will be required as there is an indented change in land use from commercial to mixed-use residential.

This Phase Two ESA was conducted in accordance with the Phase Two ESA standard defined by Ontario Regulation 153/04, as amended (O.Reg.153/04); and in accordance with generally accepted professional practices. Subject to this standard of care, EXP makes no express or implied warranties regarding its services and no third-party beneficiaries are intended. Limitation of liability, scope of report and third-party reliance are outlined in Section 8 of this report.

The objective of the Phase Two ESA was to assess the areas of potential environmental concern (APECs) identified in the Phase One ESA completed by EXP, dated March 26, 2024.

Area of Potential Environmental Concern (APEC)	Location of APEC on Phase One Property	Potentially Contaminating Activity (PCA) ¹	Location of PCA (on-Site or off-Site)	Contaminants of Potential Concern	Media Potentially Impacted (Groundwater, soil and/or sediment)
APEC 1: Entire Site (PCA-1d)	Entire Site	PCA#30-Importation of Fill Material of Unknown Quality.	On-Site	PHCs, VOCs, PAHs, Metals, Inorganics	Soil
APEC 2: Seasonal application of de- icing salt agents (PCA-1e)	Central and northeastern portion of the Site - Parking lot	PCA#48- Salt Manufacturing, Processing and Bulk Storage	On-Site	pH, EC, SAR	Soil
APEC 3: Transformers on Site (PCA-1f)	North portion of the Site	PCA#55-Transformer Manufacturing, Processing and Use.	On-Ste	PCBs	Soil

The APECs identified in the Phase One ESA are provided in the table below.

Based on the findings of the Phase One ESA, a Phase Two ESA was recommended to assess the soil conditions at the Site.

The results and findings of the Phase Two ESA conducted by EXP at the Site are summarized as follows:

• A total of fourteen (14) boreholes were advanced during this time. Boreholes 201 to 214 were advanced by Davis Drilling (Davis) to a maximum depth of 15.9 metres below ground surface (m bgs).



- The general stratigraphy at the Site, as observed in the boreholes, consists of topsoil or asphalt underlain by sand and gravel. Fill material was observed beneath the topsoil and sand and gravel. Fill material generally consisted of clayey silt and/or silty sand which overlies native soil. Native material at the Site generally consists of silt, sandy silt, and/or silt till. Shale bedrock of the Queenston Formation was encountered below the silt till at all borehole locations.
- As part of the Hydrogeological Investigation completed in conjunction with the Phase Two ESA, static water levels in the monitoring wells installed outside of the existing building were recorded on February 4, 2024. The groundwater elevation recorded in the shallow wells installed in the native material ranged from 167.74 masl to 172.30 masl. The groundwater elevation recorded in the deep wells installed in the shale bedrock ranged from 168.90 masl to 170.42 masl.
- Soil samples were submitted for the analysis of petroleum hydrocarbons (PHCs), benzene, toluene, ethylbenzene and xylenes (BTEX), volatile organic compounds (VOCs), metals and inorganics, Polycyclic Aromatic Hydrocarbons (PAHs) and Polychlorinated Bi-Phenols (PCBs).
- Six (6) soil samples were submitted for PHCs including BTEX analysis. All PHC including BTEX parameters were not detected above the laboratory Reporting Detection Limits (RDLs) in all samples submitted.
- Six (6) soil samples were submitted for metals, As, Sb, Se, Hg, CN-, and Cr(VI) analysis. The concentrations of metal and other regulated parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory RDLs
- Six (6) soil samples were submitted for VOC analysis. All VOC parameters were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.
- Six (6) soil samples were submitted for PAH analysis. All PAH parameters were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.
- One (1) soil sample was submitted for PCB analysis. PCBs were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.
- Six (6) soil samples were submitted for EC and SAR analysis. The concentrations of EC and SAR parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory RDLs with the following exceptions: BH202-SS2 (SAR), BH205-SS2 (EC and SAR), BH205-SS6 (EC and SAR), BH209-SS2 (EC and SAR), BH203-SS2 (EC and SAR), and BH211-SS2 (EC). The laboratory RDLs are below the Table 3 SCS.
- No evidence of free product (i.e. visible film or sheen), or odour was observed during soil sampling.

It is anticipated that road salt has been applied across the paved areas of the Site, for the purposes of vehicular and pedestrian safety under conditions of snow and/or ice. As such, it is the QP_{ESA}'s opinion that the applicable Table 3 Site Conditions Standards (SCS) for EC and SAR in soil were exceeded solely because salt was used on the road for the purpose of keeping the road safe for traffic under conditions of snow or ice or both, and therefore these parameters are not considered to exceed as per Section 49.1 (1) of O. Reg. 153/04.

Based on the findings of the Phase Two ESA, it is the opinion of the QP that no additional environmental investigations are required at this time and that an RSC can be filed for the Site for the proposed land use change.



2 Introduction

EXP Services Inc. (EXP) was retained by EMTC Holdings Inc. c/o EMTC Holdings Inc. c/o The Muzzo Group of Companies (the "Client") to complete a Phase One Environmental Site Assessment (ESA) of the property that encompasses the municipal addresses 2670 & 2690 Erin Center Boulevard, Mississauga, Ontario, which is hereinafter referred to as the "Site" (Figure 1). It is understood that a Record of Site Condition (RSC) will be required as there is an indented change in land use from commercial to mixed-use residential.

This Phase Two ESA was conducted in accordance with the Phase Two ESA standard defined by Ontario Regulation 153/04, as amended (O.Reg.153/04); and in accordance with generally accepted professional practices. Subject to this standard of care, EXP makes no express or implied warranties regarding its services and no third-party beneficiaries are intended. Limitation of liability, scope of report and third-party reliance are outlined in Section 8 of this report.

2.1 Site Description

The Site is an irregular shaped parcel of land that measures approximately 4.28 hectares (10.58 acres) in size. The Site is located on the southeast corner of Glen Erin Drive and Erin Center Boulevard in the city of Mississauga, Ontario. At the time of this investigation, the Site consisted of the following:

- The northeastern portion of the Site is asphalt-paved with a narrow-landscaped area located on the Site boundaries.
- The southwestern portion of the Site is landscaped with grass area.
- The central portion of the Site is developed with two (2) Site buildings that have the municipal addresses 2670 Erin Center Boulevard (Site Building A) and 2690 Erin Center Boulevard (Site Building B) and asphalt-paved with parking area towards central-east. The tenants include:
 - 2670 Erin Center Boulevard (Site Building A): Dollarama, Bouclair Home, and Home Sense.
 - 2690 Erin Center Boulevard (Site Building B): Pizza Pizza, Hero's Burger, Walk-in Medical Clinic, Cosmetic Dentistry, Studio 10 Salon, Chiropractor Practice Center.

The areas surrounding the Site buildings have been paved with asphalt and landscaped areas are located along the perimeter of the Site.

It is EXP's understanding that the Site will be redeveloped with nine (9) residential towers of 20 to 44 storeys. The development will include three (3) levels of underground car parking. In support of the proposed development plan which would see the property use change from commercial to multiple mixed-use residential buildings, a Record of Site Condition (RSC) will be required for this property.

Based on a review of historical aerial photographs, chain of title information, historical maps, and other records, the Site was first developed during the mid 1990's for commercial use (retail).

2.2 Legal Description and Property Ownership

The legal description and property ownership are as follows. A legal Site Survey is presented in Appendix B.

Municipal Address	2670 & 2690 Erin Center Boulevard, Mississauga, Ontario				
Current Land Use	Commercial Use (Retail Stores, Restaurants, Medical Walk-in Clinic)				



Proposed Land Use	Mixed- Use Residential					
Legal Description	Blk 1, Pl 43m823 Save And Except Pts 1, 2, Pl 43r16736, Pts 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, Pl 43r35938; Blk 20, Pl 43m823; Pt Blk 5, Pl 43m823 (0.30 Reserves) Des As Pt 6, Pl 43r20654; Pt Of Eglinton Avenue Being Pt Lt 2, Registrar's Compiled Pl 1003, Des As Pts 2, 4, 7, 8, 11, 13 Pl 43r20654; Save And Except Pts 19, 20, Pl 43r35938 As Closed By Bylaw No. Lt1580524; Pt Blk 18, Pl 43m823 (0.30 Reserve) Des As Pt 14, Pl 43r20654; Blk 4, Pl 43m823 (0.30 Reserve); Blk 17, Pl 43m823 (0.30 Reserve), Save And Except Pts 16, 17, 18, Pl 43r35938; Blk 6, Pl 43m823 (0.30 Reserve), Save And Except Pts 16, 17, 18, Pl 43r35938; Blk 6, Pl 43m823 Subject To An Easement As In Vs162364 Subject To An Easement Over Pts 9, 10, 11, 12, 13, 14, 15, 24, 26, 27, 28, 29, 30, 31, 32, 33, Pl 43r15700 And Pts 10, 11, 12, 13, 14, 15, 16, 17, Pl 43r16222, Save And Except Pts 1, 2, 3, 9, 10, 11, Pl 43r35938 In Favour Of Mississauga Hydro-electric Commission And Bell Canada As In Lt955564 Subject To An Easement Over Pt 1, Pl 43r16217, Pt 1, Pl 43r16218, Pt 1, Pl 43r16219 In Favour Of The Corporation Of The City Of Mississauga As In Lt955568 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r20653 In Favour Of Mississauga As In Lt955568 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r20653 In Favour Of Mississauga Hydro-electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 3, 4, Pl 43r20654 In Favour Of The Regional Municipality Of Peel As In Lt1741002 Subject To An Easement Over Pts 2, 3, 7, Lpl 43r16222 And Pts 16, 17, 18, 21, 22, 34, 35, 38, Pl 43r15700, Save And Except Pts 1, 4, 5, 6, 7, 8, 9, 12, 13, 14, 15, pl 43r35938 In Favour Of The Regional Municipality Of Peel As In Lt1741002 Subject To An Easement Over Pt 5, Pl 43r16736 In Favour Of Mississauga Hydro-electric Commission As In Lt130471 Together With An Easement As In Ro551159 Subject To An Easement In Gross Over Parts 1,2,3,4 And 5, 43r39842 As In Pr3811615					
Property Identification Number (PIN)	Part of PIN 1351 – 20035 (LT)					
Approximate Universal Transverse Mercator (UTM) coordinates	NAD83 17T 603714.75 m E m 4823575.93 m N Google Earth					
Accuracy Estimate of UTM	10-15 m					
Measurement Method	Google Earth					
Site Area	4.28 hectares (10.58 acres)					
Property Owners, Owner Contact and Address	Mr. Barry Stern EMTC Holdings Inc. c/o The Muzzo Group of Companies 50 Confederation Parkway Concord, ON, L4K 4T8					



2.3 Current and Proposed Future Uses

Based on a review of historical aerial photographs, chain of title information, historical maps, and other records, the Site was first developed circa 1990s. The Site was developed with two (2) Site buildings – A & B and a parking lot, inferred to be used for commercial use. At the time of the Site visit, the Site buildings were developed with slab-on grade single-storey buildings. Site Building A has a footprint of approximately 4,357 m2 (46,899 ft2) and Site Building B has a footprint of approximately 1,621 m2 (17,447 ft2). It is to be noted that only tenant - Home sense located in Site Building A has a mezzanine in the northern part of the building that is used for office space and employees lounge / lunch area. The mezzanine is equipped with basic home appliances such as refrigerator, microwave and coffee makers. The northeastern portion of the Site is asphalt-paved with a narrow-landscaped area located on the Site boundaries. The southwestern portion of the Site is landscaped with grass area.

It is EXP's understanding that the Site will be redeveloped with nine (9) residential towers of 20 to 44 storeys and will be completed in various phases. The development will include three (3) levels of underground car parking. In support of the proposed development plan which would see the property use change from commercial to multiple mixed-use residential buildings, a Record of Site Condition (RSC) will be required for this property, in accordance with Ontario Regulation 153/04, as amended (O.Reg.153/04).

2.4 Applicable Site Condition Standards

Analytical results obtained for Site, soil and groundwater, were assessed against Site Condition Standards (SCS) as established under subsection 169.4(1) of the Environmental Protection Act, and presented in the document Ministry of Environmental, Conservation, and Parks (MECP) "Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act", ("SGWS" Standards), (MECP, 2011a). Tabulated background SCS (Table 1) applicable to environmentally sensitive Sites and effects based generic SCS (Tables 2 to 9) applicable to non-environmentally sensitive Sites are provided in MECP (2011a). The effects based SCS (Tables 2 to 9) are protective of human health and the environment for different groundwater conditions (potable and non-potable), land use scenarios (residential, parkland, institutional, commercial, industrial, community and agricultural/other), soil texture (coarse or medium/fine) and restoration depth (full or stratified).

Tables 1 to 9 of MECP (2011a) are summarized as follows:

- Table 1 applicable to sites where background concentrations must be met (full depth), such as sensitive sites where sitespecific criteria have not been derived;
- Table 2 applicable to sites with potable groundwater and full depth restoration;
- Table 3 applicable to sites with non-potable groundwater and full depth restoration;
- Table 4 applicable to sites with potable groundwater and stratified restoration;
- Table 5 applicable to sites with non-potable groundwater and stratified restoration;
- Table 6 applicable to sites with potable groundwater and shallow soils;
- Table 7 applicable to sites with non-potable groundwater and shallow soils;
- Table 8 applicable to sites with potable groundwater and that are within 30 m of a water body; and,
- Table 9 applicable to sites with non-potable groundwater and that are within 30 m of a water body.

For assessment purposes, EXP selected the MECP, also known as MECP (2011) Table 3: Full Depth Generic Site Condition Standards (SDS) in a Non-Potable Ground Water Condition for Residential/Parkland/Institutional Property Use and medium-fine textured soil conditions. The selection of this category was based on the following factors:

- At least 2/3 of the Site has an overburden thickness greater than 2 m.
- The Site is not located within 30 m of a surface water body or an area of natural significance.



- The soil at the Site has a pH value between 5 and 9 for surficial soils; and, between 5 and 11 for subsurface soils.
- The property is not within an area of natural significance; does not include, nor is it adjacent to an area of natural significance, nor is it part of such an area; and, it does not include land that is within 30 m of an area of natural significance, nor is it part of such an area.
- The Site is serviced by the City of Mississauga water distribution system which obtains its municipal water supply from Lake Ontario.
- The predominant soil type on the Site is medium-fine textured.
- The proposed land use is residential.
- There is no intention to carry out a stratified restoration at the Site.

2.5 General Objective of the Phase Two ESA

The objective of the Phase Two ESA was to assess the areas of potential environmental concern (APECs) identified in EXP's Phase One ESA completed on March 26, 2026 (hereinafter referred to as "the 2024 Phase One ESA"); and to obtain soil data to determine the chemical quality of soil on the Site. The objective of the investigation was to support the Site redevelopment. APECs are shown on Figure 4.



3 Background Information

3.1 Physical Setting

The following physiographic, geological and soil maps were reviewed:

- "Toporama"; Natural Resources Canada. Map 030M11. Scale 1:15,000. 2008.
- Quaternary Geology of Ontario geology_II.shp [computer file], Ontario: Ontario Geological Survey, 2000.
- Bedrock Geology of Ontario geology_ll.shp [computer file], Ontario: Ontario Geological Survey, 2000.
- Bedrock Topography and Overburden Thickness Mapping, Southern Ontario, prepared by Ontario Geological Survey, published 2006.

Based on the review of the above maps, the following information was obtained:

- There is a decrease in elevation towards the north and east in the Phase I Study Area. A permanent stream is located approximately 150 m east of the Site, and it flows in a southeasterly direction towards Millet Creek draining in Lake Ontario. Based on a review of the topographic map, the inferred groundwater flow at the Site and surrounding properties is anticipated to flow in an southerly direction.
- The Site is approximately 177 m above sea level and mostly flat.
- The Site and surrounding areas are dominated by Till Moraines deposits that consist predominantly of clay to silt-textured till (derived from glaciolacustrine deposits or shale).
- The bedrock in the general area of the Site is part of a group belonging to the Queenston Formation, primarily consisting of shale, limestone, dolostone and siltstone.
- Based on a review of "Bedrock Topography and Overburden Thickness Mapping, Southern Ontario, prepared by Ontario Geological Survey, published 2006," the bedrock in the vicinity of the Site is anticipated to be encountered at a depth of approximately 11 metres below ground surface (mbgs).

3.2 Previous Environmental Investigations

The need for a Phase Two ESA at the Site was identified as per the Phase One ESA completed by EXP dated March 26, 2024. Based on the findings of the Phase One ESA, including a review of a previously completed environmental reports, various Potentially Contaminating Activities (PCAs) were identified within the Phase One ESA Study Area (see Figure 2).

Several previous environmental reports of the Site were provided to EXP by the Client. It is noted that the previous reports were completed for the larger Erim Mills Town Centre Complex which included the Site. Significant findings are summarized below.

- A report entitled *"Phase I Environmental Site Assessment, 5100 Erin Mills Parkway & 2690 Erin Centre Boulevard, Mississauga, Ontario"* dated April 28, 2014, was prepared for OPB (EMTC) Inc. by its agent and manager 20 Vic Management Inc. by Pinchin Environmental. Pertinent information from the report is as follows:
 - The subject property consisted of a multi-tenant commercial building at 5100 Erin Mills Parkway, and a one -storey multi-tenant commercial building at 2690 Erin Centre Boulevard.

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- Three (3) generator rooms with AST containing diesel fuel were identified at 5100 Erin Mills Parkway. These ASTs are located off-Site.
- 5100 Erin mills Parkway was constructed in 1989, and 2690 Erin Centre Boulevard in 1993.
- The tenant, London Cleaners, located on-Site, was reportedly a dry-cleaning drop-off depot.
- The property at 2670 Erin Centre Boulevard was listed as being occupied by a dry cleaner, however according to Site Representative it was a dry-cleaning depot.
- Based on the previous reports, the Site had a geotechnical investigation which noted the stratigraphy as clayey silt fill, compact to dense till to approximately 1.7 to 5.5 m bgs, followed by red shale bedrock with limestone interbeds. Groundwater ranged between 3.4 to 4.2 m bgs.
- Based on the findings of the Phase I ESA, no potential environmental concerns were identified.
- A report entitled "Phase I Environmental Site Assessment Update, Erin Mills Shopping Centre, Mississauga, Ontario" dated November 5, 2018, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Golder. Pertinent information from the report is as follows:
 - The subject property was noted to consist of the municipal address 5100 Erin Mills Parkway.
 - Based on the ERIS report, Loblaws Properties Ltd. At the Pumps, located approximately 30 m southwest of the Site at 5010 Glen Erin Drive, was listed as a gasoline service station with two (2) liquid fuel tanks, with expiration November 11, 2010.
 - Based on the findings, no issues of potential environmental concern were identified for the Site.
- A report entitled "Waste Audit Report, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, ON, L5M 4Z5", by Green for Life Environmental (GFL) in January 2019. No pertinent information related to an environmental concern for the Site was identified.
- A report entitled "Investigation of Indoor Air Quality, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 26, 2019, was prepared for Cushman & Wakefield Ltd. by Pinchin. Pertinent information from the report is as follows:
 - The survey addressed all common areas in the shopping mall.
 - Elevated dust measurements were recorded in several areas of the building.
 - Total Volatile Organic Compounds (TVOC) measurements were slightly elevated on the lower level in the vicinity of a fragrance shop and an eyeglass clinic.
- A report entitled "Asbestos Reassessment, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 28, 2019, was prepared for Cushman & Wakefield Asset Services by Pinchin. Pertinent information from the report is as follows:
 - No samples were collected as part of the assessment. The purpose of the report was to assess changes in the condition and quantity of previously reported asbestos-containing materials.
 - Asbestos cement (transite) rainwater leaders are present in various locations in the building, in good condition.
- A report entitled "Water Sample Collection and Analysis for Lead, Erin Mills Town Centre", dated March 4, 2019, was prepared for Cushman & Wakefield by Ontario Environmental & Safety Network Ltd. No pertinent information related to an environmental concern for the Site was identified.
- A report entitled "Test Pit Program, Erin Mills Town Centre (Former Sears), 5100 Erin Mills Parkway, Mississauga, Ontario", dated March 6, 2020, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Pinchin. Pertinent information from the report is as follows:
 - Sixteen (16) bulk soil samples were submitted for PHCs, VOCs, PAH, and metal and inorganics to a maximum depth of 4.27 m bgs using an excavator.
 - The soil samples submitted met Table 3 SCS with the exception of Electric Conductivity (EC) and Sodium Adsorption Ratio (SAR) exceedance. These exceedances can be discounted based on the recent exemption that was passed

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by the MECP associated with areas that have been subject to the application of de-icing products to walkways and driveways (eg. Road salt).

- A report entitled *"Phase I Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).
 - The Phase I ESA was prepared for Muzzo Group with the objective to identify issues of environmental concern to the Site in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
 - Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good condition. Floor drains were noted to be in close proximity to the AST's. These ASTs are located off-Site.
 - Based on the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants identified from the ERIS report and the potential fill materials used during development on-Site were considered a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for groundwater at the Site.
 - Based on the findings of the Phase I ESA, a Phase II was recommended.
- A report entitled *"Phase II Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was prepared for Muzzo Group to support identified on-Site concerns of waste generation, and off-Site concerns of a nearby gas station and hydro one facility. Based on recommendations made in the Phase I ESA (EXP, 2020).
 - Between March 2 and 6, 2020, nine (9) boreholes were advanced at the Site (BH1, BH2, BH5, BH6, BH9, BH11, , BH13, BH17 and BH20). Five (5) of the boreholes were equipped with monitoring wells (BH1, BH5, BH9, BH17 and BH20).
 - Soil samples were collected and submitted for the analysis of one or more of the following: Petroleum Hydrocarbons (PHCs), Volatile Organic Compounds (VOCs), Metals and Inorganics (M&I), Polycyclic Aromatic Hydrocarbons (PAHs), Polychlorinated Biphenyls (PCBs), pH, and grain-size (75-micron sleeve).
 - Groundwater samples were collected from one (1) monitoring well (MW17) and analyzed for PHC, Metals and VOCs. It was noted that the collection of groundwater from MW1, MW5, MW9 and MW20 were attempted on two (2) separate occasions, however, groundwater was not encountered at these locations.
 - Groundwater flow direction could not be determined as three (3) water levels screened within the sample geological formation are required to determine groundwater flow at the Site. Based on local topography, groundwater was anticipated to flow in a northeast direction towards a tributary of the Credit River.
 - The concentrations of all parameters submitted for the soil samples from the boreholes were either below the applicable MECP (2011) Table 7 Site Condition Standards (SCS) for residential/parkland/institutional property with medium to fine soil, or not detected above the laboratory detection limits, with the exception of the following:

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Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

- Electrical Conductivity (EC) and Sodium Adsorption Ratio (SAR) in BH5 SS2 (0.8 1.5 m bgs), BH9 SS2 (0.8 1.5 m bgs), BH11 SS2 (0.8 1.5 m bgs), and BH13 SS2 (0.8 1.5 m bgs). It was inferred the high concentrations were due to de-salting activities for road safety or for pedestrian walkways. As such, the elevated EC/SAR in soil was not considered an exceedance based on section 49.1 of O.Reg. 407/19.
- The concentrations of all parameters submitted for groundwater for one (1) monitoring well were either below the applicable MECP (2022) Table 7 SCS or not detected above laboratory detection limits.
- Based on the results of the Phase II ESA (EXP, 2020), no further work was recommended at the time. It was noted that, should an RSC be required for the Site or smaller portions of the Site, additional environmental work would be required to support those submissions.
- A report entitled *"Phase I Environmental Site Assessment Update, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated October 18, 2022, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).
 - The Phase I ESA Update was prepared for Muzzo Group for due diligence purposes in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
 - Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good condition. Floor drains were noted to be in close proximity to the AST's. These ASTs are not located on-Site.
 - According to information provided by the Site Representative and based on a reviewed Environmental Control Incident Report provided by the MECP, the spill occurred on November 27, 2021, during the construction activities in the corridor of mall entrance F in the west portion of 5100 Erin Mills Parkway, a diesel-filled construction heater was temporarily stationed on the concrete walkway adjacent to the building.
 - EXP was made aware that since the previous Phase I ESA, there was a fire On-Site Fire at 2575 Erin Mills Parkway in the building in 2021.
 - Similar to the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants
 identified from the ERIS report and the potential fill materials used during development on-Site were considered
 a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent
 to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for
 groundwater at the Site.
 - Based on the Phase I ESA Update (EXP, 2022) findings, the fire and spill are considered a low concern and immediate action is not required at this time. However, it was noted that this will need to be identified as an Area of Potential Environmental Concern (APEC) in any future O. Reg 153/04 compliant Phase One ESA and investigated through a Phase Two ESA both of which will be required for the purpose of filing an RSC.
 - It is noted that based on the nature and distance from the spill and the fire to the Site, neither we considered to contribute to an APEC.

4 Scope of the Investigation

4.1 Overview of Site Investigation

The objective of the Phase Two ESA was to assess the APECs to obtain soil data to further characterize the Site. APECs are shown on Figure 4.

4.1.1 Scope of Work

The scope of work for the Phase Two ESA was as follows:

- Request local utility locating companies (e.g. cable, telephone, gas, hydro, water, sewer and storm water) to mark any underground utilities present at the Site;
- Retain a private utility locating company to mark any underground utilities present in the vicinity of the proposed borehole locations and to clear the individual borehole locations;
- Oversee a licensed drilling company to advance a total of fourteen (14) boreholes across the Site.
- Collect representative soil samples from the boreholes for laboratory analysis of petroleum hydrocarbons (PHC) fraction (F1 F4) including benzene, toluene, ethylbenzene and xylenes (BTEX), metals and inorganics, Polycyclic Aromatic Hydrocarbons (PAHs), Volatile Organic Compounds (VOCs) and Polychlorinated Bi-Phenyls (PCBs). Please note that not all fourteen (14) of the boreholes were sampled as part of the Phase Two ESA. Soil samples were selected to provide adequate coverage of the APECs identified in the Phase One ESA (EXP, 2024), as shown in Table 3.
- Complete an elevation survey of the boreholes and monitoring wells to determine the groundwater flow direction in the groundwater unit(s) identified beneath the Site; and,
- Analyze the data and prepare a report of the findings, in accordance with O.Reg.153/04.

It is noted that the boreholes advanced as part of the Site investigation were also utilized to support preliminary geotechnical and hydrogeological investigations. As such, the completion depths of the boreholes were determined to support the respective investigations. Furthermore, given that the Phase One ESA did not identify and potential contaminants of concern (pCOC) in groundwater, installing monitoring wells was not considered part of the scope for the Phase Two ESA. It is noted however that monitoring wells were installed at the Site in support of the hydrogeological investigation and those wells were relied upon in this Phase Two ESA to determine groundwater flow direction at the Site.

4.2 Media Investigated

The Phase Two ESA included the investigation of the Site soil. As the Phase One ESA did not identify and potentially contaminants of concern (pCOC) in groundwater and there were no surface water bodies on the Site, groundwater and sediment sampling were not required.

Soil was selected as the media to be investigated due the presence of the PCAs/APECs at the Site that were identified in the Phase One ESA.

4.3 Phase One Conceptual Site Model

The Phase One Conceptual Site Model (CSM) is incorporated into the Phase Two CSM, presented in Appendix F.



4.4 Deviations from Sampling and Analysis Plan

The field investigative and sampling program was carried out following the requirements of the Site Sampling and Analysis Plan (SAAP) presented in Appendix A. No significant deviations from the SAAP were reported that could affect the sampling and data quality objectives for the Site.

4.5 Impediments

Soil duplicate samples, including one (1) duplicate sample for every ten (10) samples analyzed, were not obtained as part of the Phase Two ESA. However, EXP advises that duplicates be obtained prior to the RSC submission for quality assurance (QA) purposes. No other impediments were encountered during the completion of this Phase Two ESA could affect the sampling and data quality objectives for the Site



5 Investigation Method

5.1 General

The Site investigative activities consisted of the following:

• Borehole drilling to facilitate the collection of soil samples for geologic characterization and/or chemical analysis.

Boreholes were advanced in the overburden soils by a licensed drilling company under the full-time supervision of EXP staff. The drilling equipment used to advance the boreholes is described below. No petroleum-based grease or solvent was used during drilling activities.

The approximate locations of the boreholes and monitoring wells are shown on Figure 5A. The approximate locations of the boreholes and monitoring wells with APECs are shown on Figure 5B.

5.2 Underground Utilities

Prior to the commencement of drilling activities, the location of underground utilities including but not limited to cable, telephone, natural gas, electrical lines, water, sewer and storm water conduits were marked out by public locating companies. In addition, a private utility locating service was retained to clear individual borehole locations. Refer to Figure 3 for the underground utility locations depicted on the Site plan.

5.3 Borehole Drilling

The fieldwork for the Phase Two ESA drilling program was carried out between January 4 to January 22, 2024.

A total of fourteen (14) boreholes were advanced during this time. Boreholes 201 to 214 were advanced by Davis Drilling (Davis) to a maximum depth of 15.9 metres below ground surface (m bgs). A summary of the boreholes advanced by EXP is provided in Table 2. Please note that not all fourteen (14) of the boreholes were sampled as part of the Phase Two ESA. Soil samples were selected to provide adequate coverage of the APECs identified in the Phase One ESA (EXP, 2024), as shown in Table 3.

EXP continuously monitored the drilling activities to record the physical characteristics of the soil, depth of soil sample collection and total depth of boreholes (see borehole logs in Appendix C).

Representative soil samples were recovered in the overburden of the boreholes at regular intervals and submitted for chemical analysis (refer to Table 3).

5.4 Soil Sampling

The soil sampling conducted during the completion of this Phase Two ESA was undertaken in accordance with the SAAP to ensure that soil quality in the APECs identified in the Phase One ESA was characterized in accordance with O.Reg.153/04.

Soil samples for geologic characterization and chemical analysis were collected on a continuous basis in the overburden materials using sampling equipment advanced into the subsurface. Upon retrieval from the boreholes, the steel split spoons were placed on a flat surface and disassembled by drilling personnel to provide access of the recovered cores. Geologic and sampling details of the recovered cores were logged and assessed for the potential presence of non-aqueous phase liquids. Soil stratigraphy encountered in the boreholes were texturally, visually, and olfactory classified in the field and in the laboratory. Soil samples were logged for colour, grain size, moisture content, density, structures, texture, staining, and field vapour readings.

A portion of each soil sample was placed in a sealed ziplock plastic bag and allowed to reach ambient temperature prior to field screening with a RKI Eagle 2 (RKI) calibrated to hexane and isobutylene. The field screening measurements were made by

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inserting the instrument's probe into the plastic bag while manipulating the sample to ensure volatilization of the soil gases. These 'headspace' readings provide a real-time indication of the relative concentration of organic vapours encountered in the subsurface during drilling and are used to aid in the assessment of the vertical and horizontal extent of potential impacts and the selection of soil samples for analysis. The readings measured from the headspace of the soil samples collected during this Phase Two ESA, in parts per million (ppm) are provided on the borehole logs in Appendix C.

Measures were taken in the field and during transport to preserve sample integrity prior to chemical analysis. Recommended volumes of soil samples selected for chemical analysis were collected from the recovered cores into pre-cleaned, laboratory-supplied glass sample jars/vials identified for the specified analytical test group. Samples intended for PHC fractions F1 and VOCs were collected using a laboratory-supplied soil core sampler, placed into the vials containing methanol for preservation purposes and sealed using Teflon lined lids.

Soil samples selected for laboratory analysis were placed in clean coolers containing ice prior to and during transportation to the subcontract laboratory, AGAT Laboratories (AGAT Labs) of Mississauga, Ontario. The samples were transported/submitted within the acceptable holding time to AGAT Labs following Chain of Custody protocols for chemical analysis.

Decontamination and other protocols were followed during sample collection and handling to minimize the potential for sample cross-contamination. New disposable nitrile gloves were used for the handling and sampling of each retrieved soil core. The sampling equipment was decontaminated between borehole locations by the drilling contractor using a potable water/phosphate-free detergent solution followed by rinses with potable water and de-ionized water. Wash and rinse waters were collected in sealed, labeled containers. Drill cuttings were placed in labeled, sealed drums upon completion of sampling.

Soil samples submitted for specific chemical analysis were selected on the basis of visual inspection of the recovered cores, TOV readings, sample location and/or depth interval. The rationale for soil sample submission is presented in Table 3.

5.7 Groundwater Level Measurements

As part of the Hydrogeological Investigation, static water levels in the monitoring wells installed outside of the existing building were recorded on February 4, 2024. The groundwater elevation recorded in the shallow wells installed in the native material ranged from 167.74 masl to 172.30 masl. The groundwater elevation recorded in the deep wells installed in the shale bedrock ranged from 168.90 masl to 170.42 masl.

5.9 Groundwater Sampling

Given that the Phase One ESA did not identify any potential contaminants of concern (pCOC) in groundwater, groundwater sampling was not undertaken during the Phase Two ESA.

5.10 Sediment Sampling

As no water body was present on the development lands, sediment sampling was not undertaken during the Phase Two ESA.

5.11 Analytical Testing

All laboratory analyses were completed by AGAT Laboratories (AGAT), an accredited laboratory located in Mississauga, Ontario. AGAT performed the work following formal written methods and procedures. These methods include all the minimum requirements as specified in the document entitled Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act (March 9, 2004, amended as of July 1, 2011).



5.12 Residue Management Procedures

The residue materials produced during the borehole drilling soil sampling programs and monitoring well sampling programs comprised of soil cuttings from drilling activities, decontamination fluids from equipment cleaning, and waters from well development and purging (from hydrogeological investigation). All soil cuttings were stored in drums on-Site for future disposal. All development and purged water was collected and stored on-Site in labeled, sealed containers for future disposal.

5.13 Elevation Survey

An elevation survey was conducted during the Phase Two ESA investigative activities, with the purpose of obtaining relative vertical control of the monitoring well locations. The top of pipe and ground surface elevations of each monitoring well were surveyed relative to a geodetic benchmark.

5.14 Quality Assurance and Quality Control Measures

Quality Control/Quality Assurance (QA/QC) measures, as set out in the Sampling and Analysis Plan, were implemented during sample collection, storage and transport to provide accurate data representative of conditions in the surficial fill and upper overburden soils and the water table aquifer. The QA/QC measures included decontamination procedures to minimize the potential for sample cross contamination, the execution of standard operating procedures to collect representative and unbiased samples, the collection of quality control samples to evaluate sample precision and accuracy, and the implementation of measures to preserve sample integrity.

Soil duplicate samples, including one (1) duplicate sample for every ten (10) samples analyzed, were not obtained as part of the Phase Two ESA. However, EXP advises that duplicates be obtained prior to the RSC submission, as required by O.Reg. 153/04.

There were no significant deviations from the SAAP.



6 Review and Evaluation

6.1 Geology

The current soil investigation conducted at the Site consisted of the advancement of fourteen (14) boreholes into the surficial fill material and the underlying native materials to a maximum depth of 15.9 m bgs. Boundaries of soil indicated on the log sheets are intended to reflect transition zones for the purpose of environmental assessment and should not be interpreted as exact planes of geological change.

The general stratigraphy at the Site, as observed in the boreholes, consists of topsoil or asphalt underlain by sand and gravel. Fill material was observed beneath the topsoil and sand and gravel. Fill material generally consisted of clayey silt and/or silty sand which overlies native soil. Native material at the Site generally consists of silt, sandy silt, and/or silt till. Shale bedrock of the Queenston Formation was encountered below the silt till at all borehole locations. The approximate elevation for the bedrock encountered in each borehole is presented in the individual borehole and core logs. A brief description of the soil stratigraphy at the Site, in order of depth, is summarized in the following sections. Refer to borehole logs provided in Appendix C for details of soil stratigraphy. The interpreted Site geology is also shown on the enclosed cross section.

6.1.1. Surface Material

Asphalt with thickness ranging from about 70 to 140 mm was encountered at the surface of Boreholes 201, 202, 203, 204, 205, 206 and 209D.

At the ground surface of Boreholes 207, 210, 211, 212, 213 and 214D, topsoil cover with thickness ranging from about 100 to 200 mm was encountered.

At Borehole 208, a topsoil layer of about 125 mm thick was encountered below the surficial pebble layer.

6.1.2. Fill Material

Fill, comprising sand and gravel, silty sand and clayey silt was encountered below the asphalt or topsoil at all borehole locations. The fill extends to depths ranging from about 0.7 to 2 m below existing ground surface (El. ~176.1 to 173.3 m).

6.1.3. Native Soils

The fill in Borehole 205 is underlain by a silt deposit which is brown to grey in colour, contains a trace of clay and gravel. It is in a dense to very dense state of compactness (recorded 'N'-values of 41 to over 100) and extends to a depth of about 8.5 m below existing grade (El. ~166.8 m).

A sandy silt to silty sand deposit was encountered below the fill in Borehole 206. This deposit is brown in colour and is in a compact state of compactness. The sandy silt to silty sand extends to a depth of about 2.5 m below existing ground surface (El. \sim 173.5 m).

A silt till deposit was encountered below the silt in Borehole 205, below the sandy silt to silty sand in Borehole 206 and below the fill in the remaining boreholes. The silt till is generally reddish brown in colour, contains some clay, a trace of sand with a trace of shale fragments at lower level of the deposit. It has moisture contents of about 5 to 11 percent of dry mass and is in a dense to very dense state of compactness (recorded 'N'-values of 38 to over 100). The silt till extends to depths of about 2.6 to 10.1 m below existing ground surface (El. ~174.2 to 165.2 m).

6.1.4 Bedrock

Shale bedrock with some interbedded sandstone and limestone layers were encountered at all the borehole locations. The top of the shale bedrock was encountered between 2.3 and 10.1 mbgs across the boreholes 201 to 214.

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6.2 Groundwater: Elevations and Flow Direction

As previously mentioned, groundwater monitoring wells were not installed as part of the Phase Two ESA. They were however installed in support of the hydrogeological investigation that was completed concurrently. Based on the findings of the hydrogeological investigation the water levels indicated that the general groundwater flow direction is interpreted to be northeast and northwest of the Site for the shallow and deep water-bearing zones respectively, towards Mullet Creek, one of the tributaries of Credit River. This is consistent with the Oak Ridges Moraine Groundwater Program database. Groundwater levels are expected to show seasonal fluctuations and vary in response to prevailing climate conditions. This may also affect the direction and rate of flow.

It is noted that EXP will collect a current round of water levels from all accessible monitoring wells at the Site and that an updated groundwater contour plan will be prepared and included in the final submission of this Phase Two ESA.

6.2.1 Groundwater: Hydraulic Conductivity

Six (6) Single Well Response Tests (SWRT's) were completed on monitoring wells BH/MW 202, BH/MW 205, BH/MW 207, BH/MW 209D, BH/MW 212, and BH/MW 214D on February 4, 2024. The tests were completed to estimate the saturated hydraulic conductivity (K) of the soils at the well screen depths utilizing data loggers, preprogramed to take measurement on (time in sec/ half sec/minutes) intervals.

The static water level within each monitoring well was measured prior to the start of testing. In advance of performing SWRTs, each monitoring well underwent development to remove fines introduced into the screens following construction. The development process involved purging of the monitoring wells to induce the flow of fresh formation water through the screen. Each monitoring well was permitted to fully recover prior to performing SWRTs.

Hydraulic conductivity values were calculated from the SWRT and constant rate test data as per Hvorslev's solution included in the Aqtesolv Pro. V.4.5 software package. The highest K-value of the tested water-bearing zone is 1.3×10^{-6} m/s, and the geometric mean of the K-values is 2.6×10^{-7} m/s.

6.2.2 Groundwater: Horizontal Hydraulic Gradients

Based on the water levels collected as part of the hydrogeological investigation, it was noted that the horizontal hydraulic gradient across the Site is 0.0019 m/m to the north / northeast.

6.3 Soil Texture

Grain size analysis were carried out on seven (7) representative samples recovered from the boreholes. The test results are presented in Appendix A and summarized in Table 1 below:

Sample	Depth (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Description
BH203 SS2	0.8 – 1.4	0.0	3.7	83.8	12.5	Silt, some Clay, trace Sand
BH205 SS5	3.1 – 3.7	0.7	11.8	79.9	7.6	Silt, some Sand, trace Clay & Gravel
BH205 SS8	7.6 - 8.0	3.6	50.8	42.6	3.0	Sand & Silt, trace Clay & Gravel
BH207 SS3	1.5 – 2.1	0.0	4.8	68.0	27.2	Clayey Silt, trace Sand

Table 1: Summary of Grain Size Analysis Results



Sample	Depth (m)	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Description
BH211 SS2	0.8 - 1.4	0.0	2.4	73.1	24.5	Clayey Silt, trace Sand
BH212 SS4	2.3 – 2.8	0.0	6.5	78.8	14.7	Silt, some Clay, trace Sand
BH214 SS7	4.6 – 5.0	0.0	12.7	80.0	7.3	Silt, some Sand, trace Clay

Based on the 75 micron sieve of representative soil, six (6) of the seven (7) samples were identified as fine grained. As such the soil texture at the Site was determined to be medium and fine textured soils (refer to the 75 micron sieve analysis in Appendix D).

6.4 Soil Field Screening

TOV readings from each sample interval were measured for soil samples selected for BTEX/PHC and VOC analysis. Soil samples collected from previous environmental investigations were field screened in a similar manner.

Vapour concentrations readings collected during subsurface drilling were measured using the RKI Eagle 2 in ppm calibrated with hexane and isobutylene or equivalent. The vapour readings, in ppm, are provided on the borehole logs in Appendix C.

Soil samples submitted for chemical analysis were selected on the basis of visual inspection of the recovered cores, TOV readings, sample location and/or depth interval.

6.5 Soil Quality

In accordance with the scope of work, chemical analyses were performed on selected soil samples recovered from the boreholes. The selection of representative "worst case" soil samples was based on field screening, visual and/or olfactory evidence of impacts, and the presence of potential water bearing zones. A summary of the analytical results for the soil samples, including the location and depth of each sample, a comparison of concentrations against applicable SCS, and the identification of the potential contaminants of concern, are provided in Appendix D. Copies of the laboratory Certificates of Analysis for the analyzed soil samples are provided in Appendix E.

6.5.1 pH

The soil at the Site has a pH value between 5 and 9 for surficial soils; and, between 5 and 11 for subsurface soils.

6.5.2 Petroleum Hydrocarbons including Benzene, Toluene, Ethylbenzene and Xylenes (BTEX)

Six (6) soil samples were submitted for PHCs including BTEX analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 1 in Appendix D and shown in plan view on Figure 7.

All PHC including BTEX parameters were not detected above the laboratory Reporting Detection Limits (RDLs) in all samples submitted. The laboratory RDLs were below the Table 3 SCS.

6.5.3 Metals, As, Sb, Se, Hg, CN-, Cr(VI)

Six (6) soil samples were submitted for metals, As, Sb, Se, Hg, CN-, and Cr(VI) analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 4 in Appendix D and shown in plan view on Figure 10.

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The concentrations of metal and other regulated parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory RDLs.

The laboratory RDLs are below the Table 3 SCS.

6.5.4 Electrical Conductivity and Sodium Adsorption Ratio

Six (6) soil samples were submitted for EC and SAR analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 4 in Appendix D and shown in plan view on Figure 11.

The concentrations of EC and SAR parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory reportable detection limits (RDLs) with the following exceptions:

BH202-SS2 (SAR), BH205-SS2 (EC and SAR), BH205-SS6 (EC and SAR), BH209-SS2 (EC and SAR), BH203-SS2 (EC and SAR), and BH211-SS2 (EC).

The laboratory RDLs are below the Table 3 SCS.

It is anticipated that road salt has been applied across the paved areas of the Site, for the purposes of vehicular and pedestrian safety under conditions of snow and/or ice. As such, it is the QP_{ESA}'s opinion that the applicable Table 3 SCS for EC and SAR in soil were exceeded solely because salt was used on the road for the purpose of keeping the road safe for traffic under conditions of snow or ice or both, and therefore these parameters are not considered to exceed as per Section 49.1 (1) of O. Reg. 153/04.

6.5.5 Volatile Organic Compounds (VOCs)

Six (6) soil samples were submitted for VOC analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 2 in Appendix D and shown in plan view on Figure 8.

All VOC parameters were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.

6.5.6 Polycyclic Aromatic Hydrocarbons (PAHs)

Six (6) soil samples were submitted for PAH analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 3 in Appendix D and shown in plan view on Figure 9.

All PAH parameters were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.

6.5.7 Polychlorinated Bi-Phenols (PCBs)

One (1) soil sample was submitted for PCB analysis. The results of the analysis together with the applicable Table 3 SCS presented in Table 5 in Appendix D and shown in plan view on Figure 12.

PCBs were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.

6.5.8 Chemical Transformation and Soil Contaminant Sources

No soil exceedances of the applicable Table 3 SCS were identified at the Site (see section 6.5.4 for further explanation). Given that no contaminants have been identified in soil, no chemical transformations or contaminant sources have been considered.

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6.5.9 Evidence of Non-Aqueous Phase Liquid

Inspection of the soil cores retrieved from the boreholes did not indicate the presence of non-aqueous phase liquid (NAPL), staining. No hydrocarbon sheen was observed at the time of the Phase Two ESA.

6.6 Groundwater Quality

Groundwater sampling was not conducted as part of the Phase Two ESA as it was not identified as a media of concern in the Phase One ESA.

6.7 Sediment Quality

Sediment sampling was not considered as part of the Phase Two ESA scope as sediment was not identified as a media of concern in the Phase One ESA. Furthermore, no water bodies are located on-Site.

6.8 Quality Assurance and Quality Control Measures

Quality assurance and quality control measures were taken during the field activities to meet the objectives of the sampling and quality assurance plan to collect unbiased and representative samples to characterize existing conditions in the overburden at the Site.

Review of field activity documentation indicated that recommended sample volumes were collected from soil for each analytical test group into appropriate containers and preserved with proper chemical reagents in accordance with the protocols set out in the "Protocol for Analytical Methods used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act" (MECP, 2004). Samples were preserved at the required temperatures in pre-chilled insulated coolers and met applicable holding time requirements, when relinquished to the receiving laboratory.

Please note that field QA/QC samples were not collected during this Phase Two ESA. However, duplicate samples will be required at a frequency of one (1) per ten (10) samples, per analyte, prior to the RSC submission. The field duplicate sample results will be quantitatively evaluated by calculating the relative percent difference (RPD).

The contractual laboratory selected to perform the chemical analyses was AGAT Labs, of Mississauga, ON. AGAT Labs is an accredited laboratory under the Standards Council of Canada/Canadian Association of Laboratory Accreditation in accordance with ISO/IEC 17025:2005 – "General Requirements for the Competence of Testing and Calibration Laboratories". Certificates of Analysis were received from AGAT Labs reporting the results of all the chemical analyses performed on the submitted soil and groundwater samples. Copies of the Certificates of Analysis are provided in Appendix E. Review of the Certificates of Analysis, prepared by AGAT Labs, indicates that they were in compliance with the requirements set out under subsection 47(3) of O. Reg. 153/04.

The analytical program conducted by AGAT Labs included analytical test group specific QA/QC measures to evaluate the accuracy and precision of the analytical results and the efficiency of analyte recovery during solute extraction procedures. The laboratory QA/QC program consisted of the preparation and analysis of laboratory duplicate samples to assess precision and sample homogeneity, method blanks to assess analytical bias, spiked blanks and QC standards to evaluate analyte recovery, matrix spikes to evaluate matrix interferences and surrogate compound recoveries (VOCs only) to evaluate extraction efficiency. The laboratory QA/QC results are presented in the Quality Assurance Report provided in the Certificate of Analysis prepared by AGAT Labs. The QA/QC results are reported as percent recoveries for matrix spikes, spike blanks and QC standards, relative percent difference for laboratory duplicates and analyte concentrations for method blanks. The QA/QC results were assessed against test group control limits in the case of spiked blanks, matrix spikes and surrogate recoveries and alert criteria in the case of method blanks and laboratory duplicates. Review of the laboratory QA/QC results reported by BV Labs indicated that they were within acceptable control limits or below applicable alert criteria for the sampled media and analytical test groups. Based on the assessment of the QA/QC, the analytical results reported are of acceptable quality and data qualifications are not required.



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6.9 Phase Two Conceptual Site Model

A Phase Two Conceptual Site Model (CSM) provides a narrative, graphical and tabulated description integrating information related to the Site geologic and hydrogeologic conditions, areas of potential environmental concern/potential contaminating activities, the presence and distribution of potential contaminants of concern, contaminant fate and transport, and potential exposure pathways. The Phase Two CSM was completed in accordance with O. Reg.153/04 as defined by the MECP and is presented in Appendix F.

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7 Conclusions

The results and findings of the Phase Two ESA conducted by EXP at the Site are summarized as follows:

- A total of fourteen (14) boreholes were advanced during this time. Boreholes 201 to 214 were advanced by Davis Drilling (Davis) to a maximum depth of 15.9 metres below ground surface.
- The general stratigraphy at the Site, as observed in the boreholes, consists of topsoil or asphalt underlain by sand and gravel. Fill material was observed beneath the topsoil and sand and gravel. Fill material generally consisted of clayey silt and/or silty sand which overlies native soil. Native material at the Site generally consists of silt, sandy silt, and/or silt till. Shale bedrock of the Queenston Formation was encountered below the silt till at all borehole locations.
- As part of the Hydrogeological Investigation completed in conjunction with the Phase Two ESA, static water levels in the monitoring wells installed outside of the existing building were recorded on February 4, 2024. The groundwater elevation recorded in the shallow wells ranged from 167.74 masl to 172.30 masl. The groundwater elevation recorded in the deep wells ranged from 168.90 masl to 170.42 masl.
- Soil samples were submitted for the analysis of PHCs, BTEX, VOCs, metals and inorganics, PAHs, and PCBs. Please note that not all fourteen (14) of the boreholes were sampled as part of the Phase Two ESA. Soil samples were selected to provide adequate coverage of the APECs identified in the Phase One ESA (EXP, 2024).
- Six (6) soil samples were submitted for PHCs including BTEX analysis. All PHC including BTEX parameters were not detected above the laboratory RDLs in all samples submitted.
- Six (6) soil samples were submitted for VOC analysis. All VOC parameters were not detected above the laboratory RDLs in all samples submitted.
- Six (6) soil samples were submitted for metals, As, Sb, Se, Hg, CN-, and Cr(VI) analysis. The concentrations of metal and other regulated parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory RDLs.
- Six (6) soil samples were submitted for PAH analysis. All PAH parameters were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.
- One (1) soil sample was submitted for PCB analysis. PCBs were not detected above the laboratory RDLs in all samples submitted. The laboratory RDLs were below the Table 3 SCS.
- Six (6) soil samples were submitted for EC and SAR analysis. The concentrations of EC and SAR parameters in the analyzed soil samples were either below the MECP (2011) Table 3 SCS or not detected at the laboratory reportable detection limits (RDLs) with the following exceptions: BH202-SS2 (SAR), BH205-SS2 (EC and SAR), BH205-SS6 (EC and SAR), BH209-SS2 (EC and SAR), BH203-SS2 (EC and SAR), and BH211-SS2 (EC). The laboratory RDLs are below the Table 3 SCS.
- No evidence of free product (i.e. visible film or sheen), or odour was observed during soil sampling.

It is anticipated that road salt has been applied across the paved areas of the Site, for the purposes of vehicular and pedestrian safety under conditions of snow and/or ice. As such, it is the QP_{ESA} 's opinion that the applicable Table 3 SCS for EC and SAR in soil were exceeded solely because salt was used on the road for the purpose of keeping the road safe for traffic under conditions of snow or ice or both, and therefore these parameters are not considered to exceed as per Section 49.1 (1) of O. Reg. 153/04.

Based on the findings of the Phase Two ESA, it is the opinion of the QP that no additional environmental investigations are required, at this time. An RSC can be filed for the Site for the proposed land use change.

8 General Limitations

The information presented in this report is based on a limited investigation designed to provide information to support an assessment of the current environmental conditions within the subject property. The conclusions and recommendations presented in this report reflect Site conditions existing at the time of the investigation.

More specific information with respect to the conditions between samples, or the lateral and vertical extent of materials may become apparent during excavation operations. The interpretation of the borehole information must, therefore, be validated during any such excavation operations. Consequently, during the future development of the property, conditions not observed during this investigation may become apparent. Should this occur, EXP Services Inc. should be contacted to assess the situation, and the need for additional testing and reporting. EXP has qualified personnel to provide assistance in regards to any future geotechnical and environmental issues related to this property.

The environmental investigation was carried out to address the intent of applicable provincial Regulations, Guidelines, Policies, Standards, Protocols and Objectives administered by the Ministry of the Environment and Climate Change. It should also be noted that current environmental Regulations, Guidelines, Policies, Standards, Protocols and Objectives are subject to change, and such changes, when put into effect, could alter the conclusions and recommendations noted throughout this report. Achieving the study objectives stated in this report has required us to arrive at conclusions based upon the best information presently known to us. No investigative method can completely eliminate the possibility of obtaining partially imprecise or incomplete information; it can only reduce the possibility to an acceptable level. Professional judgment was exercised in gathering and analyzing information obtained and in the formulation of the conclusions. Like all professional persons rendering advice, we do not act as absolute insurers of the conclusions we reach, but we commit ourselves to care and competence in reaching those conclusions.

Our undertaking at EXP, therefore, is to perform our work within limits prescribed by our clients, with the usual thoroughness and competence of the engineering profession. It is intended that the outcome of this investigation assist in reducing the client's risk associated with environmental impairment. Our work should not be considered 'risk mitigation'. No other warranty or representation, either expressed or implied, is included or intended in this report.

This report was prepared for the exclusive use of EMTC Holdings Inc. c/o Muzzo Group of Companies and may not be reproduced in whole or in part, without the prior written consent of EXP, or used or relied upon in whole or in part by other parties for any purposes whatsoever. Any use which a third party makes of this report, or any part thereof, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. EXP Services Inc. accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.



9 Closure

We trust this report is satisfactory for your purposes. Should you have any questions, please do not hesitate to contact this office.

Yours truly,

EXP Services Inc.

H. 6

Alessandro Girardo, C.E.T. Project Manager Environmental Services

Amanda Catenaro, P.Geo., QP_{ESA} Senior Project Manager Environmental Services



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10 References

This study was conducted in general accordance with the applicable Regulations, Guidelines, Policies, Standards, Protocols and Objectives administered by the Ministry of the Environment. Specific reference is made to the following:

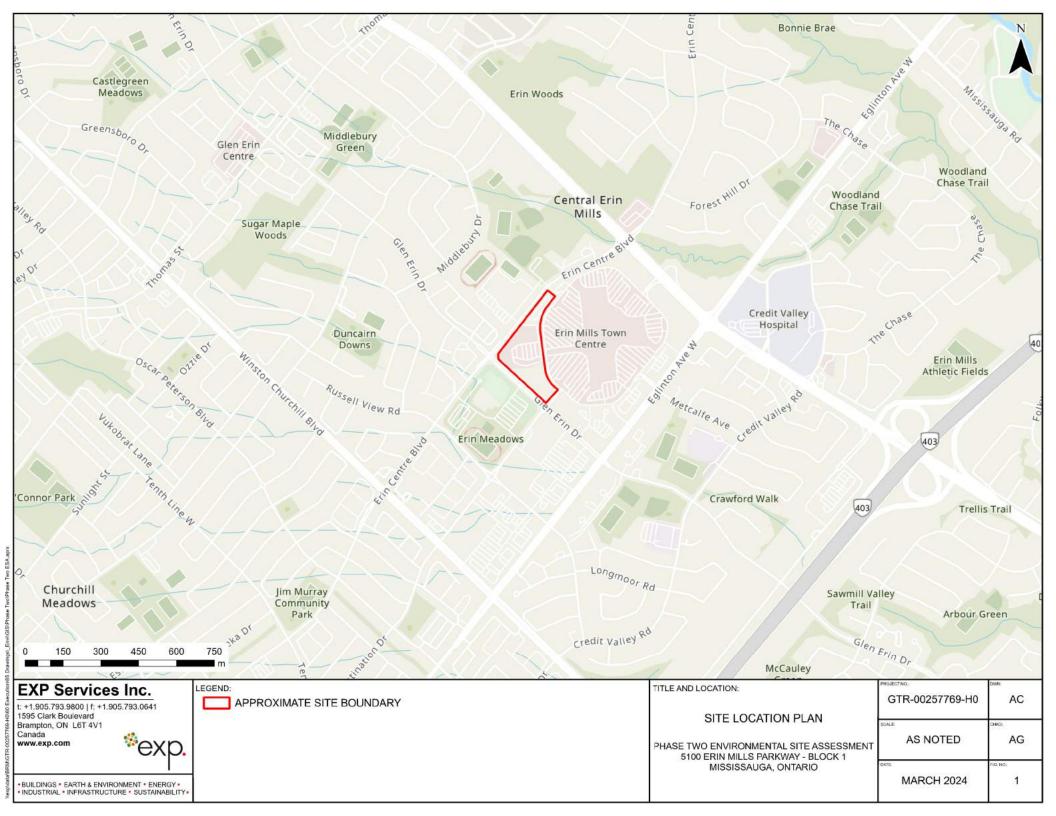
- Canadian Standards Association [CSA] (2000) Z769-00, Phase II Environmental Site Assessment. Canadian Standards Association, March 2000.
- Environmental Protection Act, R.S.O. 1990, Chapter E.19, as amended, September 2004.
- Ministry of the Environment [MECP] (1996) Guidance on Sampling and Analytical Methods for Use at Contaminated Sites in Ontario. Ontario Ministry of the Environment, December 1996.
- MECP (2011a) Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act. Ontario Ministry of the Environment, March 2004, amended as of July 1, 2011.
- MECP (2011) Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act. Ontario Ministry of the Environment, April 15, 20101.
- Occupational Health and Safety Act Ministry of Labour (MOL).
- Ontario Regulation 153/04, made under the Environmental Protection Act, May 2004, amended.
- Ontario Water Resources Act R.R.O. 1990, Regulation 903, amended.

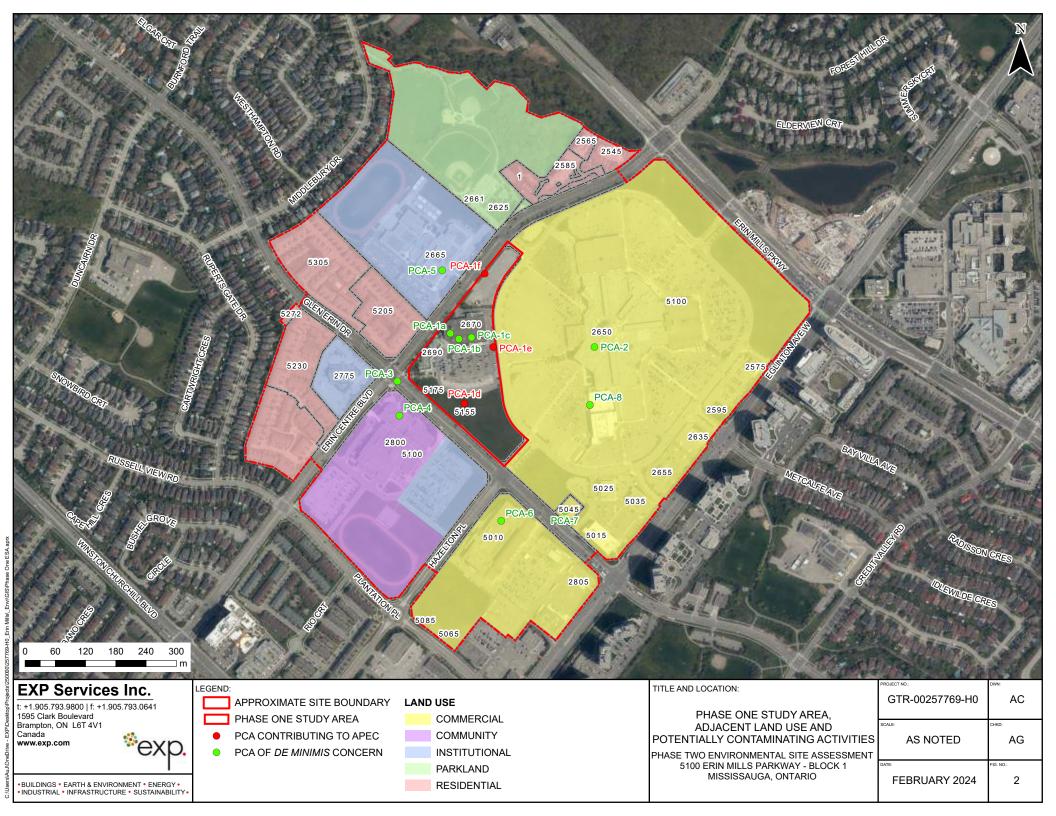
EXP Services Inc.

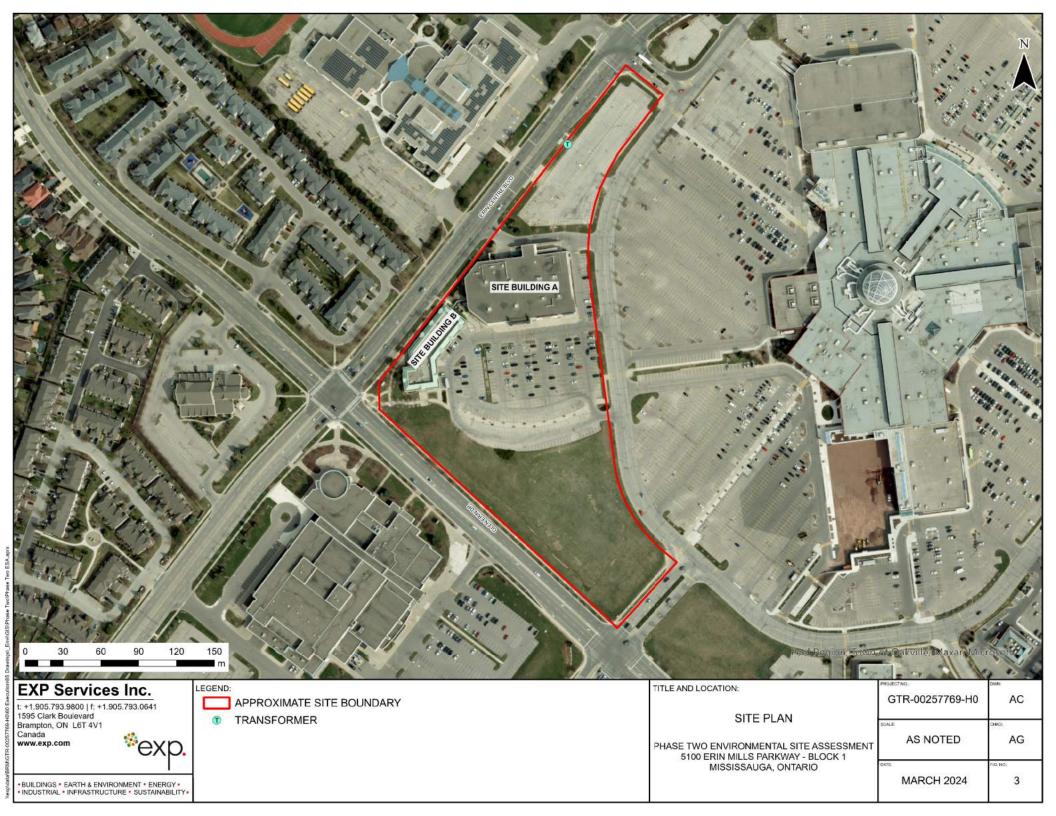
Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

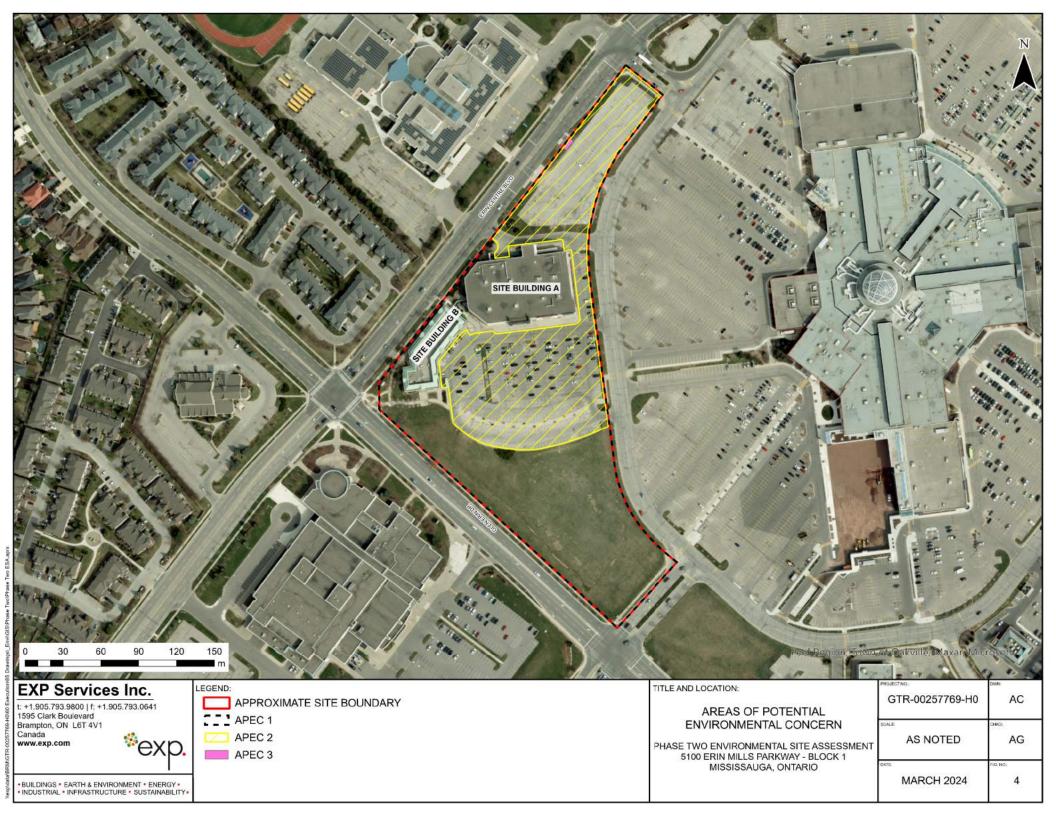
Figures

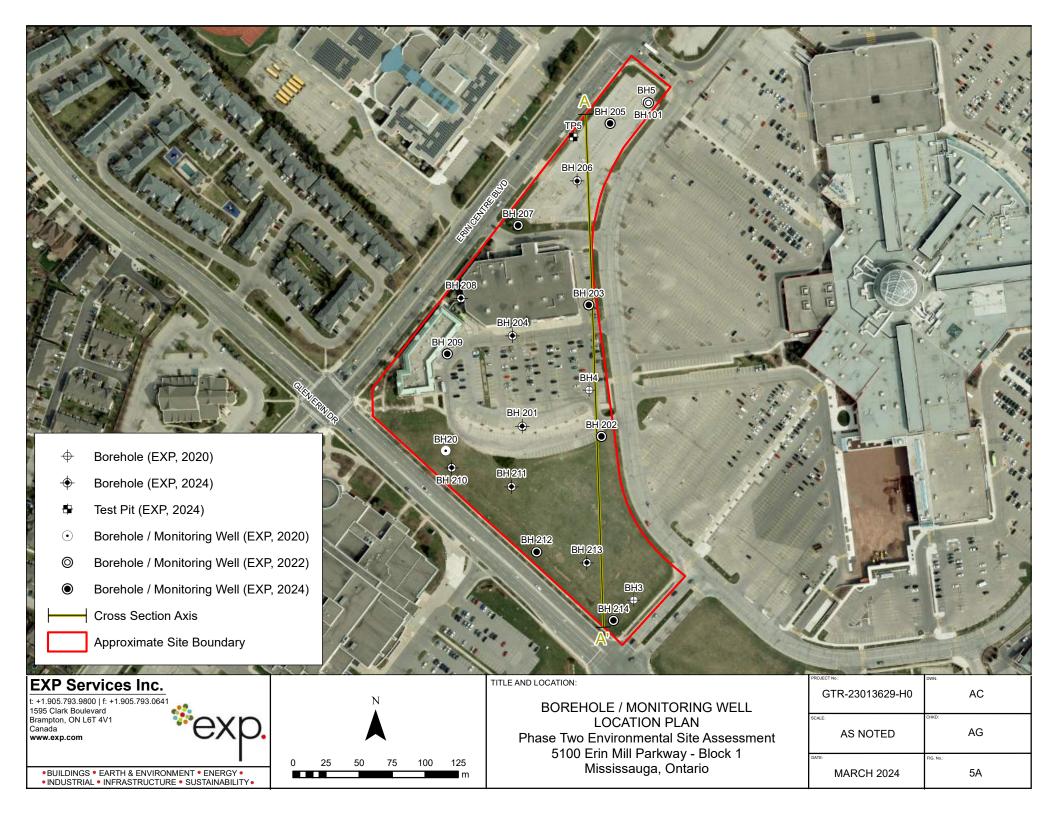


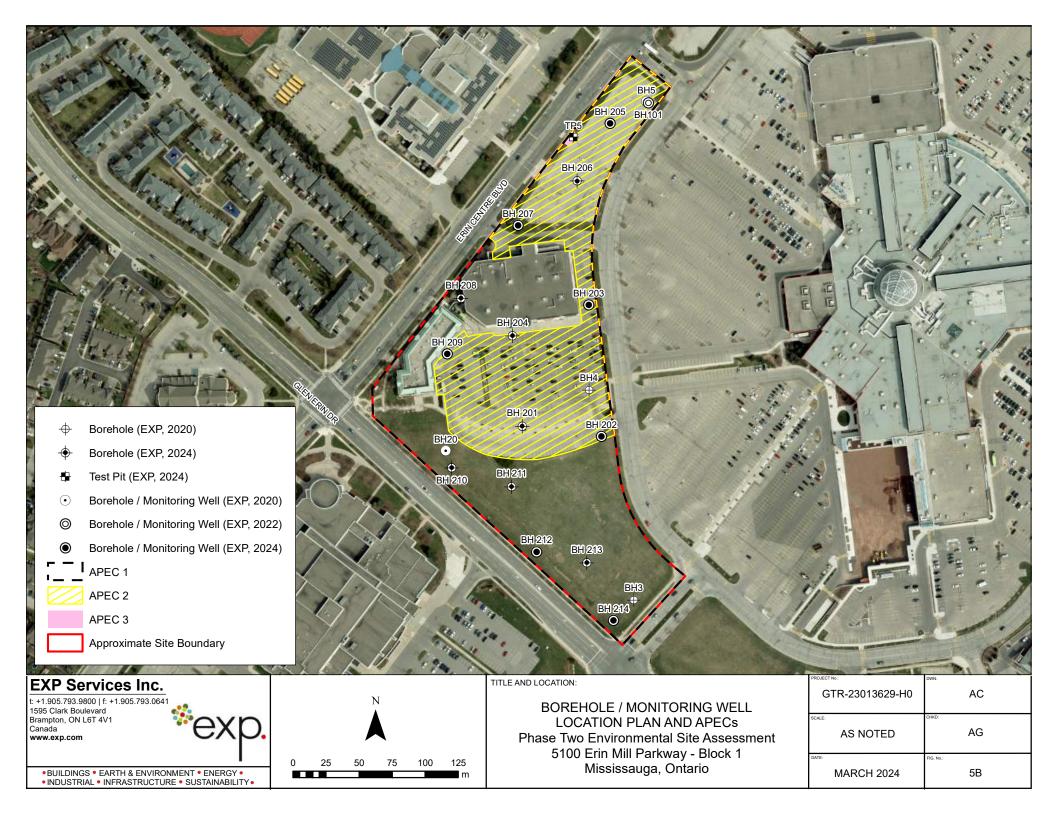


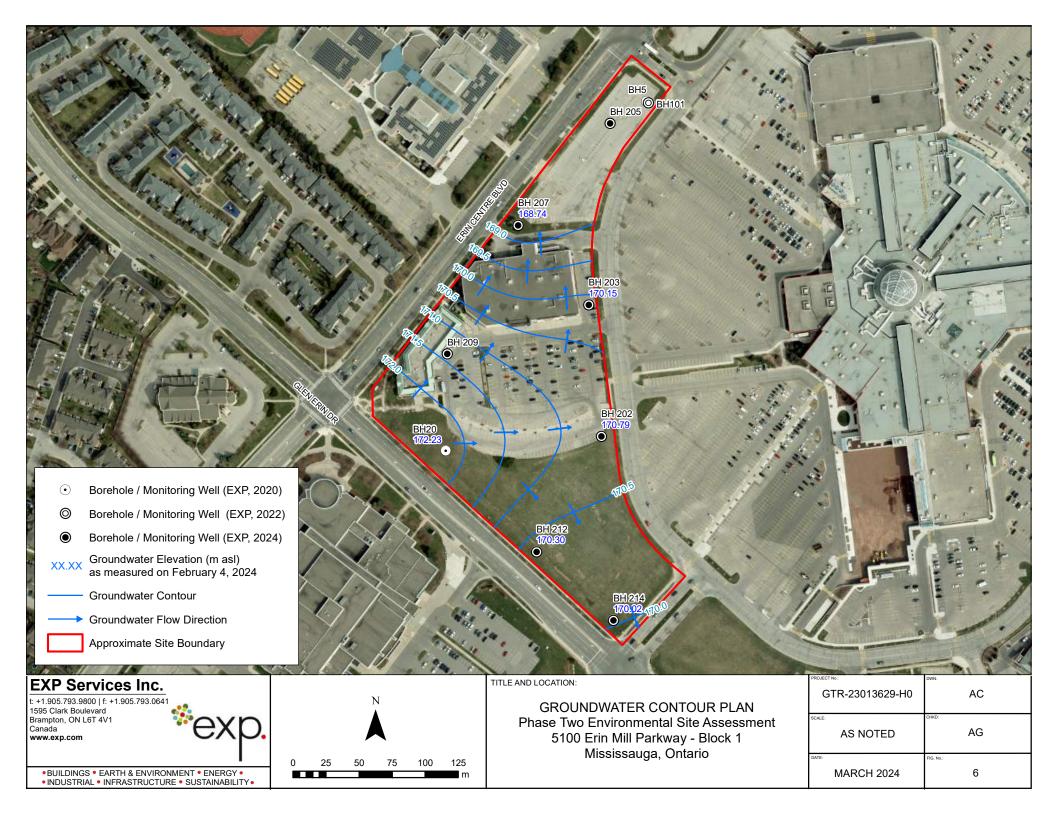


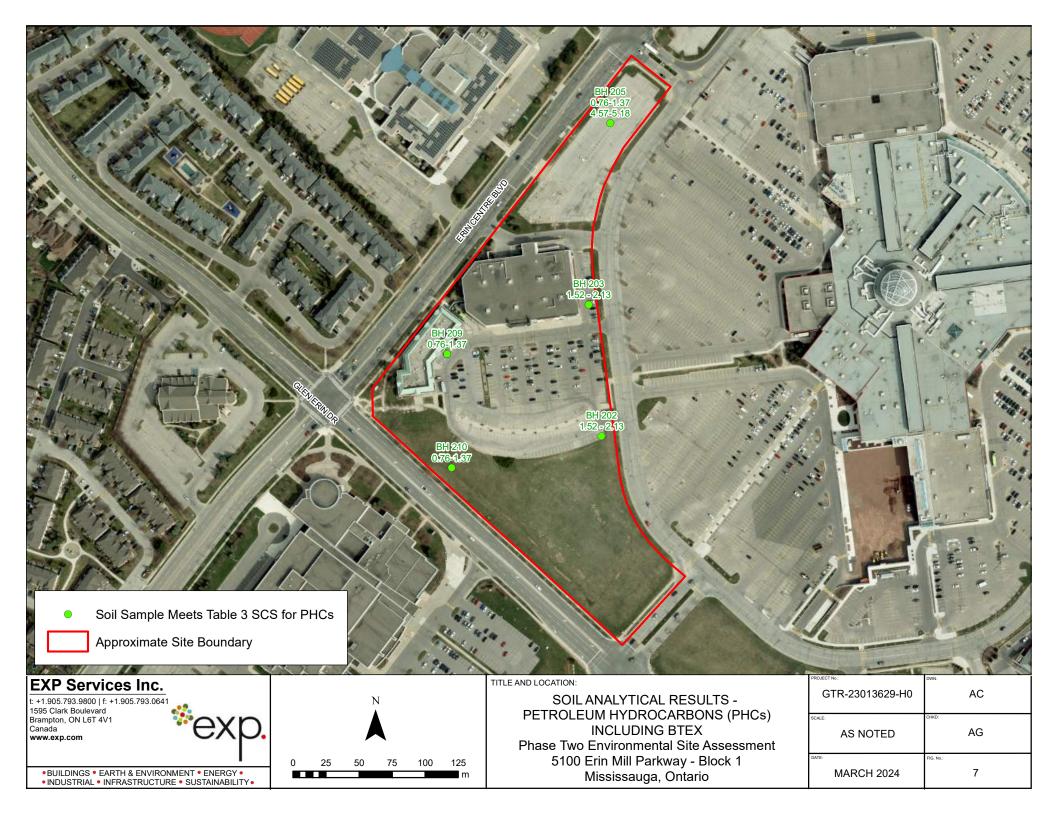


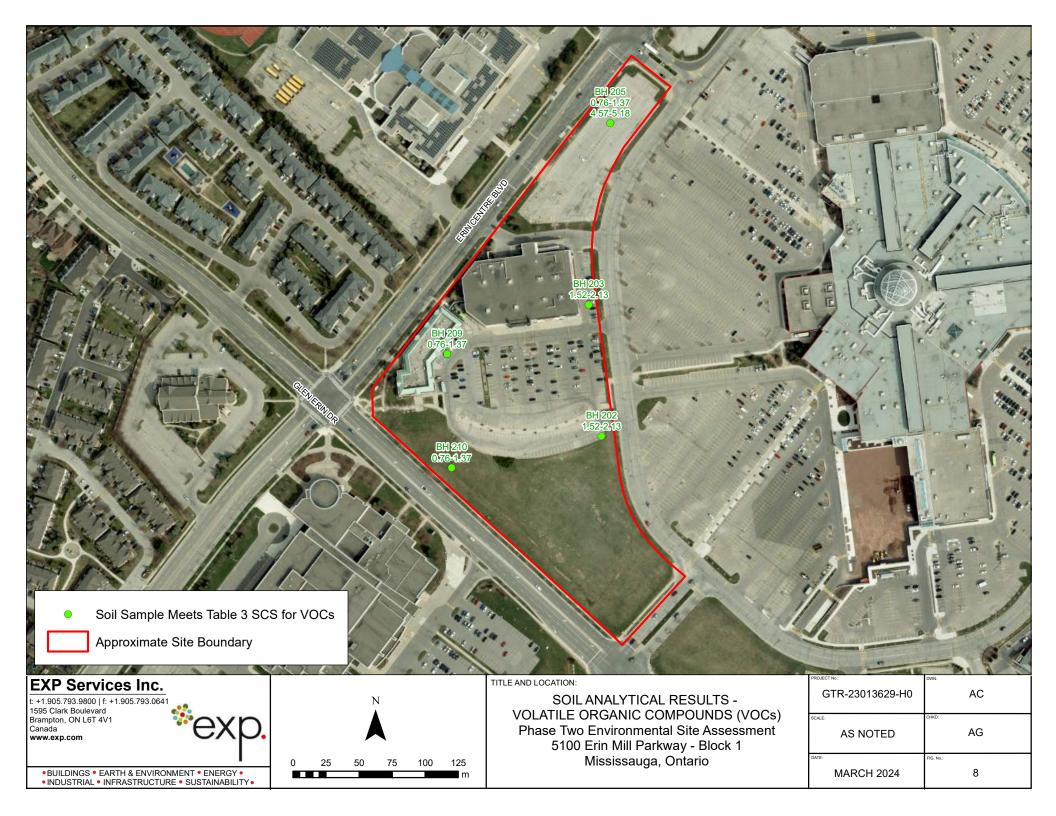


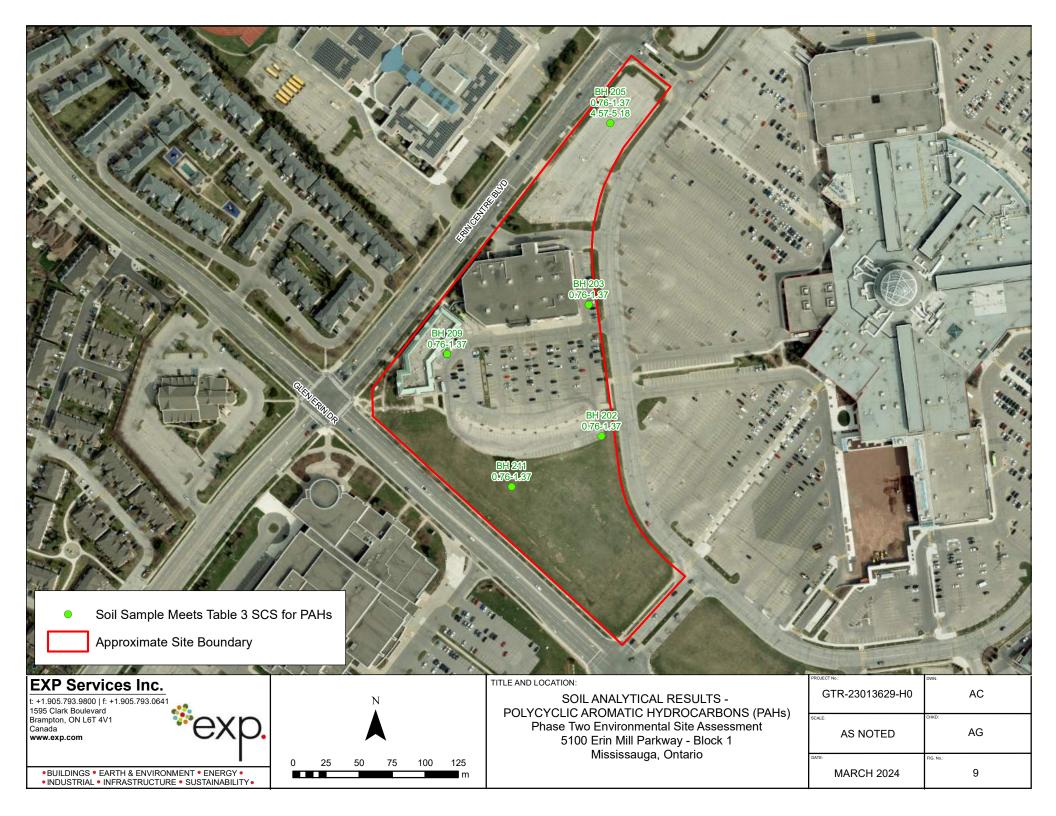


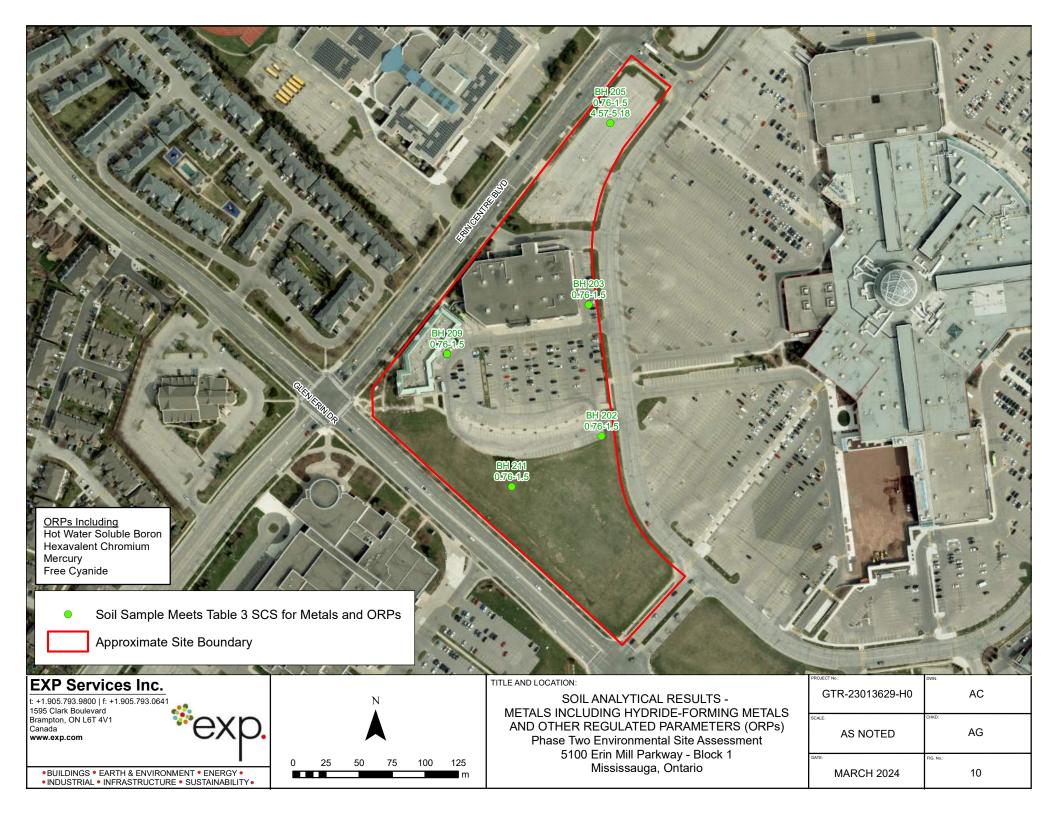


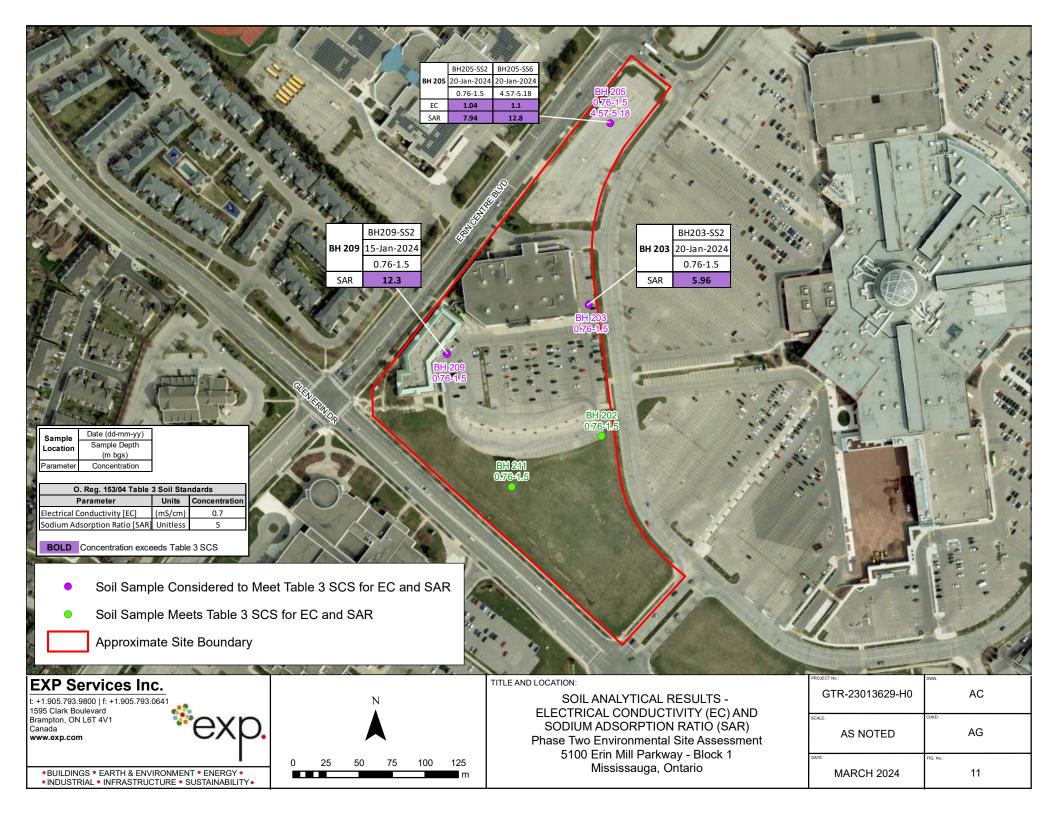


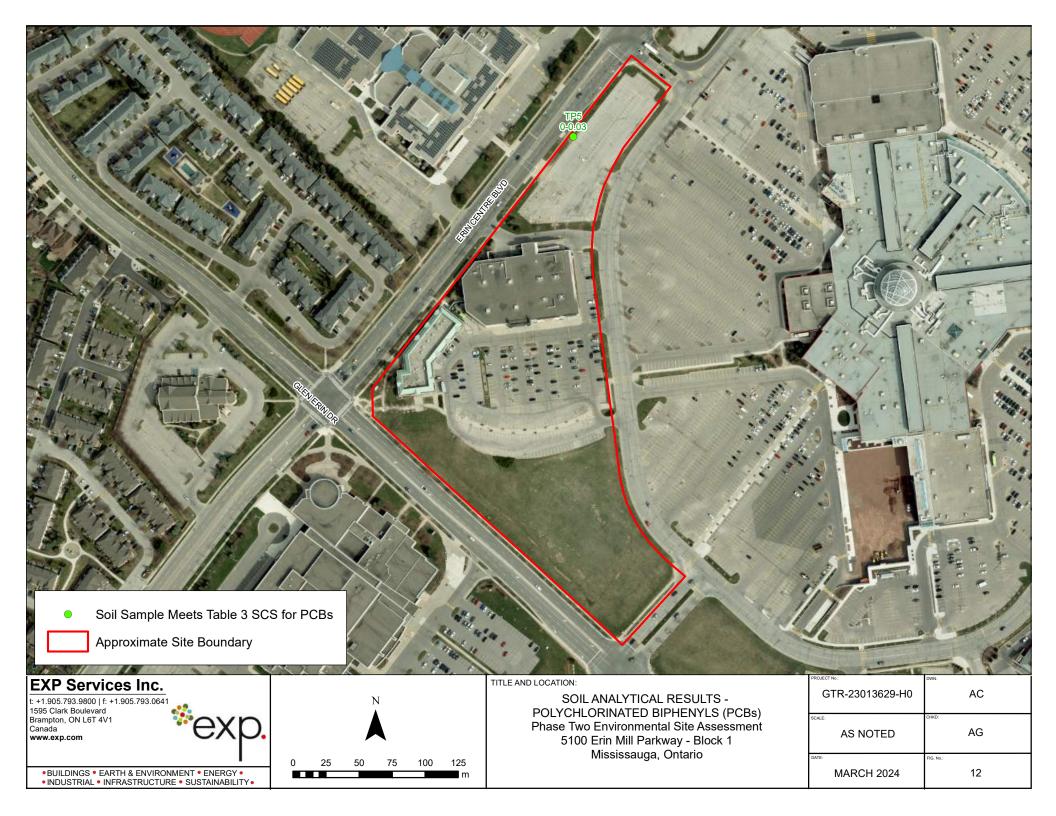












Α' А BH 201 EL: 176.84 BH 202 EL: 176.76 BH 211 EL: 176.86 SOUTH NORTH BH 204 EL: 176.51 BH 203 EL: 176.31 / BH 212 EL: 176.12 BH 213 EL: 176.15 178-3 BH 207 EL: 176.12 BH 206 EL: 176.02 BH 214D EL: 175.97 BH 214S EL: 175.97 BH 205 EL: 176.25 177 176-175--175 174-174 -173 170.02 **-**173 172· -172 -171 171 -170.30 170.15 170--170 169.70 169.69 169 169-168.74 -168-168 167-167 166-166 165--165 164-164 163-163 E 162--162 E سليسلس B 161 161 Ħ F 160-160 HORIZONTAL SCALE: VERTICAL SCALE: AS SHOWN 25 75 25 50 TITLE AND LOCATION: ROJECT NO .: DWN.: LEGEND: EXP Services Inc. t: +1.905.793.9800 | f: +1.905.793.0641 1595 Clark Boulevard FILL GROUNDWATER ELEVATION ON FEBRUARY 4, 2024 GTR-00257769-H0 NS ▼ Brampton, ON L6T 4V1 SILT TILL exp **CROSS SECTION A-A'** SCALE: CK: Canada 5100 ERIN MILLS PARKWAY - BLOCK 1, _ _ -SHALE BEDROCK AS NOTED FC www.exp.com MISSISSAUGA, ONTARIO DATE: FIG. NO.: BUILDINGS • EARTH & ENVIRONMENT • ENERGY
 INDUSTRIAL • INFRASTRUCTURE • SUSTAINABILITY

MARCH 19, 2024

13

Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

Tables



		- Areas of Potential Environm	•	•	
)0257769-H0 - Phase Two Environı 70 & 2690 Erin Centre Boulevard, N			
	207	o & 2000 Enni Centre Doulevara, N	nississuugu, Onturio	,	
Area of Potential Environmental Concern (APEC) ⁽¹⁾	Location of APEC on Phase One Property	Potentially Contaminating Activity (PCA) ⁽²⁾	Location of PCA (on-Site or off- Site)	Contaminants of Potential Concern ⁽³⁾	Media Potentially Impacted (Groundwater, soil and/or sediment)
APEC 1: Entire Site (PCA- 1d)	Entire Site	PCA#30-Importation of Fill Material of Unknown Quality.	On-Site	PHCs, VOCs, PAHs, Metals, Inorganics	Soil
APEC 2: Seasonal application of de-icing salt agents (PCA-1e)	Central and northeastern portion of the Site - Parking lot	PCA#48- Salt Manufacturing, Processing and Bulk Storage	On-Site	pH, EC, SAR	Soil
APEC 3: Transformers on Site (PCA-1f)	North portion of the Site	PCA#55-Transformer Manufacturing, Processing and Use.	On-Ste	PCBs	Soil
or more contaminants are poten (a) identification of post or prese (b) identification of potentially co	tially present, as determined throug ent uses on, in or under the phase o ontaminating activities. vity means a use or activity set out i	r under a phase one study area where on gh the P One ESA, including through, ne property, and in Column A of Table 2 of Schedule D	e		
	the Assessment of Properties under	tial concern using the Method Groups as r Part XV.1 of the Environmental Protectio	on Act,		
	PCBs	PCBs	Metals	Electrical Conductivity	SAR
		PAHs		Cr (VI)	
•		THMs		Hg	
Dioxins/Furans, PCDDs/PCI		VOCs		Methyl Mercury	
	BTEX	BTEX		high pH	
PHCs	Ca, Mg	Ca, Mg	CN-	low pH	

4. When submitting a record of site condition for filing, a copy of this table must be attached en français. Pour obtenir de l'aide en francais, veuillez communiquer avec le ministère de l'Environnement au 1-800-461-6290

TABLE 2 - Borehole Log Information

GTR-00257769-H0 - Phase Two Environmental Site Assessment 2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Location ID	Ground Elevation (m)	Depth of BH (m bgs)	Bottom Elevation (m bgs)	Date Drilled	Drilling Contractor
BH201	176.84	15.44	161.40	Jan 19-22, 2024	Davis Drilling
BH202	176.76	15.46	161.30	Jan 17-18, 2024	Davis Drilling
BH203	176.31	15.61	160.70	Jan 19-22, 2024	Davis Drilling
BH204	176.51	15.71	160.80	Jan 19-22, 2024	Davis Drilling
BH205	175.26	15.46	159.80	Jan 19-22, 2024	Davis Drilling
BH206	176.02	15.52	160.50	Jan 11, 2024	Davis Drilling
BH207	176.12	15.52	160.60	Jan 10-11, 2024	Davis Drilling
BH208	176.52	15.72	160.80	Jan 10, 2024	Davis Drilling
BH209	176.87	15.07	161.80	Jan 15, 2024	Davis Drilling
BH210	176.76	15.76	161.00	Jan 20, 2024	Davis Drilling
BH211	176.86	15.36	161.50	Jan 8, 2024	Davis Drilling
BH212	176.12	15.42	160.70	Jan 5, 2024	Davis Drilling
BH213	176.15	15.25	160.90	Jan 5, 2024	Davis Drilling
BH214	175.97	15.87	160.10	Jan 4, 2024	Davis Drilling

Elevations are based on the geoodetic benchmark (monument no. 042100306), assigned with an elevation of 184.414 m asl

TABLE 3 - S	GTR-00257769-1	I Samples Submitted HO - Phase Two Environn Erin Centre Boulevard, N	
Soil Sample ID	Sample Depth Interval (m)	Rationale	Analysis
BH202-SS2	0.76-1.37	To assess potential presence of fill material at the site.	PAHs and Metals/Inorganics
BH202-SS3	1.52-2.13	General Site Characterization	PHCs and VOCs
BH203-SS2	0.76-1.37	To assess potential presence of fill material at the site.	PAHs and Metals/Inorganics
BH203-SS3	1.52-2.13	General Site Characterization	PHCs and VOCs
BH205-SS1	0.0-0.3	To Assess presence of transformer on-site.	PCBs
BH205-SS2	0.76-1.37	To assess potential presence of fill material at the site	PHCs, VOCs, PAHs and Metals/Inorganics

the site.

General Site

Characterization

To assess potential

presence of fill material at

the site. To assess potential

presence of fill material at

the site. To assess potential

presence of fill material at the site.

PHCs, VOCs, PAHs and

Metals/Inorganics

PHCs, VOCs, PAHs and

Metals/Inorganics

PHCs and VOCs

PAHs and Metals/Inorganics

VOCs - Volatile Organic Compounds

PAHs - Polyciclic Aromatic Hydrocarbons

PCBs - Polychlorinated Bi-Phenols

PHC - Petroleum Hydrocarbons

BTEX - Benzene, Toluene, Ethylbenzene and Xylenes

4.57-5.18

0.76-1.37

0.76-1.37

0.76-1.37

Metals - O.Reg 153 Metals

BH205-SS6

BH209-SS2

BH210-SS2

BH211-SS2

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Appendix A – Sampling and Analysis Plan





Phase Two Environmental Site Assessment

1. Introduction

This Appendix presents the Sampling and Analysis Plan (SAAP) that was developed in support of the Phase Two Environmental Assessment Work (ESA) for the property located at 2670 & 2690 Erin Center Boulevard, Mississauga, Ontario (hereinafter referred to as the 'Site'). The Phase Two ESA will be conducted to provide further characterization of the Site subsurface conditions and address the Areas of Potential Environmental Concerns (APECs) outlined in EXP's March 2024 Phase One ESA to support the subsequent filing of a Record of Site Condition (RSC) on the Ontario Ministry of the Environment, Conservation & Parks (MECP) Brownfields Environmental Site Registry, which will be required. The SAAP presents the procedures and measures that will be undertaken during field investigative activities to characterize the Site conditions and meet the data quality objectives of the Phase Two ESA.

The SAAP presents the sampling program proposed for the Site, the recommended procedures and protocols for sampling and related field activities, the data quality objectives, and the quality assurance/ quality control measures that will be undertaken to provide for the collection of accurate, reproducible and representative data.

2. Field Sampling Program

The field sampling program was developed to provide for the collection of samples of the surficial and subsurface soil materials for chemical analysis of petroleum hydrocarbons (PHCs), benzene, toluene, ethylbenzene and xylenes (collectively known as 'BTEX'), volatile organic compounds (VOCs), Metals, Polycyclic Aromatic Hydrocarbons (PAHs), Other Regulated Parameters (ORPs), and Polychlorinated Bi-Phenols (PCBs) in soil. The soil sampling media is to consist of the surface soils and upper overburden materials. The soil sampling will be location-specific to assess for the potential presence of PHCs, BTEX, VOCs, Metals, ORPs, PAHs and PCBs based on the identification of areas of potential environmental concern (APECs). Vapour readings will also be collected in the field to determine samples to be submitted for BTEX and PHC F1-F2 analysis. The soil sample intervals will extend from the surface up to a maximum depth of approximately 15 meters (m) below grade surface (bgs) or sample refusal.

Vertical control of the boreholes will be obtained through the completion of an elevation survey with reference to a local structure with a known geodetic elevation. Groundwater flow and direction in the water table aquifer will also be determined through groundwater level measurements and the elevations established from the Site elevation survey.

3. Field Methods

To meet the requirements of the field sampling program, the following field investigative methods will be undertaken:

- Borehole Drilling;
- Soil Sampling; and,
- Elevation Survey

The field investigative methods will be performed following the procedures and protocols set out in EXP's standard operating procedures and are outlined below:

2670 & 2690 Erin Center Boulevard, Mississauga, Ontario GTR-00257769-H0

3.1 Borehole Drilling

It is noted that the boreholes being completed for the purposes of this Phase Two ESA are also being completed to support a geotechnical and hydrogeological investigation at the site. Boreholes will be advanced at the Site to facilitate the collection of soil samples for chemical analysis and geologic characterization; and, for the installation of groundwater monitoring wells (for the hydrogeological investigation). A total of fourteen (14) boreholes are proposed to be advanced at the Site for the combined investigation, up to a maximum depth of approximately 15 m below grade, to provide for the collection of samples of the surficial and overburden materials beneath the Site. The borehole locations will be selected to determine the presence or absence of impacts in the soils and the upper overburden groundwater and to address the APECs outlined in EXP's Phase One ESA Report dated March 26, 2024.

Prior to borehole drilling, utility clearances will be obtained from public and private locators, as required. If any uncertainty regarding the location of a buried utility at a borehole location is encountered, hand augering or digging will be performed beforehand to confirm the location of the utility.

Where there is overlying asphalt or concrete, the overlying material will be mechanically cored to provide access to the underlying soil materials. The borehole drilling program will be conducted by a licensed driller under the oversight of EXP field staff. Auger flights will be cleaned prior to the commencement of drilling at each borehole location.

3.2 Soil Sampling

Soil samples will be collected for chemical analysis and geologic property characterization. The soil samples will be collected using 5 cm diameter, 61 cm long, split spoons and solid stem augers or a 5 cm diameter, 1.2 m long, duel tube sampling system with interior dedicated vinyl sampling tubes. Upon retrieval from the boreholes, the split spoons or vinyl sampling tubes will be placed on a flat surface and disassembled by drilling personnel to provide access of the recovered cores. Geologic and sampling details of the recovered cores will be logged and the samples will be assessed for the potential presence of non-aqueous phase liquids. Soil stratigraphy encountered in the boreholes will be texturally, visually and olfactory classified in the field and in the laboratory. Soil samples will be logged for colour, grain size, moisture content, density, structures, texture, staining, and field vapour readings. A Photo-ionization Detector (PID) or RKI Eagle 2 will be utilized to screen the soil samples for Total Organic Vapour (TOV). Representative worst-case soil samples from each borehole will be collected and submitted to a certified laboratory for analysis based on TOV readings, sample depth, visual and/or olfactory field observations.

Recommended volumes of soil samples selected for chemical analysis will be collected into pre-cleaned laboratory-supplied glass sample jars/vials identified for the specified analytical test group. Samples intended for PHC/BTEX and VOCs will be collected using a laboratory-supplied soil core sampler, placed into the vials containing methanol for preservation purposes and sealed using Teflon lined septa lids. The samples will be placed into clean insulated coolers chilled with ice for storage and transport. The samples will be assigned unique identification numbers, and the date, time, location, and requested analyses for each sample will be documented in a bound field note book. The samples will be submitted to the contractual laboratory within analytical test group holding times under Chain of Custody protocols. New disposable chemical resistant gloves will be used for each soil core to prevent sample cross-contamination.

3.3 Elevation Survey

An elevation survey will be conducted to obtain vertical control of the boreholes. The ground surface elevation of the monitoring well and borehole locations will be surveyed against an geodetic benchmark, or if unavailable, against a suitable arbitrary benchmark. Elevations measured against a geodetic/arbitrary benchmark will be recorded as meters above mean sea level (m AMSL). The elevation survey will be accurate to within ± 0.3 cm.



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4. Field Quality Assurance/Quality Control Program

The objective of the field quality assurance/quality control (QA/QC) program is to obtain soil and groundwater samples and other field measurements that provide data of acceptable quality that meets the objectives of the Phase Two ESA. The objectives of the QA/QC program will be achieved through the implementation of procedures for the collection of unbiased (i.e. non-contaminated) samples, sample documentation and the collection of appropriate QC samples to provide a measure of sample reproducibility and accuracy. The field QA/QC measures will comprise:

- Decontamination Protocols;
- Equipment Calibration;
- Sample Preservation;
- Sample Documentation; and,
- Field Quality Control Samples.

Details on the field QA/QC measures are provided below.

4.1 Decontamination Protocols

Decontamination protocols will be followed during field sampling where non-dedicated sampling equipment is used to prevent sample cross contamination. For the borehole drilling and soil sampling, soil sampling devices will be cleaned/decontaminated between sampling intervals and auger flights between borehole locations in according with SOP requirements. All decontamination fluids will be collected and stored in sealed, labeled containers.

4.2 Equipment Calibration

All equipment requiring calibration will be calibrated in the field according to manufacturer's requirements using analytical grade reagents, or by the supplier prior to conducting field activities, and subsequently checked in the field. The calibration of all precalibrated instruments will be checked in the field using analytical grade reagents and re-calibrated as required. For multiple day sampling events, equipment calibration will be checked prior to the beginning of sampling activities. All calibration data will be documented in a bound hard cover notebook.

4.3 Sample Preservation

All samples will be preserved using appropriate analytical test group specific reagents, as required, and upon collection placed in pre-chilled insulated coolers packed with ice for storage and transport.

4.4 Sample Documentation

All samples will be assigned a unique identification number, which is to be recorded along with the date, time, project number, company name, location and requested analysis in a bound field notebook. All samples will be handled and transported following COC protocols.

4.5 Field Quality Control

Field quality control samples will be collected to evaluate the accuracy and reproducibility of the field sampling procedures. For groundwater sampling, one (1) field duplicate is to be collected for every ten (10) samples submitted for chemical analysis. For multiple day sampling events, at least one (1) field duplicate soil and groundwater sample will be submitted for chemical analysis. The field duplicate samples will be assessed by calculating the relative percent difference and comparing to the analytical test group specific acceptance criteria.

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Appendix B – Survey



Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

Appendix C – Borehole Logs



			ROCK CORE	L	JG	J							В	H	21	2		
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			ankuna Mineirenung Ontenin	DATE S		TED	-	OMP		D					DRA			IB
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		o Grou	p of COmpanies	Davis		ng		ME 5				HQ				- - 1 o	f 2	
ĉ					JOINT		ARÁC	TERI	STIC	S								1
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
171.9	- - -		See Borehole Log for Details															
171.5	-		QUEENSTON FORMATION															
-	-			1	в	F	С	RU						1	100	74	95	
71.1	-		Shale with interbedded siltstone, and clay layers.				С	RP										
170.7 170.7	-		Shale (71%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~5.7 m and between moderately weathered and unweathered						NC	20 mm								
170.5 170.4			slightly weathered to ~5.7 m and between	4	п		6				Π			2	100	100	100	
	6		below.	1	В	F	C C	RP RP						2	100		100	
170.0	-		Limestone (3%) fine grained, grey, medium strength, unweathered															
170.0	_		-															
69.6 69.5	-		Siltstone (25%) fine grained, grey, medium strength, unweathered.		F	v												
169.5		7/17/	Discontinuities: bedding joints are rough planar to smooth undulating and at wide to						NC NC	20 mm								
169.3 169.3	7		planar to smooth undulating and at wide to very close intervals.						INC	60								
169.1	-	ĪĪĪĪ	Vertical fractures were noted at ~6.6 m, 7.1 n 7.6 m, 9.1 m and 10.2 m.	n, 1	F	v		RP		mm				3	100	74	100	
169.0 168.7		ЩЦ			В	F	C C	SU							100	14	100	
168.7	_		A Clay (1%) layers, heavily weathered, very low strength were noted at ~5.4 m, 6.6 m and	4	F	v												
168.6 168.5	-		6.8 m.															
168.3	8	ЩЩ																
168.1	-																	
	-																	
167.6 167.6	-																	
	_			1	в	F	w	su						4	100	89	100	
167.2 167.2	-9					[•]	M	SP										
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² * exp.

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			esugation				TED		OMP		D							NUN	
		Mills P	Parkway, Mississauga, Ontario		1/05/				01/05				D. Pano				14	A	
		o Grou	p of COmpanies			R Drilli	na		ME 5				ore B/ Hq	ARREL		SHE	ET 2 o	f 🤈	
	IVIUZZ						<u> </u>		TERI										0
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION		NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4		5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
163.9 163.7			QUEENSTON FORMATION																
63.4			Shale with interbedded siltstone, and clay layers.																
63.0			Shale (71%) thinly bedded or laminated, re low strength, alternating between heavily an slightly weathered to ~5.7 m and between moderately weathered and unweathered below.	d, nd	1	В	F	w	SP						7	100	100	100	007
-			Limestone (3%) fine grained, grey, medium strength, unweathered					W	SP										
62.0 61.9	-14		Siltstone (25%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to																
-			very close intervals. Vertical fractures were noted at ~6.6 m, 7.1																_
61.1	-15		7.6 m, 9.1 m and 10.2 m. A Clay (1%) layers, heavily weathered, very low strength were noted at ~5.4 m, 6.6 m a		1	В	F	W W	SP SP						8	100	100	100	
60.9 60.7	- -		6.8 m. End of Borehole at 15.4 m																
- - - - - - - - - - - - - 	-16																		
- - - - - - - - - - - - - 	-17																		
- - - - - - - - - - - - - - - - - - -	-18																		
	-19																		
- - - - - - - - - - - - - 	-20																		

oject: ocation:	Geotechnical Investigation Erin Mills Town Centre, 51		Mills	Parkw	ay, MIssis	ssaug		Sheet No	1	of _1
ate Drill	ed: January 19 - 22, 2024		-	er Sample			Combustible \ Natural Moist	/apour Reading ure	×	
rill Type				⁻ (N) Value amic Cone ⊺	-	Ø	Plastic and Li Undrained Tri			-0
atum:	Geodetic			lby Tube d Vane Test		S S	% Strain at Fa	ailure	€	•
Soil/Rock Symbol	Soil Description	ELEV. m	Depth (m)	20 near Strength		80 kPa	25 Natural Mo Atterberg Lin	apour Reading (ppm) 50 75 isture Content % nits (% Dry Weight)	Sample	Natura Unit Weigł kN/m
	~115 mm ASPHALT over FILL - sand and gravel granular, brown, moist	176.84 ~176.3	0	18 Ö	100	200	X X	20 30		
TTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTT	FILL - clayey silt, trace sand, reddish brown, moist	~175.8	1	Č	50/100 mm		×			
	SILT TILL - some clay, trace sand, reddish brown, dense to very dense	_	2		50/125 mm		×			
	- trace shale fragments SHALE BDEROCK - Queenston	~174.1	3		50/7 <u>5</u> mm		×			
	Formation, occasional limestone and sandstone layers, reddish brown Coring Commenced	<u>!</u>								
	See Core Log for Details		5 6 7 8 9 10 11 12 13							
	End of Borehole	~161.4								
otes:			=		xp.		Elapsed Time	Water Level (m)		ble Oper to (m)

PROJI		ical In:	ROCK CORE		TATIO		E	LEVA		N (m)		DATUM Geode				JECT	NUM 5776	
OCA	TION		estigation arkway, Mississauga, Ontario	DATE \$	STAR /23	TED		OMP	LETE 2/23			D. Pan) BY chal				NUN	
CLIEN The		o Grou	p of COmpanies	DRILLE	Drilli	<u> </u>	С	ME 5	5 - T	rack	C	HQ	ARREI	-	SHE	E T 1 o	f 2	_
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS			SPACING		EILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1 74.3	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	
/4.3	- - -		See Borehole Log for Details															
73.8 73.7 73.7	-3			1	F B	V F	C C	RU RU						1	100	75	95	
73.3 73.3 73.0 73.0	- - 4		Shale with interbedded siltstone, and clay layers. Shale (78%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~7.2 and between moderately weathered and unweathered	1														
2.4 2.4 2.4	- - - - -		Signify Weathered to ~7.2 and between moderately weathered and unweathered below. Limestone (6%) fine grained, grey, medium strength, unweathered	1	B F F	F V V	C M	RU RP				=		2	100	91	100	
72.3 71.8 71.8 71.8	-5		Siltstone (14%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
71.3 71.2 71.1 71.0	- - - - - -		Vertical fractures were noted at $\sim 3.1 \text{ m}$ 4.4 r	1	В	F	VC VC	RP RP	SO NC	mm				3	100	56	90	
71.0 70.9 70.8 70.6 70.5	- - - -		m. Rubble layers, heavily weathered, very low strength were noted at ~5.1m (50 mm) and 6 m (40 mm).	.3	F F	v v			NC									
70.4 70.4 70.3 70.3 70.3	-7			1	В	F	C M	RP SU						4	100	90	100	
70.1 69.6 69.6 69.5 69.3	- - - - - - - - 8																	
69.0 68.9 68.7 68.5																		
58.1 58.0 57.7 57.6	-9			1	В	F	WW	SU SP						5	100	100	100	
67.2 67.1																		-
66.8 66.8	10			1	В	F	W W	SP SP						6	100	100	100	

‴ехр.

			ROCK CORE										E	SH	20	1		
PROJE		ical Inv	restigation	ORIE Vert		ON	E	176.8		N (m)		DATUM Geodet	io				NUM 5776	
LOCA		ical inv	estigation	DATE		RTED	c			ED	L	OGGE					NUN	
5100) Erin	Mills P	Parkway, Mississauga, Ontario	01/2	2/23			01/22	2/23			D. Pan	chal			2		
CLIEN The		o Grou	p of COmpanies	Dav	.ER is Drill	ina		DRILL DME 5				HQ	ARREI	-	SHE	ЕТ 2 о	f2	
			F			<u> </u>		TERI										9
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS		ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
66.3 66.1	-		QUEENSTON FORMATION															
	-11		Shale with interbedded siltstone, and clay layers.															
65.5 65.4 65.1			Shale (78%) thinly bedded or laminated, red low strength, alternating between heavily an slightly weathered to ~7.2 and between moderately weathered and unweathered	l, d			14/							-	100	100	100	
65.1 64.7	-12		below. Limestone (6%) fine grained, grey, medium strength, unweathered	1	В	F	WW	SP SP						7	100	100	100	
64.6			Siltstone (14%) fine grained, grey, medium strength, unweathered.															
64.0	-13		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
63.6 63.5			Vertical fractures were noted at ~3.1 m, 4.4 4.5 m, 6.3 m and 6.5 m. Clay (2%) layers, heavily weathered, very lo strength were noted at ~5.5 m, 5.9 m and 6.	1	В	F	WW	SP SP						8	100	100	100	
63.1 63.0			Rubble layers, heavily weathered, very low strength were noted at ~5.1m (50 mm) and															
62.6 62.6	-14		m (40 mm).	0.0								_						
-	-			1	В	F	w							9	100	100	100	
61.7 61.6	-15						W	SP				_						
61.4	-		End of Borehole at 15.4 m															
-	-16																	
	-17																	
	-18																	

Project No. Project: Location:	GTR-00257769-H0 Geotechnical Investigation Erin Mills Town Centre, 51												Dra		g No. t No.			3 of <u>1</u>
Date Drilled: Drill Type: Datum:	January 17 - 18, 2024 Hollow Stem Augers Geodetic		_	SPT (I Dynan Shelby	Sample N) Value nic Cone Tube /ane Te	e Tes	t	<u> </u>			Nat Plas Unc % S	stic a drain Strair	stible V Moistu and Liq ed Tria n at Fa meter	re Juid Lir Ixial at		ng H	□ × ⊕	-0
Groundwater Soil/Rock Symbol	Soil Description	ELEV. m 176.76	Depth (m)	Shea	20 Ir Streng	40		ilue) 60	80	kPa		25	5 ral Moi erg Limi	50	eading 75 ontent Dry Wei		Sample	Natural Unit Weight kN/m ³
FILL brow FILL redd SIL - redd - redd - trac SH/	Demm ASPHALT over L - sand and gravel granular, m, moist L - clayey silt, trace sand, lish brown, moist T TILL - trace sand, some clay, lish brown, very dense ce shale fragments ALE BDEROCK - Queenston nation, occasional limestone and lstone layers, reddish brown Coring Commenced See Core Log for Details	~176.2 ~176.1 ~176.1 ~174.2 ~173.7 ~173.7	1 2 3 4 5 6 7 7 8 8 9 10 11 11 12 13 14								> ×							22.0
	- End of Borehole	~161.3	15															
Notes: 1. Borehole advanced using a specialist drillir 2. This drawing forms	to completion at ~15.5 m depth by conventional soil sampling g subcontractor. For borehole definitions, see notes prior to l part of and must be read in conjunction with the subject repo porehole data requires interpretation assistance by exp profes	logs. rt (Ref. No.:			ж е Вra				•		Elar Tiu nuary bruary		2024		Water Level (m) ~5.9 ~6.0		t	le Open to (m) Well Well

CLIENT The M		VIIIIS P	arkway, Mississauga, Ontario	Vertio	STAR	TED	c	176.8 0 000 01/18	} LETE			OATUM Geodet OGGEE) BY				57769 NUM	
	IUZZU		p of COmpanies	DRILL		na	D	RILL ME 7	TYPI		C	CORE B			SHEE			
~							ARAC	KONGHNESS	STIC		WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	
ILEV/	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. O	OINT	RIEN	SPACING	SUUG	FILLING	.HER	VEAT	TRE	RAC.		ECO	RQD	VATE	
ш 1	<u>0</u> 2	о 3	4	2 5	- 6	0	ഗ 8	9	ш 10	∢೭ 11	≤ 12	0 13	14	₩ 15	₩ 16	₩ 17	≤œ 18	1
174.3	-		<u> </u>															-
-			See Borehole Log for Details															
73.7	3		QUEENSTON FORMATION												<u> </u>		<u> </u>	
73.6- 73.6-				1	F F		С	RU						1	100	76	95	
173.5 173.4					B	F	č	RU							ļ			
73.3 73.2			layers. Shale (78%) thinly bedded or laminated red							[
	4		Shale (78%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~5.0 m and between moderately weathered and unweathered	ď														
72.5			below.	1	В	F	C C	RU RP						2	98	94	100	
			Limestone (9%) fine grained, grey, medium strength, unweathered															
71.9- 71.8- -	5		Siltstone (13%) fine grained, grey, medium strength, unweathered.															
71.5 71.5			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
-			Vertical fractures were noted at ~3.2 m, 3.3 r and 8.0 m.	m														
71.0- 70.9-	-	<u></u>		1	в	F	С	RP						3	100	100	100	
70.7	6						М	SU										
-																		
70.1														<u> </u>	<u> </u>			-
69.9 69.8	,																	
69.7 69.6	1																	
0.00				1	в	F	М	SU						4	100	98	100	
69.1							С	SU		[$\left \right $							
-					_													
68.8 68.8 68.6	8				F	V												-
68.4																		
Ē																		
67.8_g				1	в	F	w	su						5	100	100	100	
67.8_ ç 67.7	ש						W	SP										
67.4 67.3																		
-																<u> </u>	\square	
66.8	10																	
66.7 - 66.5 ⁻	ł																	
66.5				1	В	F	W	SP						6	100	100	100	

			ROCK CORE						1	1 ()		AT:		717	20		N 11 7	
PROJI Geo		ical Inv	estigation	ORIEN Vertic		JN		LEVA 176.8		N (M)		ATUM Geode	ic			JECT R-002		
OCA	TION			DATE		TED		OMP		D	ΤL	OGGE				WING		
		Mills P	arkway, Mississauga, Ontario	01/18				01/18				D. Pan				3.	A	
LIEN The		o Grou	p of COmpanies	DRILLE Davis		ng		RILL ME 7				HQ	AKKEI	-	SHE	د ا 2 د	f 2	
								TERI			0			۲.	(%		(%	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	D. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
<u>武</u> 1	2	6 3	4	Ö V 5	6	0 7	8 8	<u>2</u> 9	正 10	マシ 11	<u>≥</u> 12	<u></u> 13	世世 14	2 15		22 17	≥₩ 18	1
-	-		-				w	SP										F
66.1 65.9	-	ΠΠ	QUEENSTON FORMATION															
	-11																	
	-		Shale with interbedded siltstone, and clay layers.															F
65.3 65.3	-		Shale (78%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~5.0 m and between moderately weathered and unweathered															
35.0 34.9	-		slightly weathered to ~5.0 m and between moderately weathered and unweathered below.		_	_		0.5										-
	-12		Limestone (9%) fine grained, grey, medium strength, unweathered	1	B	F	W W	SP SP						7	100	100	100	
64.4 64.4	-																	
54.4			Siltstone (13%) fine grained, grey, medium strength, unweathered.															
63.9	-13		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
33.6	- 13		Vertical fractures were noted at ~3.2 m, 3.3 r	n														
63.5 63.3	-		and 8.0 m.	1	В	F	w	SP						8	100	100	100	
63.1	-						Ŵ	SP										
62.9	-14																	
	-																	
62.2	-																	
62.1	-			1	Б	F	w	en						9	100	100	100	
61.7	-15			1	B		W	SP SP						9	100	100	100	
61.6	-																	
61.3	-		End of Borehole at 15.5 m															┢
-	-																	
-	-16																	
	-																	
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Project No.	<u>GTR-00257769-H0</u>	90	-			••••	•	U	2	-0	Dra	wing	No		4
Project:	Geotechnical Investigation											heet	No	<u> </u>	of <u>1</u>
Location:	Erin Mills Town Centre, 51	00 Erin	Mil	ls P	arkw	ay, N	llssis	sau	ıga,	Ont	ario				
			- A	luaer S	Sample						ustible Va Il Moistur	•	eading	×	
Date Drilled:	,		- s	SPT (N	, Value c Cone 1	4	0	Ø		Plastic	and Liqu	uid Limit	F		-0
Drill Type:	Hollow Stem Augers Geodetic		s	Shelby	Tube	est					ned Triax in at Fail			ŧ	ł
Datum:	Geodelic		_ F	ield Va	ane Test			s		Penetr	ometer				•
Groundwater Soil/Rock Symbol	Soil Description	ELEV. m	Depth (m)	Shear	20 Strength	SPT (N Va 40	alue) 60		<pa< td=""><td>2 Nat</td><td></td><td>50 ture Con</td><td>ding (ppm) 75 tent % Weight)</td><td>Sample</td><td>Natural Unit Weight kN/m³</td></pa<>	2 Nat		50 ture Con	ding (ppm) 75 tent % Weight)	Sample	Natural Unit Weight kN/m ³
~14	10 mm ASPHALT over	176.31	0		3 C	100 6		200		X		20	30		
bro \	L - sand and gravel granular,	~175.7 , _~175.3			C	45				2	×			Ø	
FIL	L - clayey silt, trace sand, dish brown, moist		1			Ô	70/	250 m	m	×					
	LT TILL - some clay, trace sand, dish brown, very dense		2				13/2	Ő		×					
	ace shale fragments	_				50/25 m	im			×					
		_	3			50/125 r	nm			×					
	Coring Commenced	~172.9													
For	mation, occasional limestone and	-	4												
sar	ndstone layers, reddish brown	-													
		-	5												
			6												
			7												
		_													
		_	8												
		-													
		-	9												
	See Core Log for Details	-													
			10												
			11												
		_	12												
		_													
		-	13												
		-													
		_	14												
		-													
			15												
	End of Borehole	~160.7													
		1	л Е]					<u> </u>		Elapse	<u> </u>		/ater	Hc	le Open
Notes:					6	X	\mathbf{r}		10	Time			evel (m)		to (m)
using a specialist dri	ed to completion at ~15.7 m depth by conventional soil samplin lling subcontractor. For borehole definitions, see notes prior to	logs.			C).			uary 29 ruary 4		1	6.5 6.2		Well Well
	is part of and must be read in conjunction with the subject report ; borehole data requires interpretation assistance by exp proference.					I									
Denote use by outlets	~				Brar	npto	n								

PROJI Geo		ical Inv	ROCK CORE	ORIEN Vertic	TATIO		E	LEVA		1 (m)		DATUM Geode				JECT	NUM	
LOCA	TION			DATES	STAR	TED	c	OMP	LETE	D	+	LOGGE	D BY			WING	NUN	
5100 CLIEN		Mills P	Parkway, Mississauga, Ontario	01/19				01/19		E	_	D. Par CORE E			SHE	4. ET	A	
		o Grou	p of COmpanies	Davis	Drilli		C	ME 7	5 - TI	ruck		HQ		-			of 2	
ELEVATION (m)	DEPTH (m)	SYMBOL		OF SETS			SPACING	LER SSANHONOR		APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	
ELE	DEP	SYM	GENERAL DESCRIPTION	Ň		ORIE	SPA	ROU	FILLING	APE (mm	WEA	STR	FRA	RUN	REC	RQD	REC	
1	2	3	4	5	6	7	8	9	10	11	12		14	15	16	17	18	-
173.3	-3										Т			+				┢
	•		See Borehole Log for Details															
172.9 172.8			QUEENSTON FORMATION	1	В	F	C C	RU RU						1	100	0	95	
172.6 172.6		<u> </u>						RU				-						
., 2.0	-4		Shale with interbedded siltstone, and clay layers.															
171.7			Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.5 m and between moderately weathered and unweathered	1	в	F	M C	RU RP						2	98	87	95	
171.6 171.3			Limestone (6%) fine grained, grey, medium strength, unweathered															
171.2	-0		-		F	V V										-		╞
171.2 171.1			Siltstone (19%) fine grained, grey, medium strength, unweathered.		F	v					Π			-				
171.0 170.9	-		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to															
-			very close intervals.	n 1	в	F	с	RP						3	100	90	100	
-	-6		Vertical fractures were noted at ~5.1 m, 5.2 n 5.4 m, 6.4 m, 6.8 m, 7.0 m, 7.2 m, 8.4 m, 11.0 m and 13.4 m.	6		.	č	RP							100			
170.0																		
169.9 169.7	-													1				
169.6 169.4					F	v											1	t
169.3	-7	F.			F	V												
169.1 169.0		臣			F	v												
169.0 168.9 168.8	-			1	в	F	С	RP						4	100	62	100	
168.8							М	SU										
168.6 168.5	8																	
																		1
168.0 167.9					F	v												
167.6																		
167.6															_			
167.3 167.3	-9			1	В	F	C W	SU SP						5	95	94	100	
167.1 167.0								.										
-	-																	
166.5														-		<u> </u>		╞
166.4 166.2	-10																	
166.2																		
-	- -			1	в	F	w	SP						6	100	100	100	
							W	SP										
165.4	-																	
	\sim																	

PROJ I Geo		cal Inv	ROCK CORE	ORIEN Vertic	TATIC al	ON		176.3	3	N (m)		ATUM Geodet	ic			JECT	NUM 5776	
5100		Mills P	arkway, Mississauga, Ontario	DATE \$		TED		OMP		ED		OGGE D. Pan			DRA	WING 4	i NUN ∆	IBI
CLIEN	IT			DRILLE	R		D	RILL	TYP		C	ORE B		_	SHE	ET		
The	Muzz	o Grou	p of COmpanies	Davis				ME 7				HQ 				2 0	12	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
65.3 65.1 65.0	-11		QUEENSTON FORMATION															
64.8 64.7 64.5	- - -		Shale with interbedded siltstone, and clay layers.		F	v												
64.5 64.3 64.0	-12		Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.5 m and between moderately weathered and unweathered below.	1	В	F	C W	SP SP						7	100	97	100	.
63.5 63.5	 - - -		Limestone (6%) fine grained, grey, medium strength, unweathered															
63.0	-13		Siltstone (19%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.		F	v												
62.9 62.7 62.6 62.5 62.4	- - - - - - - - - - 14		Vertical fractures were noted at ~5.1 m, 5.2 r	1 n, 6	B	F	M M	SP SP						8	100	98	100	
61.7	- - - -																	
61.6 61.3	-15			1	в	F	w	SP SP						9	100	100	100	
61.1 61.0 60.7	- - 																	
-	_		End of Borehole at 15.7 m															
-																		
-	- 17 																	
	-18																	

Project No. Project: Location:	GTR-00257769-H0 Geotechnical Investigation Erin Mills Town Centre, 510	_			eho vay, Missi		Sheet No		5 of <u>1</u>
Date Drilled: Drill Type: Datum:	January 19 - 22, 2024 Hollow Stem Augers Geodetic		- s - c - s	uger Sample SPT (N) Value Dynamic Cone Shelby Tube Sield Vane Tes	Test —	Natural Mo	l Liquid Limit Triaxial at t Failure	× ÷	-0
- FILL brow Fill - redd - redd - trac SHA - Form sance - SHA - Form sance - trac	Soil Description Omm ASPHALT over - sand and gravel granular, n, moist - clayey silt, trace sand, ish brown, moist T TILL - some clay, trace sand, ish brown, very dense ce shale fragments Coring Commenced ALE BDEROCK - Queenston nation, occasional limestone and istone layers, reddish brown See Core Log for Details	ELEV. m 176.51 ~175.8 ~175.5 ~173.2	(ii) the second	20 Shear Strengt	SPT (N Value) 40 60 100 50/75 mm 65/75 m 50/125 mm 50/125 mm	25 Natural	e Vapour Reading (ppm 50 75 Limits (% Dry Weight) 20 30		Natural Unit Weight kN/m ³
	- End of Borehole	~160.8							
Notes: 1. Borehole advanced using a specialist drillir 2. This drawing forms	to completion at ~15.7 m depth by conventional soil sampling g subcontractor. For borehole definitions, see notes prior to l part of and must be read in conjunction with the subject repo porehole data requires interpretation assistance by exp profes	ogs. rt (Ref. No.:			XP.	Elapsed Time	Water Level (m)		le Open to (m)

LOCA	otechn TION 0 Erin		restigation Parkway, Mississauga, Ontario	ORIEN Vertin DATE 01/22 DRILL	cal STAR 2/23	NC	C	LEVA 176.5 OMP 01/22 ORILL	5 LETE 2/23	D	1	DATUM Geodet LOGGEE D. Pano CORE B	ic) BY chal			JECT R-002 WING 5/	57769 NUM	9-⊢
		o Grou	p of COmpanies	Davis	s Drilli		C	ME 5	5 - T	rack				1		_ 1 o	f 2	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1 74.0	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
73.2	-3		See Borehole Log for Details															
73.1 73.1	-			1	B F	FV	C C	RU RU						1	100	0	95	
73.0 72.9 72.9	-		Shale with interbedded siltstone, and clay layers.		F	v												
72.6 72.6 72.3 72.3	_		Shale (74%) thinly bedded or laminated, red low strength, alternating between heavily and slightly weathered to ~4.9 and between moderately weathered and unweathered below.	³ 1	B	FV	M C	RU RP						2	100	90	100	
71.7 71.6	-		Limestone (3%) fine grained, grey, medium strength, unweathered		F	v												
71.6 71.5 71.4	-		Siltstone (22%) fine grained, grey, medium strength, unweathered.		F	v								-				$\left \right $
71.0 71.0			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
70.8 70.7 70.7	6			1	В	F	C C	RP RP						3	100	92	100	. (
70.4 70.3 70.2 70.1	- - -		A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm).		F	v												
70.0 69.8 69.5	_				F	v												
69.1 69.0	-			1	В	F	C C	RP SU						4	100	83	100	-
68.7 68.7 68.4																		
68.4 68.0																		
67.9 67.6 67.5 67.5 67.4	-			1	В	F	M M	SU SP						5	100	100	100	-
67.3 67.2 66.9 66.8 66.8	-				F	v												
66.7 66.5 66.4	-10 - -			1	В	F	М	SP						6	100	97	100	

PN rin Mills I Izzo Grou	vestigation Parkway, Mississauga, Ontario up of COmpanies	ORIEN Vertio DATE 9 01/22 DRILLI	al STAR			176.8	5			ATUM Geodet			PRO. GTR	JECT R-002		
PN rin Mills I Izzo Grou	Parkway, Mississauga, Ontario	DATE 9 01/22	STAR	TED						Geodet			GTF	<u>-002</u>	5776	y_⊦
rin Mills I Izzo Grou		01/22			1.1.1	:OMP	LETE	סי		OGGED) BY		DRA\	NING	NUM	
Ē	up of COmpanies	DRILLI	.1ZJ			01/22				D. Pano			DIVA	5/		
Ē	Jp of COmpanies					RILL				ORE BA	ARREL		SHEE			
or (III)		Davis		<u> </u>		ME 5				HQ 			Τ	20	r 2	6
	GENERAL DESCRIPTION	o NO. OF SETS	o JOINT TYPE	ORIENTATION	∞ SPACING	د ROUGHNESS	DI FILLING	L APERTURE (mm)		C STRENGTH	FRACTURE FREQUENCY	15 RUN NUMBER	ы RECOVERY (%)	ари 17	BRECOVERY (%)	
		J	F						12		14	15		17	10	
				V	М	SP										
	layers.															
	-] below.	d														.
2	strength, unweathered	1	В	F	W W	SP SP						7	100	100	100	
	strength, unweathered.															
3	$\frac{1}{2}$ Vertical fractures were noted at ~ 3.5 m 3.9															
	-] m, 4.3 m, 4.8 m, 5.1 m, 6.5 m, 7.0 m, 9.6 m and 10.5 m.	1	В	F	М	SP						8	100	97	100	
	- strength was noted at ~10.1 m (110 mm). I 				М	SP										
4																
5		1	В	F	W W	SP SP						9	100	100	100	
6	End of Borehole at 15.7 m															
7																
R																
2 3 4 6		Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red low strength, alternating between heavily an slightly weathered to ~4.9 and between moderately weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~ 3.5 m, 3.9 m, 4.3 m, 5.1 m, 6.5 m, 7.0 m, 9.6 m and 10.5 m. A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). Image: the strength was noted at ~10.1 m (110 mm).	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered or 4.9 and between moderately weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered. Siltstone (22%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~ 3.5 m, 3.9 m, 4.3 m, 5.1 m, 6.5 m, 7.0 m, 9.6 m and 10.5 m. A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). Image: the of Borehole at 15.7 m	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered to ~4.9 and between heavily and sightly weathered of an unweathered below. 1 B Image: Ima	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and signtly weathered to ~4.9 and between moderately weathered and unweathered below. 1 B F Image: Shale (3%) fine grained, grey, medium strength, unweathered 1 B F Image: Shale (3%) fine grained, grey, medium strength, unweathered 1 B F Image: Shale (22%) fine grained, grey, medium strength, unweathered. 1 Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F Vertical fractures were noted at ~ 3.5 m, 3.9 m, and 10.5 m. A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). 1 B F Image: Shear (1, 1, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2,	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered below. 1 B F W Image: Strength, unweathered 1 B F W Siltstone (3%) fine grained, grey, medium strength, unweathered. 1 B F W Siltstone (22%) fine grained, grey, medium strength, unweathered. 1 B F W Siltstone (22%) fine grained, grey, medium strength, unweathered. 1 B F M Wertical fractures were noted at ~ 3.5 m, 3.9 m, 4.3 m, 5.1 m, 6.5 m, 7.0 m, 9.6 m and 10.5 m. 1 B F M A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). 1 B F W Image: Strength unweathered 1 B F W March of Borehole at 15.7 m 1 B F W	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, over strength, alternating between heavily and slightly weathered to 4.9 and between heavily and slightly weathered and unweathered below. 1 B F W SP Imastone (3%) fine grained, grey, medium strength, unweathered 1 B F W SP Imastone (2%) fine grained, grey, medium strength, unweathered. 1 B F W SP Image: Siltstone (22%) fine grained, grey, medium strength, unweathered. 1 B F M SP Image: Siltstone (22%) fine grained, grey, medium strength, unweathered. 1 B F M SP Image: Siltstone (22%) fine grained, grey, medium strength, unweathered. 1 B F M SP Image: Siltstone (22%) fine grained, grey, medium strength unweathered. 1 B F M SP Image: Wather and to 5.m. A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). 1 B F W SP Image: Wather and the strength was noted at ~10.1 m (110 mm). 1 B F W SP Image: Site andit strength was noted at 15.7 m Image:	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, is wrength, alternating between heavily and slightly weathered to ~4.9 and between moderately weathered to ed.9 and between the service of the servic	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, iternating between heavily and slightly weathered to ~4.9 and between below. 1 B F W SP Limestone (3%) fine grained, grey, medium strength, unweathered below. 1 B F W SP Silistone (22%) fine grained, grey, medium strength, unweathered 1 B F W SP With the smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~3.5 m, 3.9 m, 4.3 m, 4.8 m, 5.1 m, 6.5 m, 7.0 m, 9.6 m and 10.5 m. 1 B F M SP A Rubble layers, heavily weathered, low strength was noted at ~10.1 m (110 mm). 1 B F W SP End of Borehole at 15.7 m End of Borehole at 15.7 m 1 B F W SP	Shale with interbedded siltstone, and clay layers. Shale with interbedded or laminated, red, low strength, alterating between heavily and slightly weathered to ~4.9 and between below. 1 B F W SP Image: State of the strength unweathered and unweathered 1 B F W SP Image: State of the strength unweathered Sitistone (22%) fine grained, grey, medium strength, unweathered 1 B F W SP Sitistone (22%) fine grained, grey, medium strength, unweathered Sitistone (22%) fine grained, grey, medium strength, unweathered 1 B F W SP Image: Strength, unweathered Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F M SP Image: Strength, unweathered. No.5.1 m, 0.5 m, 7.0 m, 9.6 m and 10.5 m. A Rubble layers, heavily weathered low strength was noted at ~10.1 m (110 mm). 1 B F M SP Image: Strength was noted at ~10.1 m (110 mm). 1 B F W SP Image: Strength was noted at ~10.7 m Image: Strength was noted at ~10.7 m <td< td=""><td>Shale with interbedded siltstone, and clay layers. 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Shale with interbedded siltstone, and clay significant alternating between heavily and signifity weathered and unweathered below. 1 B F W SP Image: Shale with interbedded siltstone, and unweathered below. 1 B F W SP Image: Shale with interbedded siltstone, and unweathered below. 1 B F W SP Image: Shale with interbedded siltstone, and unweathered. 1 B F W SP Image: Shale with unweathered. 1 B F W SP Image: Shale with unweathered. 1 B F M SP Image: Shale with unweathered. 1 B F M SP Image: Shale with unweathered. 1 B F M SP Image: Shale with unweathered. 1 B F M SP Image: Shale with unweathered. 1 B F M SP Image: Shale with unweathered. 1 B F <td< td=""><td>Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered and unweathered below. Immestone (3%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Mark of the grained of the grai</td><td>Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and lightly weathered or 4.9 and between mederately weathered and unweathered below. 1 B F W SP Image: SP I</td><td>Shale with interbedded siltstone, and clay layers. Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and siltstyny weathered - 49 and between moderately weathered and unweathered bedded. Image: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and siltstyny weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Image:</td><td>Shale with interbedded siltstone, and clay says: Shale with interbedded siltstone, and clay says: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and singhty weathered - 49 and between moderately weathered - 49 and between moderately weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and singhty weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedde</td></td<>	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered and unweathered below. Immestone (3%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Immestone (22%) fine grained, grey, medium strength, unweathered. Mark of the grained of the grai	Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and lightly weathered or 4.9 and between mederately weathered and unweathered below. 1 B F W SP Image: SP I	Shale with interbedded siltstone, and clay layers. Shale with interbedded siltstone, and clay layers. Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and siltstyny weathered - 49 and between moderately weathered and unweathered bedded. Image: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and siltstyny weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image:	Shale with interbedded siltstone, and clay says: Shale with interbedded siltstone, and clay says: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and singhty weathered - 49 and between moderately weathered - 49 and between moderately weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, alternating between heavily and singhty weathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedded or laminated, red, low strength, unweathered. Image: Shale (74%) thinly bedde

Pi	oject	t No.	<u>дтк-00257769-но</u>	g o	f	В	0	re	hc	ble	9	2	0	5 Dra	awing	No		6
	oject		Geotechnical Investigation												Sheet	No	<u>1</u>	of <u>1</u>
Lo	ocatio	on:	Erin Mills Town Centre, 510	00 Erin	N	lills I	Park	way	, MIs	siss	aug	a, C	Onta	rio				
-	- 4 - 5		lonuon (12, 2024		_	Auger	Samp	le		Þ	3					Reading		
		Drilled:	January 12, 2024 Hollow Stem Augers		_	SPT (I	N) Valu			OE	3	Ρ	lastic a		uid Limi	t	× ⊢	-0
	rill Ty atum		Geodetic		-	Shelby	y Tube					%	5 Strair	ed Tria n at Fai			0	,
D	atum				-	Field \	√ane T	est		5		P	enetro	meter				
Groundwater	Soil/Rock Symbol		Soil Description	ELEV. m 175.26	Depth (m)	Shea	20 ar Strer	40	(N Value 60	8	30 kPa 00		25 Natu	ral Mois erg Limi	50 sture Co	ading (ppm 75 ntent % y Weight) 30	Sample (1	Natural Unit Weight kN/m ³
5 (FILL	mm ASPHALT over - sand and gravel granular, 4 /n, moist /	~174.9		É	-	8					×	×				
		FILI	- clayey silt, trace sand, dark n to reddish brown, moist	~173.3	1		22 O	Ž					×	×				
		SIL grav dens	T - some sand, trace clay, trace el, brown, moist, dense to very se	_	2			ð					×					
			-	_	3				ð Ö					×				
			-		4				59 O				;	ĸ				
		- beo	coming more sandy, grey & wet w ~5.5 m depth	_	6													
					7					č	2 2			×				
		_	-	-	8							50 mn	۱ •	×				
		SIL ⁻ redd	T TILL - some clay, trace sand, ish brown, very dense	~166.8	9			50/	125 mm				¥					
			- 		10								~					
.GDI 3/20/24		Forn	action, occasional limestone and - stone layers, reddish brown Coring Commenced	~164.5	11	1		50 /	75 mm O								X	
			-	-	12													
			See Core Log for Details - -	-	14	1												
-AGWGLUZEXP 25//			-	~159.8	15	5												
PNG			End of Borehole															
	<u> </u>	<u> </u>			 								lapsed Time			Water Level		le Open to (m)
1. us 2. G	ing a sp This dra TR-0025	ecialist drillin awing forms	to completion at ~15.4 m depth by conventional soil sampling og subcontractor. For borehole definitions, see notes prior to k part of and must be read in conjunction with the subject repor porehole data requires interpretation assistance by exp profes	ogs. rt (Ref. No.:		ų		ЭХ Эт	-).		anuar	ry 29, ary 4,			(m) ~5.3 ~5.6	-	Well Well
							Dľ	amp	ιση									

			ROCK CORE										D	Π	20			
PROJE Geot		cal Inv	restigation	ORIEN Verti		ON	Ē	175.3		N (m)		DATUM Geodet	ic]			NUM 5776	
			esugation	DATE		TED	c	COMP	-	ED	Τ	OGGGE					NUN	
		Mills P	arkway, Mississauga, Ontario	01/1				01/12		_		D. Pan			0	6	A	
The		o Grou	p of COmpanies	DRILL Davi	s Drill		C	DRILL CME 5	55 - T	rack		HQ	AKREL		SHE	ET 1 o	f 1	
(m) NC	-			ETS	JOIN		ARAC		STIC		RING	Ŧ	щ	BER	۲۷ (%)		۲Y (%)	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SET		ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY	RQD	WATER RECOVERY (%)	
1	2	3	4	5	- 6	7	8	9	10	<u>ع</u> ے 11	> 12	13	14	15	16	17	18	1
65.0			See Borehole Log for Details															
64.5 64.4	-11		QUEENSTON FORMATION	1	В	F	VC	RU						1	92	0	90	
64.1 64.0	•		Shale with interbedded siltstone, and clay layers.		F	V	С	RP										
53.8 53.6 53.6			Shale (62%) thinly bedded or laminated, re low strength, alternating between heavily an slightly weathered to ~11.7 m and between	d, nd	F	V												7
63.1	-12		below.		B	F	C C	RP SU						2	100	78	100	
63.0 62.8 62.7	.		Limestone (11%) fine grained, grey, mediu strength, unweathered Siltstone (26%) fine grained, grey, medium		F	v												
-	-13		strength, unweatherĕd. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to								\square							
62.1 61.9			very close intervals. Vertical fractures were noted at ~ 11.0 m, 1 m, 12.5 m, 13.4 m and 14.2 m.		F	v	м	SU						3	100	87	100	
61.8 61.7 61.6	-		A Clay (1%) layer, heavily weathered, very strength was noted at ~13.5 m.	low	В	F	M	SP	NC	50 mm								
61.2 61.2 61.1 61.1	-14		A Rubble layer, heavily weathered, low strength was noted at ~10.8 m (80 mm).		F	v												
50.9 50.7 50.6	-15			1	в	F	VC C	SP SP						4	100	71	100	
59.9																		
59.8	-		End of Borehole at 15.4 m															
-	-16																	
	-17																	
	-18																	

roject:	Geotechnical Investigation												Shee	t No.	_1	of _
ocation:	Erin Mills Town Centre, 51	00 Erin	Mi	lls P	arkw	/ay,	MIs	siss	saug	a, O)nta	rio				
ata Dailla d	Jonuon 11, 2024		- ,	Auaer S	Sample			Σ	3				•	Reading	-	
ate Drilled:	January 11, 2024		- ;	SPT (N) Value	F 4		OB				Moistu and Liq		nit		< —0
rill Type:	Hollow Stem Augers Geodetic		-	Shelby	c Cone [·] Tube	lest						ed Tria n at Fai			е	Ð
atum:	Geodelic		_	Field Va	ane Test				5	Pe	enetro	meter				
Soil/Rock Symbol	Soil Description	ELEV. m 176.02	Depth (m)	Shear	20 Strength	SPT (N 40 100	Value 60		80 kPa		25	5 Iral Mois erg Limi	50	eading (p 75 ontent % Ory Weig 30	0	Natu Uni Weig kN/r
~90 FILL	mm ASPHALT over sand and gravel granular, n, moist	~175.5	0		ð					>	×					
FILL reddi	- clayey silt, trace sand, sh brown, moist	~~174.6	1	Ŏ	24							Ŷ×				
-some	DY SILT to SILTY SAND - e sand pockets, trace clay, trace	_	2		Ö							×				
SIL	el, brown, moist, compact TILL - some clay, trace sand,	~173.5				47 C	,)				×	×				
reddi	sh brown, dense to very dense	_	3						93/225 C	nim	×					
			4													
- trac	e shale fragments	_~171.4	-				5 mm				~					7
 SHA	Coring Commenced	4	5				,									1
	nation, occasional limestone and stone layers, reddish brown	_														
		-	6													
			7													
		_														
		_	8													
		_														
			9													
	Saa Cara Lag far Dataila	_	10													
	See Core Log for Details	_														
		_	11													
		_														
			12													
		_	13													
		_														
		-	14													
			15													
		_~160.5	15													
	End of Borehole															
										Ela	apsed Fime			Water Level	н	lole Ope to (m)
otes:					е Э	V	r						_	(m)		ω (III)

LOCA 510 CLIEN	otechni TION 0 Erin IT	Mills P	ROCK CORE restigation Parkway, Mississauga, Ontario p of COmpanies	ORIEN Verti DATE 01/1 DRILL	iTATIC cal STAR 1/23 ER s Drilli	TED		176.0 01/12 01/12 0RILL) LETE 2/23 TYP 5 - T	E E		DATUM Geodet LOGGEE D. Pan CORE B HQ	tic) BY chal		PRO. GTF DRAN	JECT R-002 WING 7/	5776 NUN A	9-ŀ
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS			SPACING		EILLING	APERTURE 0 (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	عت 11	<u>></u> 12		14	15	16	17	18	
171.8	-		See Borehole Log for Details															T
171.4 171.1 171.1 170.9	-5			1	В	F	VC C	RU RP						1	100	88	95	
170.9 170.7 170.6 170.1 169.9 169.8				, d 1	B F F	F V V	с с	RP RP						2	100	73	100	
169.6 169.5			Siltstone (8%) fine grained, grey, medium strength, unweathered.		F	v												
169.4 169.3	-		Discontinuities: bedding joints are rough		F	v												
169.1	-7		very close intervals. Vertical fractures were noted at ~6.0 m, 6.2 6.5 m, 6.9 m, 7.2 m, 7.4 m, 8.2 m, 10.1 m ar	m,	F	v												
168.4 168.4	- - - -		6.5 m, 6.9 m, 7.2 m, 7.4 m, 8.2 m, 10.1 m ar 12.1 m. Rubble layers, heavily weathered, very low strength were noted at ~9.9 m (80 mm) and 10.3 m (160 mm).	1	B F	F V	C C	RP RP						3	100	70	100	
168.0 168.0 167.9 167.8	-				F	v												
167.4 167.3	- - - - - - 9			1	в	F	C M	RP SU						4	100	97	100	
166.9 166.8	- - - -																	
166.1 166.1	-10				_													
165.8 165.7					F		-							_ _	100	70	100	
165.4 165.3	-			1	В	F	C M	SU SU						5	100	73	100	
165.0 164.9	-11																	
164.7 164.6	-																	
164.4 164.2	-				_													
164.1	-12			1	F B	V F	М	SU						6	100	92	100	

			ROCK CORE											В	Η	20			
PROJE			actigation	-			ON		LEVA 176.0		1 (m)		ATUM	ia				NUM	
			estigation		ertica TE S	TAR	TED		OMP		D		Geodet					57769 NUM	
		Mills P	arkway, Mississauga, Ontario		1/11/				01/12				D. Pano			0	7/	4	
The N		o Grou	p of COmpanies			Drilli	<u> </u>	С	RILL ME 5	5 - Ti	rack		ore B/ HQ			SHEI	ET 2 0	f 2	
(E)								RÁC	TERI တ	STIC		ല		<u> </u>	ĸ	(%)		(%)	
ELEVATION (m)	DEPTH (m)	BOL			OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	Ű	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY		WATER RECOVERY (%)	
ELEV	DEP1	SYMBOL	GENERAL DESCRIPTION		NO. 0	NIOL	ORIE	SPAG	ROU	FILLING	APEF (mm)	WEA	STRE	FRAG		RECO	RQD	WAT	
1	2	3	4		5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
63.8 63.7 63.6			QUEENSTON FORMATION					М	SP										
-			Shale with interbedded siltstone, and clay layers.																
62.5	-13		Shale (86%) thinly bedded or laminated, red low strength, alternating between heavily ar slightly weathered to ~5.5 m and between moderately weathered and unweathered below.	d, Id	1	В	F	w	SP SP						7	100	100	100	
62.4 62.2 62.1			Limestone (4%) fine grained, grey, medium strength, unweathered																
61.8	-14		Siltstone (8%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough																
51.6- 51.5 51.4-			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~6.0 m, 6.2	m															
51.0 60.9	15		6.5 m, 6.9 m, 7.2 m, 7.4 m, 8.2 m, 10.1 m a 12.1 m.	nd	1	В	F	W	SP SP						8	100	100	100	
60.5			Rubble layers, heavily weathered, very low strength were noted at ~9.9 m (80 mm) and 10.3 m (160 mm).																
00.0			End of Borehole at 15.6 m																
-	-16																		
	-17																		
	-18																		
	-19																		
	-20																		

oject:	Geotechnical Investigation									heet No.		of _1
ocation:	Erin Mills Town Centre, 51	00 Erin	Mi	lls Parl	way,	MIssis	sauga	, Onta	ario			
			-	Auger Samp			\boxtimes			pour Reading		
ate Drilled:	January 10 - 11, 2024		- :	SPT (N) Val	ue	0			Moisture and Liqu		X	0
rill Type:	Hollow Stem Augers		_	Dynamic Co Shelby Tube					ned Triax n at Failu		ŧ	,
atum:	Geodetic		_	Field Vane 1	est	I	S	Penetro	ometer			
Soil/Rock Symbol	Soil Description	ELEV.	Depth (m)	20 Shear Stre	SPT (N 40 ngth 100	Value) 60	80 kPa 200	2	5 5 ural Moist erg Limits	our Reading (ppm) 50 75 ure Content % % \$ (% Dry Weight) 20 20 30	Sample	Natura Unit Weigh kN/m
	mm TOPSOIL over	176.12	0	Ô			200			×		21.4
reddi 💥	sh brown, moist	~175.1	1	21					Ŷ			
SIL I reddi	TILL - some clay, trace sand, sh brown, dense to very dense	_			48 Č							
_		-	2		50/15			×				
	a abala fragmanta	- 470.4			50/12			×				
	ce shale fragments Coring Commenced	~173.1	3		Č	>		×			Z	
Form	LE BDEROCK - Queenston nation, occasional limestone and stone layers, reddish brown	_	4									
	stone layers, reduisit brown	-										
		-	5									
		-										
]	6									
		_	7									
		-										
		-	8									
		1										
	See Core Log for Details		9									
		_	10									
		-										
		-	11									
]	12									
		_										
		-	13									
		-										
			14									
		_	15									
	End of Borehole	~160.6										
					• • •			Elapseo Time		Water Level (m)		ole Oper to (m)
	to completion at ~15.5 m depth by conventional soil samplir			* (ЭХ	p.		nuary 29, pruary 4,		~6.1 ~7.4		Well Well
This drawing forms p	g subcontractor. For borehole definitions, see notes prior to part of and must be read in conjunction with the subject rep- orehole data requires interpretation assistance by exp profe	ort (Ref. No.:				1			_VL T			

LOCATION 5100 Erin Mills Parkway, Mississauga, OntarioDATE STARTEDCOMPLETEDLOCGED BY 0.1/11/23D. PanchalCOLLENT The Muzzo Group of COmpaniesDRILLER DRILLER DUINT CHARACTERISTICSLOGGED BY 0.1/11/23LOGGED BY 0.PanchalD. Panchal 0.Panchal(E) (E)(C) (E) (E) (E) (E) (E) (E) (E) (E)(D) (E) <br< th=""><th>PROJE Geot</th><th></th><th>ical Inv</th><th>ROCK CORE</th><th>ORIEN</th><th>TATIC</th><th></th><th>E</th><th>LEVA 176.1</th><th></th><th>N (m)</th><th></th><th>DATUM Geodel</th><th></th><th></th><th>20'</th><th></th><th></th><th></th></br<>	PROJE Geot		ical Inv	ROCK CORE	ORIEN	TATIC		E	LEVA 176.1		N (m)		DATUM Geodel			20'			
The Muzzo Group of COmpaniesDavis DrillingCME 55 - TrackHQ1 (1) <t< th=""><th>LOCA1 5100</th><th>FION Erin</th><th></th><th><u> </u></th><th>DATE 01/11</th><th>STAR /23</th><th>TED</th><th></th><th>OMP 01/11</th><th>LETE /23</th><th></th><th></th><th>D. Pan</th><th>) BY chal</th><th></th><th>DRA</th><th>WING 8/</th><th>NUN</th><th></th></t<>	LOCA1 5100	FION Erin		<u> </u>	DATE 01/11	STAR /23	TED		OMP 01/11	LETE /23			D. Pan) BY chal		DRA	WING 8/	NUN	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $			o Grou	p of COmpanies	Davis	Drilli		c	ME 5	5 - Ti	rack			1			1 o	f 2	
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 173.6 </th <th></th> <th></th> <th></th> <th>GENERAL DESCRIPTION</th> <th>NO. OF SETS</th> <th>JOINT TYPE</th> <th>ORIENTATION</th> <th>SPACING</th> <th>ROUGHNESS</th> <th>FILLING</th> <th>APERTURE (mm)</th> <th></th> <th></th> <th>FRACTURE FREQUENCY</th> <th></th> <th></th> <th>RQD</th> <th>WATER RECOVERY (%)</th> <th></th>				GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)			FRACTURE FREQUENCY			RQD	WATER RECOVERY (%)	
See Borehole Log for Details173.13QUEENSTON FORMATION1Shale with interbedded siltstone, and clay layers.134Shale (73%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered171.8Shale (73%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered171.8Imestone (3%) fine grained, grey, medium strength, unweathered.171.4Siltstone (24%) fine grained, grey, medium strength, unweathered.171.2Siltstone (24%) fine grained, grey, medium strength, unweathered.171.4Siltstone (24%) fine grained, grey, medium strength, unweathered.171.2Siltstone (24%) fine grained, grey, medium strength, unweathered.170.6Siltstone (24%) fine grained, grey, medium strength, unweathe	-	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	- - -	-3		.															
4 Shale (73%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered below. 1 B F C RP 171.8 Imestone (3%) fine grained, grey, medium strength, unweathered 1 B F C RP 171.8 Imestone (3%) fine grained, grey, medium strength, unweathered 1 B F V RP 171.4 Imestone (3%) fine grained, grey, medium strength, unweathered F V V Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered Imestone (3%) fine grained, grey, medium strength, unweathered F V Imestone (3%) fine grained, grey, medium strength, unweathered Imestone (3%) fine grained, grey, medium strength, unweathered Imestone (3%)	-			Shale with interbedded siltstone, and clay	1	В	F								1	100	100	95	
171.8 171.7 171.6Limestone (3%) fine grained, grey, medium strength, unweatheredFV171.6 171.4Siltstone (24%) fine grained, grey, medium strength, unweathered.FV171.7 171.4Discontinuities: bedding joints are rough 	F	-4		Shale (73%) thinly bedded or laminated, re low strength, alternating between heavily ar slightly weathered to ~3.4 m and between moderately weathered and unweathered	d, nd 1	В	F								2	100	82	100	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	171.7 171.6 171.4			-											-				
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	171.2	-5		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.			-												
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	170.4 170.0 170.0	-6		Vertical fractures were noted at ~4.5 m, 4.8 5.0 m, 5.3 m, 5.6 m, 6.8 m, 9.2 m, 12.4 m, 13.4 m and 14.8 m.	[.] m, 1										3	100	85	100	
168.9 1 B F C SU 4 100 93	169.6 169.4 169.2	-7				F	v												
	169.0 168.9 168.8 168.6 168.5				1	В	F		SU SU						4	100	93	100	
168.4	- 168.0	-8																	
167.2 167.1 167.0 166.9 FV	167.5 167.2 167.1 167.0 166.9	-9			1				SU SP						5	100	97	100	
166.7 166.2 166.2 166.2 165.9 165.8 1 B F W SP 1 B F W SP 1 D 6 100 10	166.7 166.2 166.2 165.9	-10			1	R	F	\\\/	SP						6	100	100	100	

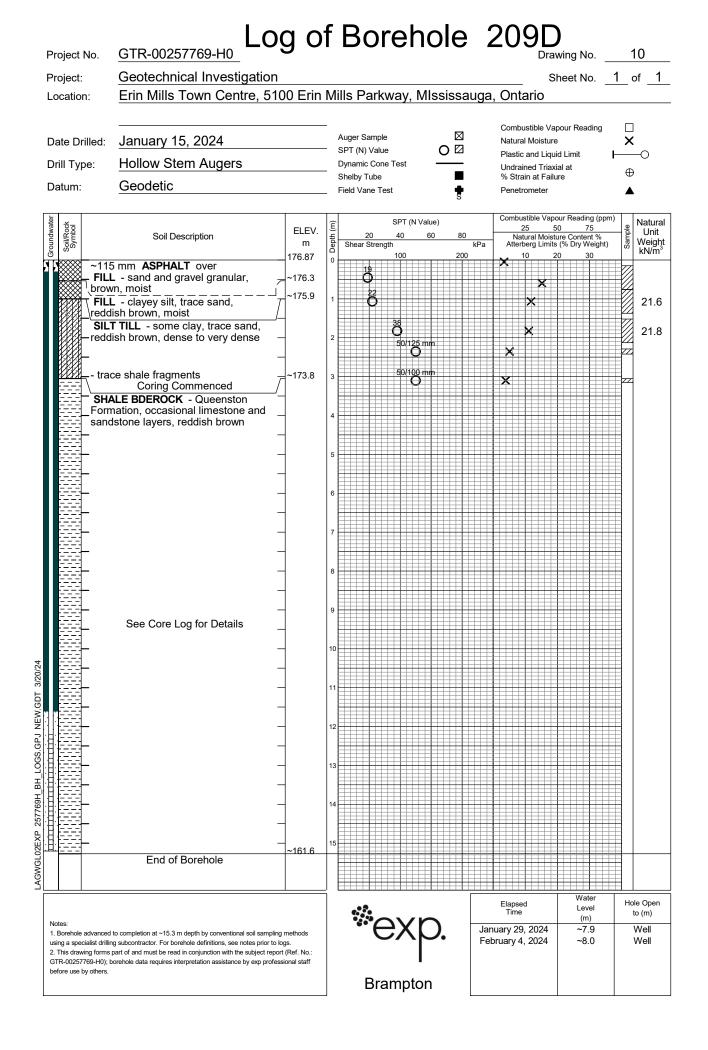
			ROCK CORE										B	H	20			
PROJI			estigation	ORIEN Verti		ON	E	LEV 176.		N (m)		ATUM Geodel	tio				NUM	
			esugauon	DATE		RTED	c	OMP		D	L	OGGE					NUN	
		Mills P	arkway, Mississauga, Ontario	01/1				01/1				D. Pan					A	
CLIEN The		o Grou	p of COmpanies	Davi	. ER s Drill	ina		ME 5				HQ	ARREL	-	SHEI	ЕТ 2 о	of 2	
	WIGEE					Ţ CH/												
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
ш 1	2	<i>о</i> 3	4	Z 5	ר 6	7	8 8	9	LL 10	∢ ڪ 11	<u>></u> 12	0 13	14	15	16	17	<u>>⊯</u> 18	1
-			QUEENSTON FORMATION Shale with interbedded siltstone, and clay				W	SP										
64.9 64.8 64.5 64.4 64.3	- - - - -		Shale (73%) thinly bedded or laminated, rec low strength, alternating between heavily an slightly weathered to ~3.4 m and between moderately weathered and unweathered below.											7	100	93	100	-
64.1 64.0 63.9 63.8 63.8	-		Limestone (3%) fine grained, grey, medium strength, unweathered Siltstone (24%) fine grained, grey, medium strength, unweathered.	1	В	F	W C	SP SU										
63.7 63.5	-13		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~4.5 m, 4.8	m,														
52.8 52.7 52.6	- - - - - - - - - - - - - - -		5.0 m, 5.3 m, 5.6 m, 6.8 m, 9.2 m, 12.4 m, 13.4 m and 14.8 m.	1	B F	FV	M M	SU SP						8	100	93	100	-
61.7 61.5 61.4 61.3	- - - - - - - - - - - - 15			1	F	V	M	SP						9	100	95	100	-
61.0 60.9 60.6	- 13 - - - - - -		End of Borehole at 15.5 m				M	SP										
- - - - -	-16																	
-	-17																	
- - - - - - - - - - - - - 																		

Project No.	GTR-00257769-H0 Geotechnical Investigatior	og o									Dra	awing			9 .f 1
Project: Location:	Erin Mills Town Centre, 51		М	ills	Park	Nav.	MI	ssiss	sauda	. Ont		Sneet	No	<u> </u>	or <u>I</u>
						, ,				,					
Date Drilled:	January 10, 2024			-	Sample						ustible V Il Moistu	apour Re re	eading	×	
Drill Type:	Hollow Stem Augers				N) Value nic Cone				2 -		and Liq	uid Limit xial at	I		-0
Datum:	Geodetic				y Tube √ane Te	st				% Stra	in at Fai ometer			€)
				1				Ę	5	Combin			-l'		
Groundwate Soil/Rock Symbol	Soil Description	ELEV. m 176.52	Depth (m)	She	20 ar Streng	40	N Valu 60)	80 kPa 200	Nat Atter	25 tural Mois	50 sture Con ts (% Dry 20	ding (ppm) 75 tent % Weight) 30	Sample	Natural Unit Weight kN/m ³
~10		~176.3	0	ð								X			
FIL	L - clayey silt, trace sand,	~175.4	1		18							x			
	Soil Description ~100 mm PEBBLE over ~125 mm TOPSOIL FILL - clayey silt, trace sand, reddish brown, moist SILT TILL - some clay, trace sand, reddish brown, very dense - trace shale fragments Coring Commenced SHALE BDEROCK - Queenston Formation, occasional limestone and sandstone layers, reddish brown See Core Log for Details	4			Ŭ			74/250	mm		×				22.0
		_	2			50/1	25 mr				×				
- tra		-					0			×				Z	
	Coring Commenced	~173.3	3			50/2	25 mm O			×				Z	
	ALE BDEROCK - Queenston	/													
			4												
		_	5												
		_													
	5 mm TOPSOIL L - clayey silt, trace sand, lish brown, moist T TILL - some clay, trace sand, lish brown, very dense ce shale fragments Coring Commenced ALE BDEROCK - Queenston nation, occasional limestone and dstone layers, reddish brown	_	6												
	5 mm TOPSOIL L - clayey silt, trace sand, tish brown, moist T TILL - some clay, trace sand, tish brown, very dense ce shale fragments Coring Commenced ALE BDEROCK - Queenston nation, occasional limestone and dstone layers, reddish brown	-													
		-	7												
		-													
		-	8												
			9												
		_													
	See Core Log for Details	_	10												
		_													
		-	11												
		-													
		-	12												
		-													
		-	13												
			14												
		_													
		_	15												
		~160.8													
	End of Borehole														
]	•						Elapse Time		L	/ater evel		ole Open to (m)
using a specialist dril 2. This drawing form	d to completion at ~15.7 m depth by conventional soil sampli ling subcontractor. For borehole definitions, see notes prior to s part of and must be read in conjunction with the subject rep borehole data requires interpretation assistance by exp profe	o logs. ort (Ref. No.:		•	Bra								(m)		

			ROCK CORE										В		20			
PROJE		iocl In		ORIEN		ON	E			N (m)		DATUM	lio				NUM	
		icai inv	estigation	Vertic		TED	c	176.5 OMP		D	-	Geode					5776 NUN	
		Mills P	arkway, Mississauga, Ontario	01/10	/23			01/10)/23			D. Pan	chal			9		_
				DRILLE				RILL			0	CORE B	ARREL		SHE			
The	IVIUZZ	o Grou	p of COmpanies	Davis		0		ME 5				HQ				1 0	or 2 ∣	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12		14	15	16	17	18	
73.8	-3		See Borehole Log for Details															
73.3	-		QUEENSTON FORMATION															
73.2 73.2	-			1	В	F	C C	RU RU						1	100	53	100	
72.9 72.8	-		layers.															
72.5 72.5 72.4 72.3			Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below.	1	B F	FV	C C	RU RP						2	100	88	100	
71.9	-		Limestone (3%) fine grained, grey, medium strength, unweathered		F	v												
71.9 71.7			-															
71.6 71.3 71.2	-5		Siltstone (9%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough															
70.9 70.8	- -		planar to smooth undulating and at wide to very close intervals.	٦,														.
70.5	-		Vertical fractures were noted at ~4.4 m, 4.7 m 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m.		В	F	M C	RP RP						3	100	93	100	1
70.4 70.1 70.1	-		ouongui wao notou at - 10.7 III.		F	v												
69.8	-																	
69.6	-																	
-	-1				F	V												
69.2	-			1	в	F	с	RP						4	100	70	100	
69.1 69.0	-						М	SU										['
68.9 68.8	-																	
68.8	8																	
68.7	-																	
68.1 68.0	-				F	v												
-	-																	
67.8 67.6				1	В	F	С	SU						5	100	95	100	·
F	-9						М	SP										
67.3 67.3	-																	
-	-																	
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	-10																	
66.5 66.3																		
-	-			1	в	F	w	SP						6	100	97	100	.
t	-	1					М	SP										1

[®] * exp.

Mills P	estigation arkway, Mississauga, Ontario p of COmpanies	ORIEN Verti DATE 01/1 DRILL	cal STAF 0/23	-		176.5 0MP	5			ATUM Geodet				R-002	5776	
Mills P	arkway, Mississauga, Ontario	DATE 01/1	STAF 0/23	RTED	c											J-1
		01/1	0/23					=D	- L(OGGEE) BY		DRA\	WING	NUM	IBE
o Grou	p of COmpanies	DRILL				01/10	0/23			D. Pan				9/		
		D 1				DRILL CME 5				ore B / HQ	ARREL		SHE	ET 2 o	<u>ــــــــــــــــــــــــــــــــــــ</u>	
		Davi	s Drill JOIN	T CH										20	12	
SYMBOL	GENERAL DESCRIPTION	Р	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	-ILLING	APERTURE mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	ROD	WATER RECOVERY (%)	
3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
h	QUEENSTON FORMATION		F	V		-			Ш							\vdash
			1	.												
	Shale with interbedded siltstone, and clay layers.															
	below.	1	В	F	W	SP						7	100	100	100	
		1	в	F	W	SP						8	100	100	100	
	strength was noted at ~15.7 m.				vv											
			F	V												
		1	В	F	M VC	SP SU						9	100	93	100	
							NC	50								
	End of Borehole at 15.7 m							 								
		3 4 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, recommoderately weathered to ~4.1 m and between moderately weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~4.4 m, 4.7 A Clay (1%) layer, heavily weathered, very lastrength was noted at ~15.7 m.	Ogg GENERAL DESCRIPTION Use 3 4 5 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below. 1 Limestone (3%) fine grained, grey, medium strength, unweathered. 1 Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1	Image: Second system Image: Second system Image: Second system Image: Second system 3 4 5 6 Image: Second system QUEENSTON FORMATION 1 F Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered to ~4.1 m and between moderately weathered and unweathered below. 1 B Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B Siltstone (9%) fine grained, grey, medium strength, unweathered. 1 B Uimestone (1%) fine grained, grey, medium strength, unweathered. 1 B Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1 B	3 4 5 6 7 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below. 1 B F Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1 B F	3 4 5 6 7 8 QUEENSTON FORMATION I F V Shale with interbedded siltstone, and clay layers. F V Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and sightly weathered or 4.1 m and between moderately weathered and unweathered below. 1 B F W Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F W Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F W HIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	3 4 5 6 7 8 9 QUEENSTON FORMATION F V V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below. 1 B F W SP Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F W SP Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W SP Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F W SP MIIIII A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1 B F W SP MIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	3 4 5 6 7 8 9 10 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale with interbedded siltstone, and clay layers. Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered 1 B F W SP Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W SP Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F W SP M Siltstone (1%) layer, heavily weathered, very low strength was noted at ~15.7 m.	3 4 5 6 7 8 9 10 11 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F W SP Siltstone (9%) fine grained, grey, medium strength, unweathered. 1 B F W SP Uncertain fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F W SP Image: the off Borebole at ~15.7 m. 1 B F W SP Image: the off Borebole at 15.7 m. 1 B F M SP	3 4 5 6 7 8 9 10 11 12 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered to ~4.1 m and between moderately weathered to ~4.1 m and between moderately weathered. 1 B F W SP 2 Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F W SP 2 Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W SP 4 A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1 B F W SP 1 B F W SP SP SP SP 4 1 B F W SP SP SP 4 1 B F W SP SP SP SP SP SP	3 4 5 6 7 8 9 10 11 12 13 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between moderately weathered and unweathered below. Limestone (3%) fine grained, grey, medium strength, unweathered. 1 B F W SP Siltstone (9%) fine grained, grey, medium strength, unweathered. 1 B F W SP Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very low strength was noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and 14.6 m. 1 B F W SP A Clay (1%) layer, heavily weathered, very low strength was noted at ~15.7 m. 1 B F W SP I B F V SP I I B F VC SU	3 4 5 6 7 8 9 10 11 12 13 14 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.1 m and between heavily and slightly weathered to ~4.1 m and between below. 1 B F W SP U	3 4 5 6 7 8 9 10 11 12 13 14 15 QUEENSTON FORMATION F V F F V F V F V F F F V <td>$\frac{1}{0} \frac{1}{0} \frac{1}$</td> <td>3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered below. 1 B F W SP 4 4 7 100 100 1 B F W SP 4 4 4 7 100 100 Unestone (3%) fine grained, grey, medium strength, unweathered 1 B F W SP 4 4 4 100 100 Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W SP 8 100 100 Sitistone (9%) fine grained, grey, medium strength, unweathered. 1 B F W SP 8 100 100 Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and</td> <td>3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered on <</td> F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V V F V SP V SP V SP V SP V SP V SP SP SP SP SP	$\frac{1}{0} \frac{1}{0} \frac{1}$	3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 QUEENSTON FORMATION F V Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered and unweathered below. 1 B F W SP 4 4 7 100 100 1 B F W SP 4 4 4 7 100 100 Unestone (3%) fine grained, grey, medium strength, unweathered 1 B F W SP 4 4 4 100 100 Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals. 1 B F W SP 8 100 100 Sitistone (9%) fine grained, grey, medium strength, unweathered. 1 B F W SP 8 100 100 Vertical fractures were noted at ~4.4 m, 4.7 m, 6.4 m, 7.1 m, 8.4 m, 10.8 m and	3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 QUEENSTON FORMATION Shale with interbedded siltstone, and clay layers. Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered on <



			ROCK CORE										B	12	209	U		
PROJE				ORIEN		NC	E			N (m)		DATUM			PRO			
Geot		ical Inv	estigation	Vertic		TED		176.9 OMP	-	-	_	Geode LOGGEI			GTF DRA		5776	
		Mills P	arkway, Mississauga, Ontario	01/15		IED		01/1				D. Pan			DRA	10 NING		ıD
CLIEN				DRILLE				RILL		E	+	CORE B			SHE	-		
The	Muzz	o Grou	p of COmpanies	Davis				ME 5				HQ				1 0	f 2	
(E)				s			ARAC		STIC		ŋ		×	ER	(%)		(%)	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SET	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING		FRACTURE	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	2 13	14	15	16	17	18	
74.4	-										Π							t
70.0	-3		See Borehole Log for Details															
73.8 73.7	5		QUEENSTON FORMATION		_													t
73.7				1	F B	V F	VC	RU						1	100	68	95	
73.4 73.3	-		Shale with interbedded siltstone, and clay layers.		F	V	С	RU										
73.0 73.0	-4		Shale (98%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~4.4 m and between moderately weathered and unweathered															
72.7 72.6	•		moderately weathered and unweathered below.	1	F	V F	с	RU						2	100	92	100	
-	-		Limestone (1%) fine grained, grey, medium strength, unweathered				М	RP										
-	-5		Siltstone (1%) fine grained, grey, medium strength, unweathered.															
71.5			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															t
71.5- 71.4			Vertical fractures were noted at ~3.2 m, 3.5 m 4.2 m, 6.2 m and 6.7 m.	n,														
71.1 71.0	-6			1	в	F	M C	RP RP						3	100	98	100	
70.7 70.6					F	v					1			•				
70.5 70.4																		
70.2 70.0	_				F	V												
-	-7																	
69.6 69.4				1	В	F	C C	RP SU						4	100	97	100	
69.0																		
68.9	-8																	
68.8 68.7 68.5	-																	
68.3 68.3																		
-	-9			1	В	F	W W	SU SP						5	100	100	100	
67.5																		
67.3 67.1	-																	
67.1 67.1 66.8	-10																	
66.8																		
66.5				1	в	F	М	SP						6	100	100	100	

			ROCK CORE										В		209			
PROJE		ical Inv	estigation	ORIEN Verti		ON	E	176.9		N (m)	1	DATUM Geode	tic				NUM 5776	
			esugation	DATE		TED	c	COMP		D	1	OGGGE					NUN	
		Mills P	arkway, Mississauga, Ontario	01/1				01/1		_		D. Pan			0.15		A	
CLIEN The		o Grou	p of COmpanies	DRILL Davi	ER s Drill	ing		DRILL CME 5			6	HQ	ARRE	_	SHE	Е Г 2 с	f 2	
(m)					JOIN		ARÁC		STIC	S	U			R	(%)		(%)	
ELEVATION (m)	DEPTH (m)	BOL		OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	Ðv	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE	RUN NUMBER	RECOVERY		WATER RECOVERY (%)	
ELEV	DEP1	SYMBOL	GENERAL DESCRIPTION	NO.	NIOC	ORIE	SPAG	ROU	FILLING	APEF (mm)	WEA	STRE	FRAC	RUN	RECO	RQD	WAT	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
66.4 66.2 66.1	-		QUEENSTON FORMATION				М	SP										
65.9	-11																	
65.7 65.5			Shale with interbedded siltstone, and clay layers.															
65.4 65.1	-		Shale (98%) thinly bedded or laminated, red low strength, alternating between heavily an slightly weathered to ~4.4 m and between moderately weathered and unweathered	d														
65.0	-12		below.	1	В	F	M M	SP SP						7	100	97	100	
64.6 64.5			Limestone (1%) fine grained, grey, medium strength, unweathered															
64.3 64.1	-		Siltstone (1%) fine grained, grey, medium strength, unweathered.															
63.9	-13		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
53.7			Vertical fractures were noted at ~3.2 m, 3.5 4.2 m, 6.2 m and 6.7 m.															-
63.3	-			1	B	F	W	SP SP						8	100	100	100	-
63.1	-14																	
62.6 62.5																		┝
62.4 62.3	-			4	В	F	w	SP						q	100	100	100	
-	-15			1			W	SP						9	100	100	100	(
61.6			End of Borehole at 15.3 m															_
-																		
-																		
-	-16																	
	-17																	
	-18																	
•••																		

Project No.	<u>дтк-00257769-но</u> Log о	Borehole 209S	11
Project: Location:	Geotechnical Investigation	Sheet No1 fills Parkway, MIssissauga, Ontario	
Date Drilled:	January 15, 2024	Auger Sample Combustible Vapour Reading SPT (N) Value D Z Plastic and Liquid Limit	×
Drill Type:	Hollow Stem Augers	Dynamic Cone Test Undrained Triaxial at Shelby Tube % Strain at Failure	⊕
Datum:	Geodetic	Field Vane Test S Penetrometer	▲
Groundwater Soil/Rock Symbol	Soil Description ELEV. m 176.87	SPT (N Value) Combustible Vapour Reading (ppm) 20 40 60 80 Shear Strength kPa Natural Moisture Content % Atterberg Limits (% Dry Weight) 100 200 10 20	Natural Unit Weight kN/m ³
	See Log of Borehole 209D for Details End of Borehole End of Borehole		
		Elapsed Vater	Hole Open
a specialist drilling sub 2. This drawing forms	I to completion at ~6.1 m depth by conventional soil sampling methods using contractor. For borehole definitions, see notes prior to logs. part of and must be read in conjunction with the subject report (Ref. No.: borehole data requires interpretation assistance by exp professional staff	Elapsed Time Level (m) January 29, 2024 No Free Water February 4, 2024 No Free Water Brampton Image: Construction of the second s	Well

oject:	Geotechnical Investigation									Shee	t No. 1	(of 1
cation:	Erin Mills Town Centre, 51		Μ	ills I	Parkv	vay,	, MIssis	sauga	, Ontar			_	
			_						Combust	ble Vapour	Reading		1
te Drilled:	<u>January 8 - 9, 2024</u>		_	•	Sample N) Value			⊠ ∅	Natural N	loisture	-	X	
ll Type:	Hollow Stem Augers		_		nic Cone		<u> </u>			nd Liquid Lin d Triaxial at	=	¢	_0
tum:	Geodetic		_		/ Tube /ane Tes	st		■ ★	% Strain Penetrom				
			-	1				5	Combustit	le Vanour R	eading (ppm)		
Soil/Rock Symbol	Soil Description	ELEV. m	Depth (m)	Shea	20 ar Strengt	40	(N Value) 60	80 kPa	25 Natura	50 al Moisture C g Limits (% E	75 ontent %	Sample	Natur Unit Weigl
	0 mm TOPSOIL over	176.76	0	Z		100		200	10	20	30		kN/m
🔆 – Fili	L - clayey silt, trace sand, lish brown, moist	_~176.0		O						×			21.4
SIL	T TILL - some clay, trace sand,	7	1				Ö		×				
redd	lish brown, very dense	-					Ö min		×				
		-	2			50/2	25 mm						
- tra	ce shale fragments	-					0		×				
	Coring Commenced	~173.6	3			- 00, 1	00 mm O		×				
	ALE BDEROCK - Queenston nation, occasional limestone and	1											
	dstone layers, reddish brown	1	4										
		-											
			5										
		-											
		1	6										
		1											
			ľ										
			8										
			°										
			5										
	See Core Log for Details		10										
			11										
		_											
		_	12										
		_											
		_	13										
		_											
		4	14										
		4											
		_	15										
		~161.0											
	End of Borehole												
I			Ī						Elapsed		Water Level		ble Oper
g a specialist drillir his drawing forms	I to completion at ~15.7 m depth by conventional soil samplin ng subcontractor. For borehole definitions, see notes prior to part of and must be read in conjunction with the subject repo borehole data requires interpretation assistance by exp profe	logs. ort (Ref. No.:		•	*e	X	(p.		Time		(m)		to (m)

			ROCK CORE	LC	JG	j							Б	H	21	U		
PROJI	ЕСТ			ORIEN			E	LEVA		l (m)		DATUM			PRO	JECT	NUM	B
		ical Inve	estigation	Vertic				176.8				Geodet				R-002		
OCA				DATE S		TED		OMP		D	Ī	LOGGED			DRA			iΒ
5100 LIEN		Mills P	arkway, Mississauga, Ontario	01/09				01/09		_	+	D. Pan CORE B			SHE	12	A	
		o Groui	p of COmpanies	Davis		na		ME 5				HQ	ARREL	-	SHEI	= i 1 o	fo	
ine	wiuzz															10	. 2	Г
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12		14	15	16	17	18	
74.3	-																	
- - - - -	-3		See Borehole Log for Details															
73.6 73.5	-		QUEENSTON FORMATION											4	100	100	05	
73.4	-			1	В	F	C	RU						1	100	100	95	
73.4	-	ШШ	Shale with interbedded siltstone, and clay		F	V	С	RP										
73.3 73.1	-		layers.															
72.8-72.7	- 4 - - -		low strength, alternating between heavily and slightly weathered to ~4.5 m and between moderately weathered and unweathered below.	1	В	F	C M	RP SU						2	100	100	100	
F	_	1	Limestone (7%) fine grained, grey, medium strength, unweathered															
71.9 71.7	5		Sitestone (15%) fine grained, grey, medium strength, unweathered.															
71.3 71.2	-		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
70.8	6		Vertical fractures were noted at ~3.6 m and 13.5 m.	1	в	F	M M	SU SP						3	100	100	100	
70.7	- - - -																	
70.0 69.9 69.8	-7																	
69.7 69.3	-			1	В	F	M W	SP SP						4	100	100	100	
	-	臣井																
69.0	-																	
68.7 68.7 68.5 68.4																		
67.8 67.7	- 9			1	В	F	W W	SP SP						5	100	100	100	
67.1 67.1 67.0 66.9	- - - - - - - - - - - - - - - - - - -																	
66.4	-			1	В	F	W W	SP SP						6	100	100	100	
JU.4	-			1														

			ROCK CORE										E	H	21			
PROJE			<i></i>	ORIEN		ON	E			N (m)					PRO			
Geoi		cal Inv	estigation	Vertic		TED		176.8		-D		Geodet				R-002 WING		
		Mills P	arkway, Mississauga, Ontario	01/09				01/09				D. Pan				12		
CLIEN			,	DRILLI				RILL		E		ORE B		-	SHE			
The	Muzz	o Grou	p of COmpanies	Davis								HQ				2 0	f 2	
(m) v				S			ARAC		STIC		NG	_	⊁	ER	(%) /		(%) /	
EVATION (m)	DEPTH (m)	SYMBOL		OF SET	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	
ELE	DEF	SYA	GENERAL DESCRIPTION	N	l P	ORI	SP/	ROI	E	APE (mn	ΝĒ	STF	FR/	RU	REC	RQD	REA	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
66.3	-																	
65.9			QUEENSTON FORMATION															
0F 0	-11	<u> </u>																
F		E	Shale with interbedded siltstone, and clay layers.															
F				.														
65.2	-		Shale (78%) thinly bedded or laminated, red low strength, alternating between heavily and slightly weathered to ~4.5 m and between moderately weathered and unweathered	ģ														
65.2			moderately weathered and unweathered	4	_	F	14/	00						-	100	100	100	-
64.9 64.8	-12		below.	1	В	-	W	SP SP						7	100	100		
υ4.ŏ		E	Limestone (7%) fine grained, grey, medium strength, unweathered															
-			Siltstone (15%) fine grained, grey, medium strength, unweathered.		F	v												
64.0			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.															
63.8	-12		very close intervals.															
	13		Vertical fractures were noted at ~3.6 m and															
Ē		E	13.5 m.	1	в	F	М	SP						8	100	100	100	
63.3 63.2	-	ЩЩ				.	M	SP										0
63.1-																		
F																		
62.6	-14																	
-																		
	-	EEE																
62.2 62.1																		
		EE		1	в	F	w	SP						9	100	100	100	
F	-15						Ŵ	SP SP						9	100	100		
61.6																		
61.4	-																	
61.0	. <u> </u>																	
01.0			End of Borehole at 15.7 m															
E	-16																	
F																		
F																		
F	-																	
F																		
F	-17																	
+																		
ļ																		
þ	-																	
Ļ																		
ŀ	-18																	
E	10																	
E																		
• 2-							L											
	21	In	`															
			1															

Proje		GTR-00257769-HU	-	Borehole 211 Drawing No.	13
Proje Locat		Geotechnical Investigation		Sheet No ills Parkway, MIssissauga, Ontario	<u>1</u> of <u>1</u>
Local	1011.			inio Fantway, mississauga, omano	
Date	Drilled:	January 8, 2024		Auger Sample Combustible Vapour Reading Natural Moisture	□ ×
Drill T	ype:	Hollow Stem Augers		SPT (N) Value O IZ Plastic and Liquid Limit Dynamic Cone Test Undrained Triaxial at	⊢O
Datur	n:	Geodetic		Shelby Tube % Strain at Failure Field Vane Test Penetrometer	⊕
5				S Combustible Vapour Reading (pp	m)
Groundwate	6	Soil Description	ELEV. m 176.86	SPT (N Value) Controlstatile Vapour Reading (pp) 20 40 60 80 Shear Strength kPa Natural Moisture Content % Atterberg Limits (% Dry Weight 100 200 10 20	
	FILI	0 mm TOPSOIL over L - clayey silt, trace sand, lish brown, moist	~175.9	Å X	21.4
	SIL _redd	T TILL - some clay, trace sand, lish brown, very dense	_	OX S0/150 mm OX X	
	- trac	ce shale fragments Coring Commenced		50/100 mm O 50/150 mm X	
		ALE BDEROCK - Queenston nation, occasional limestone and	4		
2577 257709H BH_LOGS.GPJ NEW.GDI 372024		dstone layers, reddish brown			
		End of Borehole	~161.5		
			L	Elapsed Time Level	Hole Open to (m)
using a s 2. This d GTR-00	specialist drillir Irawing forms	to completion at ~15.4 m depth by conventional soil samplin g subcontractor. For borehole definitions, see notes prior to part of and must be read in conjunction with the subject rep porehole data requires interpretation assistance by exp profe	logs. ort (Ref. No.:	Brampton	

PROJ	ECT		ROCK CORE							(m)		DATUM			21		NI 184	
		nical Inv	estigation	Vertic		JN		LEVA 176.9		v (m)		Geode	tic				NUM	
LOCA				DATE		TED	C	OMP		ED		LOGGE					S NUM	
		Mills P	arkway, Mississauga, Ontario	01/08				01/08				D. Par					BA	
CLIEN The		zo Grou	p of COmpanies	DRILLE Davis		na		ME 5				CORE E HQ	ARRE	L	SHE		of 2	
						TČH/		TERI							;;			
ELEVATION (m)	DEPTH (m)	SYMBOL		OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	DNG	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	()
ELEY	ЕР.	ΜΥ	GENERAL DESCRIPTION	NO.	NIOI	ORIE	SPA(SOU	FILLING	APEI M m	VEA	STRE	RAC	SUN SUN	SEC.	RQD	NAT	
1	2	3	4	5	6	7	8	9	10	11	12		14	15		17	18	
174.4	-					-					П							+
-	-		See Borehole Log for Details															
173.7	-3		-															
173.6	-		QUEENSTON FORMATION		_		_							4	04		00	ſ
173.5	-			1	В	F	C C	RU RP						1	94	94	90	
173.3	-		Shale with interbedded siltstone, and clay layers.		F	V									-			t
173.1	-4		Shale (87%) thinly bedded or laminated, red,															
-	-		Shale (87%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~5.8 and between moderately weathered and unweathered		F	V												
-	-		below.	1	В	F	С	RP						2	100	90	100	
-	-		Limestone (6%) fine grained, grey, medium strength, unweathered				М	SU										
-	-5		Siltstone (7%) fine grained, grey, medium strength, unweathered.															
-			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.											\mid	+	<u> </u>		╞
171.2	-		very close intervals. Vertical fractures were noted at ~3.6 m, 4.2 r	n														
171.2			and 6.9 m.	n 1	В	F	N.4	SU						3	100	97	100	
-	-6					-	M C	SU						3	100	91	100	
-	-																	
-															+	-	1	t
170.0 169.9					F	V												
169.9	-1				.													
169.4				1	в	F	м	su						4	100	100	100	
	-						М	SP										
169.2 169.0																		
168.9	-8																	
																		T
168.3	-																	
168.3	-																	
	Q			1	в	F	W	SP						5	100	100	100	
167.7							W	SP										
167.6																		
167.3 167.2	-																<u> </u>	
	-																	
166 7	-10																	
166.7 166.7	-																	
166.4	-													6	100	100	100	

			ROCK CORE										B	SH	21			
PROJI			actigation	ORIEN Verti		ON		LEVA 176.9		N (m)	D	ATUM Geodet	io		-		NUM 5776	
LOCA		icai inv	estigation	DATE		TED		OMP		D	L	OGGGEL					NUN	
5100	0 Erin	Mills P	arkway, Mississauga, Ontario	01/08	3/23			01/08	8/23			D. Pan	chal			13		
		o Grou	p of COmpanies	Davis	ER 3 Drilli	ina		ME 5				HQ	ARREI	-	SHE	ET 2 o	f 🤈	
	IVIUZZ															20		0
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
166.2	-		QUEENSTON FORMATION	1	В	F	W W	SP SP										
65.9 65.8	-11		Shale with interbedded siltstone, and clay layers.															
65.2 65.1	- - - - - -		Shale (87%) thinly bedded or laminated, red. low strength, alternating between heavily and slightly weathered to ~5.8 and between moderately weathered and unweathered below.		В	F	w	SP						7	100	100	100	Pod
64.8 64.8	- 1 2 - - -		Limestone (6%) fine grained, grey, medium strength, unweathered				W	SP										
64.1	 - - -		Siltstone (7%) fine grained, grey, medium strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to															
63.8	-13		very close intervals. Vertical fractures were noted at ~3.6 m, 4.2 i and 6.9 m.	m														
63.4 63.2	- - - - - - -			1	В	F	W W	SP SP						8	100	100	100	
62.8 62.8 62.3	- 14 - - - -																	
62.2 61.9 61.8	-15			1	В	F	W W	SP SP						9	100	100	100	
61.5	- - 		End of Borehole at 15.4 m															
-																		
- - - - - - - - - - - - - - - - - - -																		
-	- 																	

Project: Location:	Geotechnical Investigation Erin Mills Town Centre, 5 [°]		Mills Par	kway, MIssis	sauga		neet No. <u>1</u>	of1
Date Drilled:	January 5, 2024		- Auger Sam		\boxtimes	Combustible Vap Natural Moisture	•	□ ×
Drill Type:	Hollow Stem Augers		- SPT (N) Va Dynamic C	-	0	Plastic and Liqui	d Limit	0
Datum:	Geodetic		Shelby Tub Field Vane	e	S	Undrained Triaxi % Strain at Failu Penetrometer		⊕
Groundwater Soil/Rock Symbol	Soil Description	ELEV.	(iii) 20 Shear Str	-	80 kPa	Combustible Vapo 25 5 Natural Moistu Atterberg Limits	0 75 ure Content % (% Dry Weight)	Natura Unit Weight kN/m ³
~12	5 mm TOPSOIL over L - clayey silt, trace sand, dish brown, moist	176.12	° <mark>Ó</mark>	100	200		0 30	20.5
SIL redo	TTILL - some clay, trace sand, dish brown, very dense	~175.1 	2	Ö 58		* *		
— — — —	ice shale fragments	_	3	39 0 50/125 mm		× ×		
For	ALE BDEROCK - Queenston mation, occasional limestone and dstone layers, reddish brown Coring Commenced	~172.1 	4	50/75.mm				
	See Core Log for Details		6 7 8 9 10 11 12 13 14					
using a specialist drill 2. This drawing forms	d to completion at ~15.4 m depth by conventional soil sampling subcontractor. For borehole definitions, see notes prior to spart of and must be read in conjunction with the subject reportehole data requires interpretation assistance by exp profe	o logs. oort (Ref. No.:		EXP.		Elapsed Time nuary 29, 2024 oruary 4, 2024	Water Level (m) ~5.7 ~5.8	Hole Open to (m) Well Well

PROJ	ЕСТ		ROCK CORE				I F	LEV		N (m)	-	DATUM			212		NUM	BF
		ical Inv	restigation	Vertio				176.		• (11)		Geode	tic			R-002		
LOCA				DATE		TED	C	OMP		D		LOGGE			DRA	WING		1B
510 CLIEN		Mills F	Parkway, Mississauga, Ontario	01/05				01/0		E	_	D. Pan CORE B		_	SHE	14 ET	A	
		o Grou	p of COmpanies	Davis	s Drilli		C	ME 5	5 - T	rack		HQ	1	_		1 0	f 2	
(m) NO	Ē			SETS							RING	E	RE	MBER	RY (%)		RY (%)	
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF S	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY	RQD	WATER RECOVERY (%)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	•
171.9	- - -		See Borehole Log for Details															
171.5	-		QUEENSTON FORMATION	1	В	F	с	RU						1	100	74	95	
71.1 71.0	-		Shale with interbedded siltstone, and clay layers.				č	RP						_				
70.7 70.7 70.5	-		Shale (71%) thinly bedded or laminated, rec low strength, alternating between heavily ar slightly weathered to ~5.7 m and between moderately weathered and unweathered	ł, id					NC	20 mm								
70.4	6		below.		В	F	C C	RP RP						2	100	100	100	1
70.0 69.6	-		Limestone (3%) fine grained, grey, medium strength, unweathered Siltstone (25%) fine grained, grey, medium strength, unweathered.															
69.5 69.5 69.3	-		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to		F	V			NC NC	20 mm 60		┼						
69.3 69.1 69.0	7 - -		very close intervals. Vertical fractures were noted at ~6.6 m, 7.1 7.6 m, 9.1 m and 10.2 m.	^{m,} 1	FB	V	C C	RP SU		mm				3	100	74	100	
68.7 68.7 68.6	-		A Clay (1%) layers, heavily weathered, very low strength were noted at ~5.4 m, 6.6 m ar 6.8 m.	nd	F	v		30										
68.5 68.3 68.1	-8																	
67.6 67.6	-																	
67.6 67.2 67.2				1	В	F	W M	SU SP						4	100	89	100	
67.2 67.1 66.9 66.8	- - -				F	V												
66.7 66.5	-																	
66.0	10				F	v												
65.7	-			1	B	F	M W	SP SP						5	100	91	100	
65.1	- - 11																	
65.0 64.9 64.7	-																	
о т .1	 - -																	
	12			1	В	F	W W	SP SP						6	100	100	100	

			ROCK CORE										B	Н	21	2		
PROJI				ORIEN		ON	E			N (m)	D	ATUM			PRO			
LOCA		cai inv	estigation	Verti DATE		RTED	c	176. OMP		ED	L	Geodet OGGEE					57769 NUM	
5100) Erin	Mills P	arkway, Mississauga, Ontario	01/0	5/23			01/0	5/23			D. Pano				14		
CLIEN The		- Grou	p of COmpanies	Davi	. ER s Dril	lina		DRILL CME 5				ORE BA	ARREL		SHE	ET 2 o	f 2	
						T CH												Q
ELEVATION (m)	DEPTH (m)	ω SYMBOL	GENERAL DESCRIPTION	4 NO. OF SETS	o JOINT TYPE			ه ROUGHNESS	5 FILLING	L APERTURE (mm)		C STRENGTH	FRACTURE FREQUENCY	15 RUN NUMBER	B RECOVERY (%)	00 17	© RECOVERY (%)	
163.9	. –		QUEENSTON FORMATION															-
163.7			QUEENSION FORMATION															
63.4	40		Shale with interbedded siltstone, and clay layers.															
163.0	-13		Shale (71%) thinly bedded or laminated, rec low strength, alternating between heavily an slightly weathered to ~5.7 m and between moderately weathered and unweathered below.	1	В	F	w	SP						7	100	100	100	
162.0	-14		Limestone (3%) fine grained, grey, medium strength, unweathered Siltstone (25%) fine grained, grey, medium strength, unweathered.				W	SP										
161.9			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.	_														
161.1 160.9	-15		Vertical fractures were noted at ~6.6 m, 7.1 7.6 m, 9.1 m and 10.2 m. A Clay (1%) layers, heavily weathered, very low strength were noted at ~5.4 m, 6.6 m ar	1	В	F	W W	SP SP						8	100	100	100	
160.7			6.8 m. End of Borehole at 15.4 m			-												
-	-16																	
-	-17																	
-																		
	-18																	
- - - -																		
- - - - -	-19																	
-	- -																	
-	-20																	

Project No.	GTR-00257769-Н0	g o	f	Bc	oreh	ole	21	3 _{Dra}	wing No.		15
Project:	Geotechnical Investigation								heet No.		
Location:	Erin Mills Town Centre, 51		Μ	ills Pa	rkway, M	Ississa	luga, Or				
										_	
Date Drilled:	January 4 - 5, 2024			Auger Sar				ibustible Va iral Moistur	ipour Reading e	×	-
Drill Type:	Hollow Stem Augers			SPT (N) V Dynamic (0 🛛		tic and Liqu rained Triax			-0
Datum:	Geodetic			Shelby Tu			% S	train at Fail		€)
Datam.			-	Field Vane	e lest	S	Pen	etrometer		-	•
vater ock ool		ELEV.	(u		SPT (N Va			25	our Reading (ppm) 50 75		Natural Unit
Groundwater Soil/Rock Symbol	Soil Description	m	Depth	20 Shear St	rength	60 80			ture Content % s (% Dry Weight)	Sample	Weight kN/m ³
~15	0 mm TOPSOIL over	176.15	0	10 Ö	100	200		10 2	20 30		21.8
redc	L - clayey silt, trace sand, lish brown, moist	~175.4				70/275 mm		\checkmark			22.5
	T TILL - some clay, trace sand, lish brown, compact to very dense		1			0	00/075 mm				22.5
			2				98/27 5 mm	(
		_			50/50 m	m	×	•			
		_	3		50/50 m	m	×				
		_			50/125 m	nn					
tra	ce shale fragments	_	4		0						
	Coring Commenced	~171.4			50/100 m	nn -	×			zz	
	ALE BDEROCK - Queenston nation, occasional limestone and		5								
	dstone layers, reddish brown										
			6								
			7								
		_	8								
		_									
		_	9								
		_									
4	See Core Log for Details	_	10								
3/20/24		_									
			11								
			12								
		_									
		_	13								
		_									
H692			14								
AGWGL02EXP 257769H_BH_LOGS.GPJ NEW.GD1		_									
		~160.9	15								
ewe	End of Borehole										
] T					<u> </u>	Water	<u>≓</u> ,.	
Noto-:					\sim		Elap Tin	sed ne	Level (m)		ble Open to (m)
	to completion at ~15.3 m depth by conventional soil samplin ng subcontractor. For borehole definitions, see notes prior to				exp	J.					
2. This drawing forms	part of and must be read in conjunction with the subject report borehole data requires interpretation assistance by exp profes	ort (Ref. No.:			1						
before use by others.					ramptor						

PROJI Geo		cal Inv	ROCK CORE	ORIEN Vertic	TATIO		E	LEVA		N (m)		DATUM Geode			21 PRO. GTF	JECT	NUM 5776	
LOCA 5100 CLIEN	TION) Erin T	Mills F	arkway, Mississauga, Ontario	DATE \$ 01/05 DRILLI	5/23 ER		D	OMP 01/05	5/23 TYP	E		D. Pan	chal		DRA	15 ET	βA	1B
	Muzzo	o Grou	p of COmpanies		s Drilli JOIN ⁻							HQ		~	(%	10		
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION	NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
1 172.0	2	3	4	5	6	7	8	9	10	11	12	2 13	14	15	16	17	18	
474 4	-		See Borehole Log for Details															
171.4	-5		QUEENSTON FORMATION	1	в	F	M M	RU RP						1	100	100	95	
170.9 170.9	- - -		Shale with interbedded siltstone, and clay layers.		F	v	IVI							•				
170.7 170.6 170.4 170.4 170.2	6		moderately weathered and unweathered below.	d 1	В	F	C	RP SU	NC NC	20 mm 20				2	100	97	100	
170.2 169.8 169.7	- - -		Limestone (2%) fine grained, grey, medium strength, unweathered Siltstone (20%) fine grained, grey, medium strength, unweathered.							mm								
169.1 169.1	-7		strength, unweathered. Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.						NC	20								
169.0 168.9 168.9 168.8	- - - - -			w 0 1	В	F	C M	SU SP		mm				3	100	90	100	
168.7 168.6 168.4	8																	
167.5 167.3	- - - - 9			1	В	F	w	SP						4	100	100	100	
167.0 166.9	- - - 						Ŵ	SP SP										
166.3 166.2	-10																	
165.7 165.6	- 			1	В	F	W W	SP SP						5	100	100	100	
- - - - -	11																	
164.5 164.4	-12			1	В	F	w	SP						6	100	100	100	

OCAT 5100 CLIENT The N	echni 10N	cal Inv	estigation	ORIE	ENT	ATIC)N	E			l (m)	D	ATUM			PRO.	IFCT	NUM	n=
OCAT 5100 CLIENT The N	ION			1 1/0	rtica				176.2		• •		Geodet	io			R-002		
The N	E rim		esugation	DAT			TED		OMP		D		OGEOGEC			DRA			
The N		Mills P	arkway, Mississauga, Ontario		/05/2				01/05				D. Pano				15	A	
		o Grou	o of COmpanies	DRIL Da		R Drilliı	na		RILL ME 5				ore B/ Hq	ARREL		SHEE	=1 2 o	f 2	
			-				<u> </u>		TERI							-			2
ELEVATION (m)	DEPTH (m)	SYMBOL	GENERAL DESCRIPTION		NO. OF SETS	JOINT TYPE	ORIENTATION	SPACING	ROUGHNESS	FILLING	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)	RQD	WATER RECOVERY (%)	
<u>団</u> 1	<u> </u>	б 3	4		ž 5	<u> </u>	0	5 8	2 9	正 10	₹5 11	<u>≥</u> 12	່ທ 13	14	₩ 15	₩ 16	₩ 17	<u>≥⊡</u> 18	<u>}</u>
63.9 ⁻	-				-	•		w	SP			 TTT							Ē
63.8			QUEENSTON FORMATION					vv											
53.6																			
			Shale with interbedded siltstone, and clay layers.																
3.2	13		Shale (78%) thinly bedded or laminated, red	I,															
2.9			Shale (78%) thinly bedded or laminated, red low strength, alternating between heavily an slightly weathered to ~5.6 and between moderately weathered and unweathered	a															
2.7		ЩЦ	below.			_	_												_ ح
2.4			Limestone (2%) fine grained, grey, medium strength, unweathered		1	В	F	M W	SP SP						7	100	100	100	2 Po D
2.3-	14		Siltstone (20%) fine grained grey medium																
Ę			strength, unweathered.																
-		F	Discontinuities: bedding joints are rough planar to smooth undulating and at wide to very close intervals.																
			A Vertical fracture was noted at ~5.3 m.		1	В	F	м	SP						8	100	100	100	
1.3- 1.2-	15		Clay (1%) layers, heavily weathered, very lo strength were noted at ~5.8 m, 5.9 m and 7.			2		M	SP							100	100	100	
0.9			m.	0															
			End of Borehole at 15.3 m																
F																			
F	16																		
E	16																		
-																			
-																			
F																			
-	17																		
E																			
F																			
F																			
Ē	18																		
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F																			
F																			
Ē																			
-	19																		
F																			
F																			
Ē																			
Ļ	20																		

	Project No.		g o	f	B	or	eh	ole	2				16
	Project: Location:	Geotechnical Investigation Erin Mills Town Centre, 51	00 Frin	Mi	lls P	arkw	av M	llssiss	auda		Sheet No.	<u>1</u> (of <u>1</u>
	Location.						ay, w	100100	uugu				
	Date Drille	d: January 4, 2024			Auger S	•				Combustible V Natural Moistu		×	
	Drill Type:	Hollow Stem Augers		_ 1	SPT (N) Dynamic	c Cone⊺	Fest			Plastic and Liq Undrained Tria	xial at	•	-0
	Datum:	Geodetic			Shelby 1 Field Va					% Strain at Fai Penetrometer	lure		
	Groundwater Soil/Rock Symbol	Soil Description	ELEV. m	Depth (m)		20 Strength		,	0 kPa	25	50 75 50 75 Sture Content % ts (% Dry Weight)	Sample	Natural Unit Weight kN/m ³
7	- F	100 mm TOPSOIL over ILL - clayey silt, trace sand, - ddish brown, moist	175.97 ~174.9	0	Ô	29	100	20	0		20 30		21.7
		ILT TILL - some clay, trace sand, ddish brown, compact to very dense - -		2			50/50 m			×			
		- race shale fragments below ~3.5 m epth - -		3 4 5			50/75.m 50/100 n 50/75.m	m nm 87	/275 mm	× × × ×			
	F.F.	Coring Commenced HALE BDEROCK - Queenston ormation, occasional limestone and andstone layers, reddish brown	_~170.2 ~169.9 _ _	6									
24		-	-	8 9 10									
NEW.GDT 3/20/24		See Core Log for Details	_	11									
H BH LOGS.GPJ		-	-	13									
LAGWGL02EXP 257769H		-	-	14 15									
AGWC		End of Borehole	~160.1	+									
	using a specialist 2. This drawing fo	need to completion at ~15.9 m depth by conventional soil sampling drilling subcontractor. For borehole definitions, see notes prior to 1 ms part of and must be read in conjunction with the subject repo (0); borehole data requires interpretation assistance by exp profes	ogs. rt (Ref. No.:				X mptor			Elapsed Time uary 29, 2024 ruary 4, 2024	Water Level (m) ~6.1 ~6.3	1	le Open io (m) Well Well

			ROCK CORE										DI		214			
PROJE			restigation	ORIEN Vertio		ON	E	176.0		N (m)	ר	DATUM Geode	tic			JECT R-002		
.OCA		cai inv	esugation	DATE		TED	c	OMP		ED	L	.OGGEI				K-002		
5100) Erin	Mills P	Parkway, Mississauga, Ontario	01/04	/23			01/04	1/23			D. Pan	chal			16		
CLIEN The		o Grou	p of COmpanies	DRILLI Davis		na		RILL ME 5			C	HQ	ARREL	•	SHE	ET 1 o	 f 2	
								TERI										<u>0</u>
ELEVATION (m)	۲			SETS	PE	ORIENTATION	6	ROUGHNESS		RE	WEATHERING	Ŧ	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	
(ATI	DEPTH (m)	SYMBOL		OF S	JOINT TYPE	NTA	SPACING	GHN	0 Z	APERTURE (mm)	품	STRENGTH		Ñ	OVE		R N N	
Ē	EPI	W X	GENERAL DESCRIPTION	NO.	NO	RIE	PAG	no Q	FILLING	DEF DEF	VEA	TRE	RAC	NN	U U U U	RQD	KAT ECC	
ш 1	2	3	4	5	ר 6	7	8	9	ш 10	عت 11	> 12	13	14	15		17	<u>>12</u>	> 1
70.3	-		See Borehole Log for Details									+	+	-				
~ ~	-6		-															
69.9	-		QUEENSTON FORMATION															7
ŀ	-		.	1	В	F	C C	RU RP						1	100	88	95	Pad Pad
E	-		Shale with interbedded siltstone, and clay layers.								$\left \right $			-				-
69.0	7		Shale (91%) thinly bedded or laminated, red,															
68.9 68.8	-		Shale (91%) thinly bedded or laminated, red, low strength, alternating between heavily and slightly weathered to ~6.6 and between moderately weathered and unweathered															
68.7 68.6	_		below.	1	в	F	М	RP						2	100	97	100	Red
68.4			Limestone (1%) fine grained, grey, medium strength, unweathered				М	SP										
68.1 68.1	-8		Siltstone (8%) fine grained, grey, medium strength, unweathered.															
ļ	-		Discontinuities: bedding joints are rough planar to smooth undulating and at wide to close intervals.															-
F	-		close intervals.															
ļ	-																	
67.0	9			1	в	F	w	su						3	100	100	100	Pod Dod
66.9	-						W	SP										
66.6 66.5	-																	
Ē	-																	
F	10																	
ŀ																		
F	-			1	в	F	w	SP						4	100	100	100	Pod
65.3 65.2	-					.	Ŵ	SP SP										
55.Z	-11																	
F																		
64.5	-																	
64.4	-																	
64.0	-12			4	P	_	10/	en						5	100	100	100	Red
63.9				1	В	F	W W	SP SP						5	100	100		d
63.7 63.6	-																	
ļ																		
63.0	-13																	
62.8		ЩЦ																
-	-				_	_												7
-	-			1	В	F	W	SP						6	100	100	100	
÷:	<u> </u>	<u> </u>)			r	vv	05						0	100			_

			ROCK CORE	ELC)G	j							Bł	12	214	D		
ROJ				ORIEN		ON	E			l (m)		TUM			PRO			
	techni TION	ical Inv	estigation	Vertic				176.0) LETE			Geodet			GTF DRA		5776	
		Mills P	arkway, Mississauga, Ontario	01/04		IED		01/04		D		D. Pano			DRA	16		IDE
LIEN				DRILLE			D		TYPE	Ξ			ARREL		SHE			
The	Muzz	o Grou	p of COmpanies	Davis		<u> </u>			5 - Tr		ł	HQ				2 0	f 2	
ELEVATION (m)	DEPTH (m)	BOL		OF SETS			SPACING		STIC	APERTURE (mm)	WEATHERING	STRENGTH	FRACTURE FREQUENCY	RUN NUMBER	RECOVERY (%)		WATER RECOVERY (%)	
_		SYMBOL	GENERAL DESCRIPTION	NO.					EILLING	APEF (mm)	MEA 12	13				RQD		
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	1
-	-		QUEENSTON FORMATION				W	SP										
62.0 61.9	-14		layers.															
51.6 51.6	- - - -		Shale (91%) thinly bedded or laminated, red low strength, alternating between heavily ar slightly weathered to ~6.6 and between moderately weathered and unweathered below.	d, nd														
	-15		Limestone (1%) fine grained, grey, medium strength, unweathered	1	в	F	w	SP						7	100	100	100	
0.7 0.5	_		Siltstone (8%) fine grained, grey, medium strength, unweathered.				W	SP										
50.3 50.1			Discontinuities: bedding joints are rough planar to smooth undulating and at wide to close intervals.															
	-16		End of Borehole at 15.9 m															
-	- -																	
-	-17																	
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	-21																	
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CKCORE 257769H_ROCK_LOGS GPJ CORE_LOG GDT 3/20/24

[®] * exp.

Project No. Project: Location:	GTR-00257769-H0 Geotechnical Investigatio Erin Mills Town Centre, 5	n		Borehole		Dra	wing No heet No1		17 of <u>1</u>
Date Drilled: Drill Type: Datum:	January 4, 2024 Hollow Stem Augers Geodetic	lollow Stem Augers				Combustible Va Natural Moisturd Plastic and Liqu Undrained Triax % Strain at Failu Penetrometer	e id Limit ┣ ial at	□ × ⊕	-0
Groundwater Soli/Rock Symbol	Soil Description	ELEV. m		SPT (N Value) 20 40 60 80 Shear Strength 100 20	kPa	25 5 Natural Moist	pour Reading (ppm) 50 75 sture Content % its (% Dry Weight)		Natural Unit Weight kN/m ³
	See Log of Borehole 214D for Details	175.97 	0 1 2 3 4 5 6						
Notes: 1. Borehole advanced	to completion at ∼6.1 m depth by conventional soil sampl	ing methods using		*exn		Elapsed Time nuary 29, 2024	Water Level (m) ~5.9		le Open to (m) Well
a specialist drilling sub 2. This drawing forms	to competitor at "or. I'm deput by conventional out samp contractor. For borehole definitions, see notes prior to log part of and must be read in conjunction with the subject porehole data requires interpretation assistance by exp pro	s. eport (Ref. No.:		Brampton		oruary 4, 2024	~6.0		Well

EXP Services Inc.

Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

Appendix D – Analytical Results – Soil and Groundwater



Table 1 - Petroleum Hydrocarbons (PHCs) including Benzene, Toluene, Ethylbenzene and Xylene (BTEX) in Soil GTR-00257769-H0 - Phase Two Environmental Site Assessment

2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Sample ID				BH202-SS3	BH205-SS2	BH205-SS6	BH209-SS2	BH203-SS3	BH210-SS2
Depth (mbgs)				1.52-2.13	0.76-1.37	4.57-5.18	0.76-1.37	1.52-2.13	0.76-1.37
Date Sampled	Units	Ontario Regulation 153/04	Minimum DDI *	1/18/2024	1/20/2024	1/20/2024	1/15/2024	1/20/2024	1/20/2024
Date Analyzed	Units	Table 3 Soil Standards**	Minimum RDL*	EXP	EXP	EXP	EXP	EXP	EXP
Laboratory				AGAT	AGAT	AGAT	AGAT	AGAT	AGAT
Lab Job Number				24T116256	24T116256	24T116256	24T116256	24T116256	24T116256
Benzene	μg/g	0.21	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02
Toluene	μg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Ethylbenzene	μg/g	2.3	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Xylenes (Total)	μg/g	3.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
F1 (C6 - C10)	μg/g	55	5	<5	<5	<5	<5	<5	<5
F1 (C6 to C10) minus BTEX	μg/g	55	5	<5	<5	<5	<5	<5	<5
F2 (C10 to C16)	μg/g	98	10	<10	<10	<10	<10	<10	<10
F3 (C16 to C34)	μg/g	300	50	<50	<50	<50	<50	<50	<50
F4 (C34 to C50)	μg/g	2800	50	<50	<50	<50	<50	<50	<50

NOTES:

All results in ppm (ug/g) and based on dry weight basis.

* Minimum Reporting Detection Limit is listed. Refer to individual Certificate of Analyses for sample-specific Reporting Detection Limit (RDL) value.

** Standards shown are for a Residential/Parkland/Institutional property use

with Medium/Fine textured soil.

Exceedances of Table 3 Standards are shown in **bold**, red and shaded.

NA - Not Applicable



Table 2 - Volatile Organic Compounds (VOCs) in Soil

GTR-00257769-H0 - Phase Two Environmental Site Assessment

2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Sample ID				BH202-SS3	BH205-SS2	BH205-SS6	BH209-SS2	BH203-SS3	BH210-SS2
Depth (mbgs)				1.52-2.13	0.76-1.37	4.57-5.18	0.76-1.37	1.52-2.13	0.76-1.37
Date Sampled		Ontario Regulation 153/04		1/18/2024	1/20/2024	1/20/2024	1/15/2024	1/20/2024	1/20/2024
Date Analyzed	Units	Table 3 Soil Standards**	Minimum RDL*	EXP	EXP	EXP	EXP	EXP	EXP
Laboratory				AGAT	AGAT	AGAT	AGAT	AGAT	AGAT
Lab Job Number				24T116256	24T116256	24T116256	24T116256	24T116256	24T116256
1,1,1,2-Tetrachloroethane	µg/g	0.058	0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04
1,1,1-Trichloroethane	μg/g	0.38	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,1,2,2-Tetrachloroethane	μg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,1,2-Trichloroethane	μg/g	0.05	0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04
1,1-Dichloroethane	μg/g	3.5	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02
1,1-Dichloroethylene	μg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,2-Dichlorobenzene	μg/g	3.4	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,2-Dichloroethane	μg/g	0.05	0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03
1,2-Dichloropropane	μg/g	0.05	0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03
1,3-Dichlorobenzene	μg/g	4.8	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,3-Dichloropropene (Cis + Trans)	µg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1,4-Dichlorobenzene	µg/g	0.083	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Acetone	µg/g	16	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50
Benzene	µg/g	0.21	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02
Bromodichloromethane	µg/g	13	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Bromoform	μg/g	0.27	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Bromomethane	µg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Carbon Tetrachloride	µg/g	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Chlorobenzene	µg/g	2.4	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Chloroform	μg/g	0.05	0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04
Cis- 1,2-Dichloroethylene	µg/g	3.4	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02
Dibromochloromethane	µg/g	9.4	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Dichlorodifluoromethane	μg/g	16	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Ethylbenzene	µg/g	2	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Ethylene Dibromide	µg/g	0.05	0.04	<0.04	<0.04	<0.04	<0.04	<0.04	<0.04
m & p-Xylene	µg/g	-	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Methyl Ethyl Ketone	µg/g	16	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50
Methyl Isobutyl Ketone	µg/g	1.7	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50
Methyl tert-butyl Ether	µg/g	0.75	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Methylene Chloride	µg/g	0.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
n-Hexane	µg/g	2.8	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
o-Xylene	µg/g	-	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Styrene	µg/g	0.7	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Tetrachloroethylene	µg/g	0.28	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Toluene	µg/g	2.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Trans- 1,2-Dichloroethylene	µg/g	0.084	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Trichloroethylene	µg/g	0.061	0.03	<0.03	<0.03	<0.03	<0.03	<0.03	<0.03
Trichlorofluoromethane	µg/g	4	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Vinyl Chloride	µg/g	0.02	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02
Xylenes (Total)	µg/g	3.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05

NOTES:

All results in ppm (ug/g) and based on dry weight basis.

* Minimum Reporting Detection Limit is listed. Refer to individual Certificate of Analyses for sample-specific Reporting Detection Limit (RDL) value.

** Standards shown are for a Residential/Parkland/Institutional property use

with Medium/Fine-textured soil.

Exceedances of Table 3 Standards are shown in **bold**, red and shaded.

NA - Not Applicable



Table 3 - Polycyclic Aromatic Hydrocarbons (PAHs) in Soil

GTR-00257769-H0 - Phase Two Environmental Site Assessment

2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Sample ID				BH202-SS2	BH205-SS2	BH205-SS6	BH209-SS2	BH203-SS2	BH211-SS2
Depth (mbgs)				0.76-1.37	0.76-1.37	4.57-5.18	0.76-1.37	0.76-1.37	0.76-1.37
Date Sampled	Units	Ontario Regulation 153/04	Minimum RDL*	1/18/2024	1/20/2024	1/20/2024	1/15/2024	1/20/2024	1/8/2024
Date Analyzed	Onics	Table 3 Soil Standards**		EXP	EXP	EXP	EXP	EXP	EXP
Laboratory				AGAT	AGAT	AGAT	AGAT	AGAT	AGAT
Lab Job Number				24T116256	24T116256	24T116256	24T116256	24T116256	24T116256
Naphthalene	µg/g	9.6	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Acenaphthylene	μg/g	0.15	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Acenaphthene	μg/g	96	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Fluorene	μg/g	62	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Phenanthrene	μg/g	12	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Anthracene	μg/g	0.67	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Fluoranthene	μg/g	9.6	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Pyrene	μg/g	96	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Benz(a)anthracene	μg/g	0.96	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Chrysene	μg/g	9.6	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Benzo(b)fluoranthene	μg/g	0.96	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Benzo(k)fluoranthene	μg/g	0.96	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Benzo(a)pyrene	μg/g	0.3	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Indeno(1,2,3-cd)pyrene	μg/g	0.76	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Dibenz(a,h)anthracene	μg/g	0.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
Benzo(g,h,i)perylene	μg/g	9.6	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05
1 and 2 Methlynaphthalene	μg/g	76	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05

NOTES:

All results in ppm (ug/g) and based on dry weight basis.

* Minimum Reporting Detection Limit is listed. Refer to individual Certificate of Analyses for sample-specific

Reporting Detection Limit (RDL) value.

** Standards shown are for a Residential/Parkland/Institutional property use

with Medium/Fine-textured soil.

Exceedances of Table 3 Standards are shown in **bold, red and shaded**.

pH exceedances of Applicable Standards are shown in purple text.

NA - Not Applicable



Table 4 - Metals and Inorganics in Soil

GTR-00257769-H0 - Phase Two Environmental Site Assessment 2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Sample ID				BH202-SS2	BH205-SS2	BH205-SS6	BH209-SS2	BH203-SS2	BH211-SS2
Depth (mbgs)				0.76-1.5	0.76-1.5	4.57-5.18	0.76-1.5	0.76-1.5	0.76-1.5
Date Sampled	l lucitor	Ontario Regulation 153/04		1/18/2024	1/20/2024	1/20/2024	1/15/2024	1/20/2024	1/8/2024
Date Analyzed	Units	Table 3 Soil Standards**	Minimum RDL*	EXP	EXP	EXP	EXP	EXP	EXP
Laboratory				AGAT	AGAT	AGAT	AGAT	AGAT	AGAT
Lab Job Number				24T110066	24T110066	24T110066	24T110066	24T110066	24T110066
Antimony	µg/g	7.5	0.8	<0.8	<0.8	<0.8	<0.8	<0.8	<0.8
Arsenic	μg/g	18	1	6	3	2	5	5	5
Barium	μg/g	390	2	70.1	51.4	22.4	65.7	72.5	74.4
Beryllium	μg/g	4	0.5	0.8	<0.5	<0.5	0.8	0.7	0.7
Boron	μg/g	120	5	20	8	<5	14	18	17
Boron (Hot Water Soluble)	μg/g	1.5	0.1	0.21	0.16	<0.10	0.11	0.18	0.11
Cadmium	μg/g	1.2	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Chromium	μg/g	160	5	26	18	8	24	26	24
Chromium, Hexavalent	μg/g	8	0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2
Cobalt	μg/g	22	0.8	14.4	7.3	3.3	15.4	14.9	14.3
Copper	μg/g	140	1	7.6	14.1	7.3	9.7	8	8.8
Cyanide, WAD	μg/g	0.051	0.04	<0.040	<0.040	<0.040	<0.040	<0.040	<0.040
Electrical Conductivity (2:1)	mS/cm	0.7	0.005	0.282	1.04	1.1	0.697	0.486	0.141
Lead	μg/g	120	1	9	5	2	9	10	9
Mercury	μg/g	0.27	0.1	<0.10	<0.10	<0.10	<0.10	<0.10	<0.10
Molybdenum	μg/g	6.9	0.5	1.2	0.5	<0.5	1.1	1.1	0.7
Nickel	μg/g	100	1	31	16	6	32	31	30
pH, 2:1 CaCl2 Extraction	pH Units	5.0-9.0	NA	7.45	8.1	7.5	7.43	7.4	7.15
Selenium	μg/g	2.4	0.8	<0.8	<0.8	<0.8	<0.8	<0.8	<0.8
Silver	μg/g	20	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Sodium Adsorption Ratio (2:1) (Calc.)	N/A	5	NA	3.08	7.94	12.8	12.3	5.96	2.08
Thallium	µg/g	1	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Uranium	μg/g	23	0.5	0.64	0.6	<0.50	0.64	0.61	0.81
Vanadium	μg/g	86	2	31.7	27.7	16.9	31.8	32.5	40.8
Zinc	μg/g	340	5	65	37	19	67	71	61

NOTES:

All results in ppm (ug/g) and based on dry weight basis.

* Minimum Reporting Detection Limit is listed. Refer to individual Certificate of Analyses for sample-specific Reporting Detection Limit (RDL) value.

** Standards shown are for a Residential/Parkland/Institutional property use

with Medium/Fine textured soil.

Exceedances of Table 3 Standards are shown in **bold**, red and shaded.

pH exceedances of Applicable Standards are shown in purple text.

NA - Not applicable



Table 5 - Polychlorinated Bi-Phenols in Soil

GTR-00257769-H0 - Phase Two Environmental Site Assessment 2670 & 2690 Erin Centre Boulevard, Mississauga, Ontario

Sample ID				BH205-SS1 (Collected from TP5A)
Depth (mbgs)				0.0-0.3
Date Sampled	Units	Ontario Regulation 153/04	Minimum RDL*	1/20/2024
ite Analyzed	Units	Table 3 Soil Standards**		EXP
Laboratory				AGAT
Lab Job Number				24T110066
Polychlorinated Biphenyls	µg/g	0.35	0.1	<0.1

NOTES:

All results in ppm (ug/g) and based on dry weight basis.

* Minimum Reporting Detection Limit is listed. Refer to individual Certificate of Analyses for sample-specific Reporting Detection Limit (RDL) value.

** Standards shown are for a Residential/Parkland/Institutional property use with Medium/Fine textured soil.

Exceedances of Table 3 Standards are shown in **bold**, red and shaded.

pH exceedances of Applicable Standards are shown in purple text.

NA - Not applicable

GTR-23007732-B0

Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

Appendix E – Laboratory Certificates of Analysis





CLIENT NAME: EXP SERVICES INC 1595 CLARK BLVD. BRAMPTON, ON L6T4V1 (905) 793-9800 ATTENTION TO: Alessandro Girardo PROJECT: GTR-00257769-H0 AGAT WORK ORDER: 24T116256 SOIL ANALYSIS REVIEWED BY: Nivine Basily, Inorganic Team Lead TRACE ORGANICS REVIEWED BY: Neli Popnikolova, Senior Chemist DATE REPORTED: Feb 07, 2024 PAGES (INCLUDING COVER): 19 VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

*Notes

Disclaimer:

- All work conducted herein has been done using accepted standard protocols, and generally accepted practices and methods. AGAT test methods may
 incorporate modifications from the specified reference methods to improve performance.
- All samples will be disposed of within 30 days after receipt unless a Long Term Storage Agreement is signed and returned. Some specialty analysis may be exempt, please contact your Client Project Manager for details.
- AGAT's liability in connection with any delay, performance or non-performance of these services is only to the Client and does not extend to any other third party. Unless expressly agreed otherwise in writing, AGAT's liability is limited to the actual cost of the specific analysis or analyses included in the services.
- This Certificate shall not be reproduced except in full, without the written approval of the laboratory.
- The test results reported herewith relate only to the samples as received by the laboratory.
- Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, warranties of
 merchantability, fitness for a particular purpose, or non-infringement. AGAT assumes no responsibility for any errors or omissions in the guidelines
 contained in this document.
- All reportable information as specified by ISO/IEC 17025:2017 is available from AGAT Laboratories upon request.
- For environmental samples in the Province of Quebec: The analysis is performed on and results apply to samples as received. A temperature above 6°C upon receipt, as indicated in the Sample Reception Notification (SRN), could indicate the integrity of the samples has been compromised if the delay between sampling and submission to the laboratory could not be minimized.

AGAT Laboratories (V1)

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Member of: Association of Professional Engineers and Geoscientists of Alberta	
(APEGA)	
Western Envire Agricultural Laboratory Association (M/EALA)	

Western Enviro-Agricultural Laboratory Association (WEALA) Environmental Services Association of Alberta (ESAA) AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation. AGAT Laboratories (Mississauga) is also accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) for specific drinking water tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. Measurement Uncertainty is not taken into consideration when stating conformity with a specified requirement.

Page 1 of 19



Certificate of Analysis

AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

SAMPLED BY:

DATE RECEIVED: 2024-01-31								0	DATE REPORTE	ED: 2024-02-07
Parameter	Unit	G / S: A		SCRIPTION: MPLE TYPE: SAMPLED: RDL	BH202-SS2 Soil 2024-01-30 5614359	BH205-SS2 Soil 2024-01-30 5614370	BH205-SS6 Soil 2024-01-30 5614371	BH209-SS2 Soil 2024-01-30 5614372	BH203-SS2 Soil 2024-01-30 5614373	BH211-SS2 Soil 2024-01-30 5614382
Antimony	µg/g	1.3	7.5	0.8	<0.8	<0.8	<0.8	<0.8	<0.8	<0.8
Arsenic	μg/g	1.5	18	1	<0.0 6[<a]< td=""><td>3[<a]< td=""><td>2[<a]< td=""><td><0.0 5[<a]< td=""><td><0.0 5[<a]< td=""><td>5[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	3[<a]< td=""><td>2[<a]< td=""><td><0.0 5[<a]< td=""><td><0.0 5[<a]< td=""><td>5[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	2[<a]< td=""><td><0.0 5[<a]< td=""><td><0.0 5[<a]< td=""><td>5[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	<0.0 5[<a]< td=""><td><0.0 5[<a]< td=""><td>5[<a]< td=""></a]<></td></a]<></td></a]<>	<0.0 5[<a]< td=""><td>5[<a]< td=""></a]<></td></a]<>	5[<a]< td=""></a]<>
Barium	µg/g	220	390	2.0	70.1[<a]< td=""><td>51.4[<a]< td=""><td>22.4[<a]< td=""><td>65.7[<a]< td=""><td>72.5[<a]< td=""><td>74.4[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	51.4[<a]< td=""><td>22.4[<a]< td=""><td>65.7[<a]< td=""><td>72.5[<a]< td=""><td>74.4[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	22.4[<a]< td=""><td>65.7[<a]< td=""><td>72.5[<a]< td=""><td>74.4[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	65.7[<a]< td=""><td>72.5[<a]< td=""><td>74.4[<a]< td=""></a]<></td></a]<></td></a]<>	72.5[<a]< td=""><td>74.4[<a]< td=""></a]<></td></a]<>	74.4[<a]< td=""></a]<>
Beryllium	µg/g	2.5	4	0.5	0.8[<a]< td=""><td><0.5</td><td><0.5</td><td>0.8[<a]< td=""><td>0.7[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	<0.5	<0.5	0.8[<a]< td=""><td>0.7[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<></td></a]<>	0.7[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<>	0.7[<a]< td=""></a]<>
Boron	µg/g	36	120	5	20[<a]< td=""><td>8[<a]< td=""><td><5</td><td>14[<a]< td=""><td>18[<a]< td=""><td>17[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	8[<a]< td=""><td><5</td><td>14[<a]< td=""><td>18[<a]< td=""><td>17[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	<5	14[<a]< td=""><td>18[<a]< td=""><td>17[<a]< td=""></a]<></td></a]<></td></a]<>	18[<a]< td=""><td>17[<a]< td=""></a]<></td></a]<>	17[<a]< td=""></a]<>
Boron (Hot Water Soluble)	µg/g	NA	1.5	0.10	0.21[<b]< td=""><td>0.16[<b]< td=""><td><0.10</td><td>0.11[<b]< td=""><td>0.18[<b]< td=""><td>0.11[<b]< td=""></b]<></td></b]<></td></b]<></td></b]<></td></b]<>	0.16[<b]< td=""><td><0.10</td><td>0.11[<b]< td=""><td>0.18[<b]< td=""><td>0.11[<b]< td=""></b]<></td></b]<></td></b]<></td></b]<>	<0.10	0.11[<b]< td=""><td>0.18[<b]< td=""><td>0.11[<b]< td=""></b]<></td></b]<></td></b]<>	0.18[<b]< td=""><td>0.11[<b]< td=""></b]<></td></b]<>	0.11[<b]< td=""></b]<>
Cadmium	µg/g	1.2	1.2	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Chromium	µg/g	70	160	5	26[<a]< td=""><td>18[<a]< td=""><td>8[<a]< td=""><td>24[<a]< td=""><td>26[<a]< td=""><td>24[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	18[<a]< td=""><td>8[<a]< td=""><td>24[<a]< td=""><td>26[<a]< td=""><td>24[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	8[<a]< td=""><td>24[<a]< td=""><td>26[<a]< td=""><td>24[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	24[<a]< td=""><td>26[<a]< td=""><td>24[<a]< td=""></a]<></td></a]<></td></a]<>	26[<a]< td=""><td>24[<a]< td=""></a]<></td></a]<>	24[<a]< td=""></a]<>
Cobalt	µg/g	21	22	0.8	14.4[<a]< td=""><td>7.3[<a]< td=""><td>3.3[<a]< td=""><td>15.4[<a]< td=""><td>14.9[<a]< td=""><td>14.3[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	7.3[<a]< td=""><td>3.3[<a]< td=""><td>15.4[<a]< td=""><td>14.9[<a]< td=""><td>14.3[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	3.3[<a]< td=""><td>15.4[<a]< td=""><td>14.9[<a]< td=""><td>14.3[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	15.4[<a]< td=""><td>14.9[<a]< td=""><td>14.3[<a]< td=""></a]<></td></a]<></td></a]<>	14.9[<a]< td=""><td>14.3[<a]< td=""></a]<></td></a]<>	14.3[<a]< td=""></a]<>
Copper	µg/g	92	140	1.0	7.6[<a]< td=""><td>14.1[<a]< td=""><td>7.3[<a]< td=""><td>9.7[<a]< td=""><td>8.0[<a]< td=""><td>8.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	14.1[<a]< td=""><td>7.3[<a]< td=""><td>9.7[<a]< td=""><td>8.0[<a]< td=""><td>8.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	7.3[<a]< td=""><td>9.7[<a]< td=""><td>8.0[<a]< td=""><td>8.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	9.7[<a]< td=""><td>8.0[<a]< td=""><td>8.8[<a]< td=""></a]<></td></a]<></td></a]<>	8.0[<a]< td=""><td>8.8[<a]< td=""></a]<></td></a]<>	8.8[<a]< td=""></a]<>
Lead	µg/g	120	120	1	9[<a]< td=""><td>5[<a]< td=""><td>2[<a]< td=""><td>9[<a]< td=""><td>10[<a]< td=""><td>9[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	5[<a]< td=""><td>2[<a]< td=""><td>9[<a]< td=""><td>10[<a]< td=""><td>9[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	2[<a]< td=""><td>9[<a]< td=""><td>10[<a]< td=""><td>9[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	9[<a]< td=""><td>10[<a]< td=""><td>9[<a]< td=""></a]<></td></a]<></td></a]<>	10[<a]< td=""><td>9[<a]< td=""></a]<></td></a]<>	9[<a]< td=""></a]<>
Molybdenum	µg/g	2	6.9	0.5	1.2[<a]< td=""><td>0.5[<a]< td=""><td><0.5</td><td>1.1[<a]< td=""><td>1.1[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	0.5[<a]< td=""><td><0.5</td><td>1.1[<a]< td=""><td>1.1[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	<0.5	1.1[<a]< td=""><td>1.1[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<></td></a]<>	1.1[<a]< td=""><td>0.7[<a]< td=""></a]<></td></a]<>	0.7[<a]< td=""></a]<>
Nickel	µg/g	82	100	1	31[<a]< td=""><td>16[<a]< td=""><td>6[<a]< td=""><td>32[<a]< td=""><td>31[<a]< td=""><td>30[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	16[<a]< td=""><td>6[<a]< td=""><td>32[<a]< td=""><td>31[<a]< td=""><td>30[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	6[<a]< td=""><td>32[<a]< td=""><td>31[<a]< td=""><td>30[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	32[<a]< td=""><td>31[<a]< td=""><td>30[<a]< td=""></a]<></td></a]<></td></a]<>	31[<a]< td=""><td>30[<a]< td=""></a]<></td></a]<>	30[<a]< td=""></a]<>
Selenium	µg/g	1.5	2.4	0.8	<0.8	<0.8	<0.8	<0.8	<0.8	<0.8
Silver	µg/g	0.5	20	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Thallium	µg/g	1	1	0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5
Uranium	µg/g	2.5	23	0.50	0.64[<a]< td=""><td>0.60[<a]< td=""><td><0.50</td><td>0.64[<a]< td=""><td>0.61[<a]< td=""><td>0.81[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	0.60[<a]< td=""><td><0.50</td><td>0.64[<a]< td=""><td>0.61[<a]< td=""><td>0.81[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	<0.50	0.64[<a]< td=""><td>0.61[<a]< td=""><td>0.81[<a]< td=""></a]<></td></a]<></td></a]<>	0.61[<a]< td=""><td>0.81[<a]< td=""></a]<></td></a]<>	0.81[<a]< td=""></a]<>
Vanadium	µg/g	86	86	2.0	31.7[<a]< td=""><td>27.7[<a]< td=""><td>16.9[<a]< td=""><td>31.8[<a]< td=""><td>32.5[<a]< td=""><td>40.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	27.7[<a]< td=""><td>16.9[<a]< td=""><td>31.8[<a]< td=""><td>32.5[<a]< td=""><td>40.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	16.9[<a]< td=""><td>31.8[<a]< td=""><td>32.5[<a]< td=""><td>40.8[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	31.8[<a]< td=""><td>32.5[<a]< td=""><td>40.8[<a]< td=""></a]<></td></a]<></td></a]<>	32.5[<a]< td=""><td>40.8[<a]< td=""></a]<></td></a]<>	40.8[<a]< td=""></a]<>
Zinc	µg/g	290	340	5	65[<a]< td=""><td>37[<a]< td=""><td>19[<a]< td=""><td>67[<a]< td=""><td>71[<a]< td=""><td>61[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	37[<a]< td=""><td>19[<a]< td=""><td>67[<a]< td=""><td>71[<a]< td=""><td>61[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<></td></a]<>	19[<a]< td=""><td>67[<a]< td=""><td>71[<a]< td=""><td>61[<a]< td=""></a]<></td></a]<></td></a]<></td></a]<>	67[<a]< td=""><td>71[<a]< td=""><td>61[<a]< td=""></a]<></td></a]<></td></a]<>	71[<a]< td=""><td>61[<a]< td=""></a]<></td></a]<>	61[<a]< td=""></a]<>
Chromium, Hexavalent	µg∕g	0.66	8	0.2	<0.2	<0.2	<0.2	<0.2	<0.2	<0.2
Cyanide, WAD	µg/g	0.051	0.051	0.040	<0.040	<0.040	<0.040	<0.040	<0.040	<0.040
Mercury	µg/g	0.27	0.27	0.10	<0.10	<0.10	<0.10	<0.10	<0.10	<0.10
Electrical Conductivity (2:1)	mS/cm	0.57	0.7	0.005	0.282[<a]< td=""><td>1.04[>B]</td><td>1.10[>B]</td><td>0.697[A-B]</td><td>0.486[<a]< td=""><td>0.141[<a]< td=""></a]<></td></a]<></td></a]<>	1.04[>B]	1.10[>B]	0.697[A-B]	0.486[<a]< td=""><td>0.141[<a]< td=""></a]<></td></a]<>	0.141[<a]< td=""></a]<>
Sodium Adsorption Ratio (2:1) (Calc.)	N/A	2.4	5	N/A	3.08[A-B]	7.94[>B]	12.8[>B]	12.3[>B]	5.96[>B]	2.08[<a]< td=""></a]<>
pH, 2:1 CaCl2 Extraction	pH Units			NA	7.45	8.10	7.50	7.43	7.40	7.15



Certified By:



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

SAMPLED BY:

O. Reg. 153(511) - Metals & Inorganics (Soil)

DATE RECEIVED: 2024-01-31

DATE REPORTED: 2024-02-07

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Table 1: Full Depth Background Site Condition Standards - Soil -Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use, B Refers to O. Reg. 406/19 TABLE 2.1: Full Depth Potable Ground Water Condition Volume Independent - RP

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614359-5614382 EC was determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl2 extract prepared at 2:1 ratio. SAR is a calculated parameter.

Analysis performed at AGAT Toronto (unless marked by *)



Certified By:



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

SAMPLED BY:

					Sulphate	9
DATE RECEIVED: 2024-01-31						DATE REPORTED: 2024-02-05
		SAMPLE DES	CRIPTION:	BH202-SS2	BH205-SS6	
		SAM	PLE TYPE:	Soil	Soil	
		DATES	SAMPLED:	2024-01-30	2024-01-30	
Parameter	Unit	G/S	RDL	5614359	5614371	
Sulphate (2:1)	µg/g		2	13	49	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard

5614359-5614371 Sulphate was determined on the extract obtained from the 2:1 leaching procedure (2 parts DI water: 1 part soil).





AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0

O. Reg. 153(511) - PAHs (Soil)

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

SAMPLED BY:

DATE RECEIVED: 2024-01-31									DATE REPORTE	ED: 2024-02-06	
			SA	SCRIPTION: MPLE TYPE: E SAMPLED:	BH202-SS2 Soil 2024-01-30	BH205-SS2 Soil 2024-01-30	BH205-SS6 Soil 2024-01-30	BH209-SS2 Soil 2024-01-30	BH203-SS2 Soil 2024-01-30	BH211-SS2 Soil 2024-01-30	
Parameter	Unit	G / S: A	G / S: B	RDL	5614359	5614370	5614371	5614372	5614373	5614382	
Naphthalene	µg/g	0.09	0.2	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Acenaphthylene	µg/g	0.093	0.093	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Acenaphthene	µg/g	0.072	2.5	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Fluorene	µg/g	0.12	6.8	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Phenanthrene	µg/g	0.69	6.2	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Anthracene	µg/g	0.16	0.16	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Fluoranthene	µg/g	0.56	0.69	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Pyrene	µg/g	1	28	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Benz(a)anthracene	µg/g	0.36	0.5	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Chrysene	µg/g	2.8	7	0.05	< 0.05	< 0.05	<0.05	< 0.05	<0.05	<0.05	
Benzo(b)fluoranthene	µg/g	0.47	3.2	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Benzo(k)fluoranthene	µg/g	0.48	3.1	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Benzo(a)pyrene	µg/g	0.3	0.31	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
ndeno(1,2,3-cd)pyrene	µg/g	0.23	0.38	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Dibenz(a,h)anthracene	µg/g	0.1	0.57	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Benzo(g,h,i)perylene	µg/g	0.68	6.6	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
2-and 1-methyl Naphthalene	µg/g	0.59	0.59	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Moisture Content	%			0.1	5.1	3.6	10.2	8.4	6.7	7.0	
Surrogate	Unit	A	cceptable Lim	its							
Naphthalene-d8	%		50-140		85	90	100	100	90	85	
Acridine-d9	%		50-140		80	70	80	110	95	90	
Terphenyl-d14	%		50-140		95	105	95	110	95	105	

Comments:

RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Table 1: Full Depth Background Site Condition Standards - Soil -Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use, B Refers to O. Reg. 406/19 TABLE 2.1: Full Depth Potable Ground Water Condition Volume Independent - RP Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614359-5614382 Results are based on the dry weight of the soil.

Note: The result for Benzo(b)Fluoranthene is the total of the Benzo(b)&j)Fluoranthene isomers because the isomers co-elute on the GC column. 2- and 1-Methyl Naphthalene is a calculated parameter. The calculated value is the sum of 2-Methyl Naphthalene and 1-Methyl Naphthalene.

Certified By:

NPopukoloj



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

DATE REPORTED: 2024-02-07

SAMPLED BY:

O. Reg. 153(511) - PCBs (Soil)

DATE RECEIVED: 2024-01-31

		SAMPLE DESC	RIPTION:	BH205-SS1	
		SAMP	LE TYPE:	Soil	
		DATE S	AMPLED:	2024-01-30	
Parameter	Unit	G/S	RDL	5614365	
Polychlorinated Biphenyls	µg/g	0.3	0.1	<0.1	
Moisture Content	%		0.1	5.0	
Surrogate	Unit	Acceptable	e Limits		
Decachlorobiphenyl	%	50-14	40	80	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 1: Full Depth Background Site Condition Standards - Soil -Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614365 Results are based on the dry weight of soil extracted.

PCB total is a calculated parameter. The calculated value is the sum of Aroclor 1242, Aroclor 1248, Aroclor 1254 and Aroclor 1260. The calculated parameter is non-accredited. The parameters that are components of the calculation are accredited.

Certified By:

NPopukolof



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

DATE REPORTED: 2024-02-07

SAMPLED BY:

O. Reg. 153(511) - PHCs F1 - F4 (with PAHs and VOC) (Soil)

DATE RECEIVED: 2024-01-31

			SAMPLE DE	SCRIPTION:	BH205-SS2	BH205-SS6	BH209-SS2
			SA	MPLE TYPE:	Soil	Soil	Soil
			DATI	E SAMPLED:	2024-01-30	2024-01-30	2024-01-30
Parameter	Unit	G / S: A	G / S: B	RDL	5614370	5614371	5614372
F1 (C6 to C10)	µg/g	25		5	<5	<5	<5
F1 (C6 to C10) minus BTEX	µg/g	25	25	5	<5	<5	<5
F2 (C10 to C16)	µg/g	10	10	10	<10	<10	<10
F2 (C10 to C16) minus Naphthalene	µg/g			10	<10	<10	<10
F3 (C16 to C34)	µg/g	240	240	50	<50	<50	<50
F3 (C16 to C34) minus PAHs	µg/g		240	50	<50	<50	<50
F4 (C34 to C50)	µg/g	120	2800	50	<50	<50	<50
Gravimetric Heavy Hydrocarbons	µg/g	120		50	NA	NA	NA
Moisture Content	%			0.1	3.6	10.2	8.4
Surrogate	Unit	A	cceptable Limi	its			
Toluene-d8	%		50-140		94	92	95
Terphenyl	%		60-140		78	78	92

Comments:

RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Table 1: Full Depth Background Site Condition Standards - Soil -

Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use, B Refers to O. Reg. 406/19 TABLE 2.1: Full Depth Potable Ground Water Condition Volume Independent - RP Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614370-5614372 Results are based on sample dry weight.

The C6-C10 fraction is calculated using toluene response factor.

C6-C10 (F1 minus BTEX) is a calculated parameter. The calculated value is F1 minus BTEX. The calculated parameter is non-accredited. The parameters that are components of the calculation are accredited.

The C10 - C16, C16 - C34, and C34 - C50 fractions are calculated using the average response factor for n-C10, n-C16, and n-C34.

Gravimetric Heavy Hydrocarbons are not included in the Total C16-C50 and are only determined if the chromatogram of the C34 - C50 hydrocarbons indicates that hydrocarbons >C50 are present. The chromatogram has returned to baseline by the retention time of nC50.

Total C6 - C50 results are corrected for BTEX and PAH contributions.

C>10 - C16 (F2- Naphthalene) is a calculated parameter. The calculated value is F2 - Naphthalene.

C>16 - C34 (F3-PAH) is a calculated parameter. The calculated value is F3-PAH (PAH: sum of Phenanthrene, Benzo(a)anthracene, Benzo(b)fluoranthene, Benzo(k)fluoranthene, Benzo(a)pyrene, Fluoranthene, Dibenzo(a,h)anthracene, Indeno(1,2,3-c,d)pyrene and Pyrene).

This method complies with the Reference Method for the CWS PHC and is validated for use in the laboratory.

nC10, nC16 and nC34 response factors are within 10% of their average.

C50 response factor is within 70% of nC10 + nC16 + nC34 average.

Linearity is within 15%.

Extraction and holding times were met for this sample.

Certified By:

NPopukoloj



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

DATE REPORTED: 2024-02-07

SAMPLED BY:

O. Reg. 153(511) - PHCs F1 - F4 (with VOC) (Soil)

DATE RECEIVED: 2024-01-31

			SAMPLE DE	ESCRIPTION:	BH202-SS3	BH203-SS3	BH210-SS2
			SA	MPLE TYPE:	Soil	Soil	Soil
			DAT	E SAMPLED:	2024-01-30	2024-01-30	2024-01-30
Parameter	Unit	G / S: A	G / S: B	RDL	5614360	5614376	5614381
F1 (C6 to C10)	µg/g	25		5	<5	<5	<5
F1 (C6 to C10) minus BTEX	µg/g	25	25	5	<5	<5	<5
F2 (C10 to C16)	µg/g	10	10	10	<10	<10	<10
F3 (C16 to C34)	µg/g	240	240	50	<50	<50	<50
F4 (C34 to C50)	µg/g	120	2800	50	<50	<50	<50
Gravimetric Heavy Hydrocarbons	µg/g	120		50	NA	NA	NA
Moisture Content	%			0.1	4.2	6.6	7.1
Surrogate	Unit	A	cceptable Lim	its			
Toluene-d8	%		50-140		96	97	98
Terphenyl	%		60-140		80	82	90

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Table 1: Full Depth Background Site Condition Standards - Soil -

Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use, B Refers to O. Reg. 406/19 TABLE 2.1: Full Depth Potable Ground Water Condition Volume Independent - RP Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614360-5614381 Results are based on sample dry weight.

The C6-C10 fraction is calculated using toluene response factor.

C6–C10 (F1 minus BTEX) is a calculated parameter. The calculated value is F1 minus BTEX. The calculated parameter is non-accredited. The parameters that are components of the calculation are accredited.

The C10 - C16, C16 - C34, and C34 - C50 fractions are calculated using the average response factor for n-C10, n-C16, and n-C34.

Gravimetric Heavy Hydrocarbons are not included in the Total C16-C50 and are only determined if the chromatogram of the C34 - C50 hydrocarbons indicates that hydrocarbons >C50 are present. The chromatogram has returned to baseline by the retention time of nC50.

Total C6 - C50 results are corrected for BTEX contribution.

This method complies with the Reference Method for the CWS PHC and is validated for use in the laboratory.

nC6 and nC10 response factors are within 30% of Toluene response factor.

nC10, nC16 and nC34 response factors are within 10% of their average.

C50 response factor is within 70% of nC10 + nC16 + nC34 average.

Linearity is within 15%.

Extraction and holding times were met for this sample.

Fractions 1-4 are quantified without the contribution of PAHs. Under Ontario Regulation 153, results are considered valid without determining the PAH contribution if not requested by the client.

Analysis performed at AGAT Toronto (unless marked by *)

NPopukolof

Certified By:



ATTENTION TO: Alessandro Girardo

SAMPLED BY:

AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

O. Reg. 153(511) - VOCs (with PHC) (Soil)

DATE RECEIVED: 2024-01-31								ſ	DATE REPORTI	ED: 2024-02-06	
				SCRIPTION: MPLE TYPE: SAMPLED:	BH202-SS3 Soil 2024-01-30	BH205-SS2 Soil 2024-01-30	BH205-SS6 Soil 2024-01-30	BH209-SS2 Soil 2024-01-30	BH203-SS3 Soil 2024-01-30	BH210-SS2 Soil 2024-01-30	
Parameter	Unit	G / S: A	G / S: B	RDL	5614360	5614370	5614371	5614372	5614376	5614381	
Dichlorodifluoromethane	µg/g	0.05	1.5	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Vinyl Chloride	ug/g	0.02	0.02	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	
Bromomethane	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Trichlorofluoromethane	ug/g	0.25	0.25	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Acetone	ug/g	0.5	0.5	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50	
1,1-Dichloroethylene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	< 0.05	<0.05	<0.05	
Methylene Chloride	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Trans- 1,2-Dichloroethylene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Methyl tert-butyl Ether	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,1-Dichloroethane	ug/g	0.05	0.05	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	
Methyl Ethyl Ketone	ug/g	0.5	0.5	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50	
Cis- 1,2-Dichloroethylene	ug/g	0.05	0.05	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	
Chloroform	ug/g	0.05	0.05	0.04	<0.04	< 0.04	<0.04	< 0.04	<0.04	<0.04	
1,2-Dichloroethane	ug/g	0.05	0.05	0.03	<0.03	< 0.03	<0.03	<0.03	<0.03	<0.03	
1,1,1-Trichloroethane	ug/g	0.05	0.11	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Carbon Tetrachloride	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Benzene	ug/g	0.02	0.02	0.02	<0.02	<0.02	<0.02	<0.02	<0.02	<0.02	
1,2-Dichloropropane	ug/g	0.05	0.05	0.03	<0.03	< 0.03	<0.03	<0.03	<0.03	<0.03	
Trichloroethylene	ug/g	0.05	0.05	0.03	<0.03	< 0.03	< 0.03	<0.03	<0.03	<0.03	
Bromodichloromethane	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Methyl Isobutyl Ketone	ug/g	0.5	0.5	0.50	<0.50	<0.50	<0.50	<0.50	<0.50	<0.50	
1,1,2-Trichloroethane	ug/g	0.05	0.05	0.04	<0.04	< 0.04	<0.04	<0.04	<0.04	<0.04	
Toluene	ug/g	0.2	0.2	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Dibromochloromethane	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Ethylene Dibromide	ug/g	0.05	0.05	0.04	<0.04	<0.04	< 0.04	<0.04	<0.04	<0.04	
Tetrachloroethylene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,1,1,2-Tetrachloroethane	ug/g	0.05	0.05	0.04	< 0.04	< 0.04	< 0.04	<0.04	<0.04	<0.04	
Chlorobenzene	ug/g	0.05	0.083	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Ethylbenzene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
m & p-Xylene	ug/g			0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	

Certified By:

NPopukolof



AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

CLIENT NAME: EXP SERVICES INC

SAMPLING SITE:5100 Erin Mills

ATTENTION TO: Alessandro Girardo

SAMPLED BY:

O. Reg. 153(511) - VOCs (with PHC) (Soil)

DATE RECEIVED: 2024-01-31	TE RECEIVED: 2024-01-31							[DATE REPORTE	ED: 2024-02-06	
			SA	SCRIPTION: MPLE TYPE: E SAMPLED:	BH202-SS3 Soil 2024-01-30	BH205-SS2 Soil 2024-01-30	BH205-SS6 Soil 2024-01-30	BH209-SS2 Soil 2024-01-30	BH203-SS3 Soil 2024-01-30	BH210-SS2 Soil 2024-01-30	
Parameter	Unit	G / S: A	G / S: B	RDL	5614360	5614370	5614371	5614372	5614376	5614381	
Bromoform	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Styrene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,1,2,2-Tetrachloroethane	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
o-Xylene	ug/g			0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,3-Dichlorobenzene	ug/g	0.05	0.26	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,4-Dichlorobenzene	ug/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,2-Dichlorobenzene	ug/g	0.05	3.4	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Xylenes (Total)	ug/g	0.05	0.091	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
1,3-Dichloropropene (Cis + Trans)	µg/g	0.05	0.05	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
n-Hexane	µg/g	0.05	2.5	0.05	<0.05	<0.05	<0.05	<0.05	<0.05	<0.05	
Moisture Content	%			0.1	4.2	3.6	10.2	8.4	6.6	7.1	
Surrogate	Unit	A	cceptable Limi	its							
Toluene-d8	% Recovery		50-140		96	94	92	95	97	98	
4-Bromofluorobenzene	% Recovery		50-140		80	75	82	72	76	77	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: A Refers to Table 1: Full Depth Background Site Condition Standards - Soil -Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use, B Refers to O. Reg. 406/19 TABLE 2.1: Full Depth Potable Ground Water Condition Volume Independent - RP Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

5614360-5614381 The sample was analyzed using the high level technique. The sample was extracted using methanol, a small amount of the methanol extract was diluted in water and the purge & trap GC/MS analysis was performed. Results are based on the dry weight of the soil.

Xylenes total is a calculated parameter. The calculated value is the sum of m&p-Xylene + o-Xylene.

1,3-Dichloropropene total is a calculated parameter. The calculated value is the sum of Cis-1,3-Dichloropropene and Trans-1,3-Dichloropropene.

The calculated parameters are non-accredited. The parameters that are components of the calculation are accredited.

Certified By:

NPopukolof



Exceedance Summary

AGAT WORK ORDER: 24T116256 PROJECT: GTR-00257769-H0

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: EXP SERVICES INC

ATTENTION TO: Alessandro Girardo

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
5614359	BH202-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	2.4	3.08
5614370	BH205-SS2	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity (2:1)	mS/cm	0.7	1.04
5614370	BH205-SS2	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	5	7.94
5614370	BH205-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity (2:1)	mS/cm	0.57	1.04
5614370	BH205-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	2.4	7.94
5614371	BH205-SS6	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity (2:1)	mS/cm	0.7	1.10
5614371	BH205-SS6	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	5	12.8
5614371	BH205-SS6	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity (2:1)	mS/cm	0.57	1.10
5614371	BH205-SS6	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	2.4	12.8
5614372	BH209-SS2	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.)	N/A	5	12.3
5614372	BH209-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity (2:1)	mS/cm	0.57	0.697
5614372	BH209-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.)	N/A	2.4	12.3
5614373	BH203-SS2	ON 406/19 T2.1 RP	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.	N/A	5	5.96
5614373	BH203-SS2	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio (2:1) (Calc.)	N/A	2.4	5.96



Quality Assurance

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CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0

SAMPLING SITE:5100 Erin Mills

AGAT WORK ORDER: 24T116256 ATTENTION TO: Alessandro Girardo

SAMPLED BY:

			Soi	l Ana	alysis	5								
RPT Date:		6	UPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLANK		MAT	RIX SPI	KE
PARAMETER	Batch Sample	Dup #1	Dup #2	RPD	Method Blank	Measured		ptable nits	Recovery	Lin	ptable nits	Recovery		ptable nits
	Id					Value	Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inor	ganics (Soil)													
Antimony	5615305	<0.8	<0.8	NA	< 0.8	105%	70%	130%	104%	80%	120%	96%	70%	130%
Arsenic	5615305	4	4	NA	< 1	124%	70%	130%	101%	80%	120%	100%	70%	130%
Barium	5615305	133	138	3.7%	< 2.0	106%	70%	130%	102%	80%	120%	113%	70%	130%
Beryllium	5615305	1.0	0.9	NA	< 0.5	106%	70%	130%	117%	80%	120%	93%	70%	130%
Boron	5615305	7	9	NA	< 5	86%	70%	130%	107%	80%	120%	85%	70%	130%
Boron (Hot Water Soluble)	5614359 5614359	0.21	0.21	NA	< 0.10	101%	60%	140%	105%	70%	130%	95%	60%	140%
Cadmium	5615305	<0.5	<0.5	NA	< 0.5	114%	70%	130%	100%	80%	120%	101%	70%	130%
Chromium	5615305	34	35	2.9%	< 5	100%	70%	130%	99%	80%	120%	99%	70%	130%
Cobalt	5615305	12.4	12.7	2.4%	< 0.8	111%	70%	130%	103%	80%	120%	101%	70%	130%
Copper	5615305	17.9	18.7	4.4%	< 1.0	99%	70%	130%	99%	80%	120%	93%	70%	130%
Lead	5615305	15	15	0.0%	< 1	116%	70%	130%	93%	80%	120%	87%	70%	130%
Molybdenum	5615305	<0.5	<0.5	NA	< 0.5	119%	70%	130%	105%	80%	120%	107%	70%	130%
Nickel	5615305	23	24	4.3%	< 1	114%	70%	130%	105%	80%	120%	104%	70%	130%
Selenium	5615305	<0.8	<0.8	NA	< 0.8	121%	70%	130%	102%	80%	120%	101%	70%	130%
Silver	5615305	<0.5	<0.5	NA	< 0.5	105%	70%	130%	101%	80%	120%	97%	70%	130%
Thallium	5615305	<0.5	<0.5	NA	< 0.5	104%	70%	130%	97%	80%	120%	90%	70%	130%
Uranium	5615305	0.87	0.83	NA	< 0.50	124%	70%	130%	108%	80%	120%	108%	70%	130%
Vanadium	5615305	50.1	49.9	0.4%	< 2.0	109%	70%	130%	107%	80%	120%	99%	70%	130%
Zinc	5615305	75	78	3.9%	< 5	108%	70%	130%	102%	80%	120%	100%	70%	130%
Chromium, Hexavalent	5620358	<0.2	<0.2	NA	< 0.2	104%	70%	130%	87%	80%	120%	73%	70%	130%
Cyanide, WAD	5615066	<0.040	<0.040	NA	< 0.040	107%	70%	130%	101%	80%	120%	92%	70%	130%
Mercury	5615305	<0.10	<0.10	NA	< 0.10	120%	70%	130%	102%	80%	120%	108%	70%	130%
Electrical Conductivity (2:1)	5614359 5614359	0.282	0.345	20.1%	< 0.005	89%	80%	120%						
Sodium Adsorption Ratio (2:1) (Calc.)	5614359 5614359	3.08	3.34	8.1%	NA									
pH, 2:1 CaCl2 Extraction	5615975	6.32	6.60	4.3%	NA	102%	80%	120%						

Comments: NA signifies Not Applicable.

pH duplicates QA acceptance criteria was met relative as stated in Table 5-15 of Analytical Protocol document.

Duplicate NA: results are under 5X the RDL and will not be calculated.

Sulphate														
Sulphate (2:1)	5615730	72	72	0.0%	< 2	103%	70%	130%	92%	80%	120%	94%	70%	130%

Certified By:



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AGAT QUALITY ASSURANCE REPORT (V1)

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Quality Assurance

CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0

SAMPLING SITE:5100 Erin Mills

AGAT WORK ORDER: 24T116256 ATTENTION TO: Alessandro Girardo SAMPLED BY:

Trace Organics Analysis

			Trac	e Or	yanı	cs Ai	laiys	15							
RPT Date:			C	UPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLAN		MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured		eptable nits	Recovery	1 1 10	eptable nits	Recovery		eptable nits
		ld					Value	Lower	Upper		Lower	Upper		Lower	Uppe
O. Reg. 153(511) - PAHs (Soil)															
Naphthalene	5620834		<0.05	<0.05	NA	< 0.05	71%	50%	140%	85%	50%	140%	73%	50%	140%
Acenaphthylene	5620834		<0.05	<0.05	NA	< 0.05	67%	50%	140%	103%	50%	140%	95%	50%	140%
Acenaphthene	5620834		<0.05	<0.05	NA	< 0.05	95%	50%	140%	100%	50%	140%	83%	50%	140%
Fluorene	5620834		<0.05	<0.05	NA	< 0.05	107%	50%	140%	105%	50%	140%	88%	50%	140%
Phenanthrene	5620834		<0.05	<0.05	NA	< 0.05	83%	50%	140%	75%	50%	140%	93%	50%	140%
Anthracene	5620834		<0.05	<0.05	NA	< 0.05	113%	50%	140%	95%	50%	140%	80%	50%	140%
Fluoranthene	5620834		<0.05	<0.05	NA	< 0.05	108%	50%	140%	88%	50%	140%	75%	50%	1409
Pyrene	5620834		<0.05	<0.05	NA	< 0.05	90%	50%	140%	73%	50%	140%	98%	50%	140%
Benz(a)anthracene	5620834		<0.05	<0.05	NA	< 0.05	113%	50%	140%	73%	50%	140%	78%	50%	140%
Chrysene	5620834		<0.05	<0.05	NA	< 0.05	74%	50%	140%	80%	50%	140%	100%	50%	140%
Benzo(b)fluoranthene	5620834		<0.05	<0.05	NA	< 0.05	106%	50%	140%	85%	50%	140%	73%	50%	140%
Benzo(k)fluoranthene	5620834		<0.05	<0.05	NA	< 0.05	95%	50%	140%	108%	50%	140%	80%	50%	140%
Benzo(a)pyrene	5620834		<0.05	<0.05	NA	< 0.05	83%	50%	140%	100%	50%	140%	75%	50%	140%
Indeno(1,2,3-cd)pyrene	5620834		<0.05	<0.05	NA	< 0.05	101%	50%	140%	95%	50%	140%	80%	50%	140%
Dibenz(a,h)anthracene	5620834		<0.05	<0.05	NA	< 0.05	97%	50%	140%	100%	50%	140%	108%	50%	140%
Benzo(g,h,i)perylene	5620834		<0.05	<0.05	NA	< 0.05	98%	50%	140%	95%	50%	140%	100%	50%	140%
O. Reg. 153(511) - PHCs F1 - F	4 (with VOC) (Soil)													
F1 (C6 to C10)	5613514		<5	<5	NA	< 5	86%	60%	140%	105%	60%	140%	102%	60%	140%
F2 (C10 to C16)	5614495		< 10	< 10	NA	< 10	115%	60%	140%	110%	60%	140%	110%	60%	140%
F3 (C16 to C34)	5614495		< 50	< 50	NA	< 50	112%	60%	140%	118%	60%	140%	120%	60%	1409
F4 (C34 to C50)	5614495		< 50	< 50	NA	< 50	72%	60%	140%	83%	60%	140%	81%	60%	140%
O. Reg. 153(511) - VOCs (with	PHC) (Soil)														
Dichlorodifluoromethane	5613514		<0.05	<0.05	NA	< 0.05	108%	50%	140%	102%	50%	140%	85%	50%	140%
Vinyl Chloride	5613514		<0.02	<0.02	NA	< 0.02	95%	50%	140%	103%	50%	140%	113%	50%	140%
Bromomethane	5613514		<0.05	<0.05	NA	< 0.05	97%	50%	140%	97%	50%	140%	90%	50%	140%
Trichlorofluoromethane	5613514		<0.05	<0.05	NA	< 0.05	103%	50%	140%	91%	50%	140%	110%	50%	1409
Acetone	5613514		<0.50	<0.50	NA	< 0.50	94%	50%	140%	105%	50%	140%	112%	50%	140%
1,1-Dichloroethylene	5613514		<0.05	<0.05	NA	< 0.05	100%	50%	140%	86%	60%	130%	86%	50%	1409
Methylene Chloride	5613514		<0.05	<0.05	NA	< 0.05	108%	50%	140%	96%	60%	130%	94%	50%	140%
Trans- 1,2-Dichloroethylene	5613514		<0.05	<0.05	NA	< 0.05	82%	50%	140%	90%	60%	130%	85%	50%	140%
Methyl tert-butyl Ether	5613514		<0.05	<0.05	NA	< 0.05	69%	50%	140%	97%	60%	130%	72%	50%	140%
1,1-Dichloroethane	5613514		<0.02	<0.02	NA	< 0.02	102%	50%	140%	93%	60%	130%	93%	50%	140%
Methyl Ethyl Ketone	5613514		<0.50	<0.50	NA	< 0.50	105%	50%	140%	94%	50%	140%	110%	50%	140%
Cis- 1,2-Dichloroethylene	5613514		<0.02	<0.02	NA	< 0.02	66%	50%	140%	77%	60%	130%	66%	50%	140%
Chloroform	5613514		<0.04	<0.04	NA	< 0.04	84%	50%	140%	75%	60%	130%	106%	50%	140%
1,2-Dichloroethane	5613514		<0.03	< 0.03	NA	< 0.03	78%	50%	140%	67%	60%	130%	97%	50%	140%
1,1,1-Trichloroethane	5613514		<0.05	<0.05	NA	< 0.05	87%	50%	140%	60%	60%	130%	88%	50%	140%
Carbon Tetrachloride	5613514		<0.05	<0.05	NA	< 0.05	101%	50%	140%	88%	60%	130%	81%	50%	140%
AGAT QUALITY ASSUR	ANCE REPOR	T (V1)											Р	age 13	of 19

AGAT QUALITY ASSURANCE REPORT (V1)

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Quality Assurance

CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0

SAMPLING SITE:5100 Erin Mills

AGAT WORK ORDER: 24T116256 ATTENTION TO: Alessandro Girardo SAMPLED BY:

Trace Organics Analysis (Continued)

			<u>U</u>				•								
RPT Date:			C	OUPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured Value		ptable nits	Recovery	Lin	ptable nits	Recovery	Lin	eptable nits
		Ia	-	-			value	Lower	Upper	-	Lower	Upper	-	Lower	Upper
Benzene	5613514		<0.02	<0.02	NA	< 0.02	67%	50%	140%	109%	60%	130%	91%	50%	140%
1,2-Dichloropropane	5613514		<0.03	<0.03	NA	< 0.03	70%	50%	140%	102%	60%	130%	87%	50%	140%
Trichloroethylene	5613514		<0.03	<0.03	NA	< 0.03	82%	50%	140%	100%	60%	130%	98%	50%	140%
Bromodichloromethane	5613514		<0.05	<0.05	NA	< 0.05	88%	50%	140%	86%	60%	130%	93%	50%	140%
Methyl Isobutyl Ketone	5613514		<0.50	<0.50	NA	< 0.50	99%	50%	140%	95%	50%	140%	96%	50%	140%
1,1,2-Trichloroethane	5613514		<0.04	<0.04	NA	< 0.04	80%	50%	140%	90%	60%	130%	100%	50%	140%
Toluene	5613514		<0.05	<0.05	NA	< 0.05	111%	50%	140%	101%	60%	130%	105%	50%	140%
Dibromochloromethane	5613514		<0.05	<0.05	NA	< 0.05	89%	50%	140%	94%	60%	130%	93%	50%	140%
Ethylene Dibromide	5613514		<0.04	<0.04	NA	< 0.04	84%	50%	140%	102%	60%	130%	74%	50%	140%
Tetrachloroethylene	5613514		< 0.05	< 0.05	NA	< 0.05	89%	50%	140%	90%	60%	130%	93%	50%	140%
1,1,1,2-Tetrachloroethane	5613514		<0.04	<0.04	NA	< 0.04	84%	50%	140%	97%	60%	130%	103%	50%	140%
Chlorobenzene	5613514		<0.05	<0.05	NA	< 0.05	76%	50%	140%	104%	60%	130%	81%	50%	140%
Ethylbenzene	5613514		<0.05	<0.05	NA	< 0.05	83%	50%	140%	96%	60%	130%	82%	50%	140%
m & p-Xylene	5613514		<0.05	<0.05	NA	< 0.05	106%	50%	140%	108%	60%	130%	106%	50%	140%
Bromoform	5613514		<0.05	<0.05	NA	< 0.05	92%	50%	140%	90%	60%	130%	85%	50%	140%
Styrene	5613514		<0.05	<0.05	NA	< 0.05	89%	50%	140%	79%	60%	130%	85%	50%	140%
1,1,2,2-Tetrachloroethane	5613514		<0.05	<0.05	NA	< 0.05	98%	50%	140%	94%	60%	130%	90%	50%	140%
o-Xylene	5613514		<0.05	<0.05	NA	< 0.05	99%	50%	140%	91%	60%	130%	104%	50%	140%
1,3-Dichlorobenzene	5613514		<0.05	<0.05	NA	< 0.05	89%	50%	140%	97%	60%	130%	87%	50%	140%
1,4-Dichlorobenzene	5613514		<0.05	<0.05	NA	< 0.05	90%	50%	140%	74%	60%	130%	83%	50%	140%
1,2-Dichlorobenzene	5613514		<0.05	<0.05	NA	< 0.05	83%	50%	140%	75%	60%	130%	103%	50%	140%
n-Hexane	1		< 0.05	< 0.05	NA	< 0.05	93%	50%	140%	85%	60%	130%	101%	50%	140%
4-Bromofluorobenzene	5613514		77	73	5.7%	< 1	NA			NA			106%		
O. Reg. 153(511) - PHCs F1 - F4	(with PAHs	and VOC)	(Soil)												
F1 (C6 to C10)	5613514		<5	<5	NA	< 5	86%	60%	140%	105%	60%	140%	102%	60%	140%
O. Reg. 153(511) - PCBs (Soil)															
Polychlorinated Biphenyls	5620362		< 0.1	< 0.1	NA	< 0.1	106%	50%	140%	91%	50%	140%	93%	50%	140%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

NPopukot

AGAT QUALITY ASSURANCE REPORT (V1)

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CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

Method Summary

AGAT WORK ORDER: 24T116256

ATTENTION TO: Alessandro Girardo

SAMPLING SITE:5100 Erin Mills		SAMPLED BY:				
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE			
Soil Analysis	•					
Antimony	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Arsenic	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Barium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Beryllium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Boron	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Boron (Hot Water Soluble)	MET-93-6104	modified from EPA 6010D and MSA PART 3, CH 21	ICP/OES			
Cadmium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Chromium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Cobalt	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Copper	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Lead	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Molybdenum	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Nickel	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Selenium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Silver	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Thallium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Uranium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Vanadium	MET-93-6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Zinc	MET 93 -6103	modified from EPA 3050B and EPA 6020B and ON MOECC	ICP-MS			
Chromium, Hexavalent	INOR-93-6068	modified from EPA 3060 and EPA 7196	SPECTROPHOTOMETER			
Cyanide, WAD	INOR-93-6052	modified from ON MOECC E3015, SM 4500-CN- I, G-387	SEGMENTED FLOW ANALYSIS			
Mercury	MET-93-6103	modified from EPA 7471B and SM 3112 B	ICP-MS			
Electrical Conductivity (2:1)	INOR-93-6075	modified from MSA PART 3, CH 14 and SM 2510 B	PC TITRATE			
Sodium Adsorption Ratio (2:1) (Calc.)	INOR-93-6007	modified from EPA 6010D & Analytica Protocol	I ICP/OES			
pH, 2:1 CaCl2 Extraction	INOR-93-6075	modified from EPA 9045D, MCKEAGUE 3.11 E3137	PC TITRATE			
Sulphate (2:1)	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH			



Method Summary

CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0 SAMPLING SITE:5100 Erin Mills

AGAT WORK ORDER: 24T116256

SAMPLED BY:

ATTENTION TO: Alessandro Girardo

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Trace Organics Analysis		modified from EDA 2570 and EDA	
Naphthalene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Acenaphthylene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Acenaphthene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Fluorene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Phenanthrene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Anthracene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Fluoranthene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Pyrene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Benz(a)anthracene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Chrysene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Benzo(b)fluoranthene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Benzo(k)fluoranthene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Benzo(a)pyrene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Indeno(1,2,3-cd)pyrene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Dibenz(a,h)anthracene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Benzo(g,h,i)perylene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
2-and 1-methyl Naphthalene	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Naphthalene-d8	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Acridine-d9	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Terphenyl-d14	ORG-91-5106	modified from EPA 3570 and EPA 8270E	GC/MS
Moisture Content	VOL-91-5009	modified from CCME Tier 1 Method	BALANCE
Polychlorinated Biphenyls	ORG-91-5113	modified from EPA SW-846 3570 & 8082A	GC/ECD
Decachlorobiphenyl	ORG-91-5113	modified from EPA SW-846 3541 & 8082A	GC/ECD
F1 (C6 to C10)	VOL-91-5009	modified from CCME Tier 1 Method	(P&T)GC/FID
F1 (C6 to C10) minus BTEX	VOL-91-5009	modified from CCME Tier 1 Method	P&T GC/FID
Toluene-d8	VOL-91- 5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
F2 (C10 to C16)	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID
F2 (C10 to C16) minus Naphthalene	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID
F3 (C16 to C34)	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID
F3 (C16 to C34) minus PAHs	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID
F4 (C34 to C50)	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID



Method Summary

CLIENT NAME: EXP SERVICES INC

PROJECT: GTR-00257769-H0

AGAT WORK ORDER: 24T116256 ATTENTION TO: Alessandro Girardo

SAMPLING SITE:5100 Erin Mills		SAMPLED BY:	- 1		
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE		
Gravimetric Heavy Hydrocarbons	VOL-91-5009	modified from CCME Tier 1 Method	BALANCE		
Terphenyl	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID		
F1 (C6 to C10) minus BTEX	VOL-91-5009	modified from CCME Tier 1 Method	(P&T)GC/FID		
F3 (C16 to C34)	VOL-91-5009	modified from CCME Tier 1 Method	GC/FID		
Dichlorodifluoromethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Vinyl Chloride	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Bromomethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Trichlorofluoromethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Acetone	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,1-Dichloroethylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Methylene Chloride	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Trans- 1,2-Dichloroethylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Methyl tert-butyl Ether	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,1-Dichloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Methyl Ethyl Ketone	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Cis- 1,2-Dichloroethylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Chloroform	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,2-Dichloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,1,1-Trichloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Carbon Tetrachloride	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Benzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,2-Dichloropropane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Trichloroethylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Bromodichloromethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Methyl Isobutyl Ketone	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
1,1,2-Trichloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Toluene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Dibromochloromethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Ethylene Dibromide	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		
Tetrachloroethylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS		



Method Summary

CLIENT NAME: EXP SERVICES INC PROJECT: GTR-00257769-H0

AGAT WORK ORDER: 24T116256 ATTENTION TO: Alessandro Girardo

SAMPLING SITE:5100 Erin Mills		SAMPLED BY:	
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
1,1,1,2-Tetrachloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Chlorobenzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Ethylbenzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
m & p-Xylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Bromoform	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Styrene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
1,1,2,2-Tetrachloroethane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
o-Xylene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
1,3-Dichlorobenzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
1,4-Dichlorobenzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
1,2-Dichlorobenzene	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Xylenes (Total)	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
1,3-Dichloropropene (Cis + Trans)	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
n-Hexane	VOL-91-5002	modified from EPA 5035A and EPA 8260D	(P&T)GC/MS
Toluene-d8	VOL-91-5002	modified from EPA 5035A & EPA 8260D	(P&T)GC/MS
4-Bromofluorobenzene	VOL-91-5002	modified from EPA 5035A & EPA 8260D	(P&T)GC/MS

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5835 Coopers Avenue Mississauga, Ontario L4Z 1Y2 005 740 5400

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Contact: Address: Address: 1595 CLARI LGT 4V1 905-793-90 alessendos 2. Email:	CO Fax:		- 12	- 7 - [- [- Soil	Regulation 153/04 able	Regulation 406 Table 2.1 Regulation 556	<u>S-</u>	S	ver Use anitary Region v. Water ectives (er Indicate C	Qualit PWQC	y		Tu Rej	rnar gulai sh T/	r TAT T(Rush Busine Days	Surcharg CSS	e (TAT)) Req to 7 Bus Busines ays	uired siness [ss	: Days	t Business y):
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Document ID: DN-78-1511-022 Any and all products and/or services provided by AGAT Labs are pursuant to the terms and conditions as set forth at www.agatlabs.com/termsandconditions unless otherwise agreed in a current written contractual document.

Phase Two Environmental Site Assessment 2670 & 2690 Erin Center Boulevard, Mississauga, ON GTR-00257769-H0 March 28, 2024

Appendix F – Phase Two Conceptual Site Model





Phase Two Conceptual Site Model – 2670 & 2690 Erin Center Boulevard, Mississauga, ON

This section presents the Phase Two Conceptual Site Model (CSM), as it relates to the portion of the property addressed as 2670 & 2690 Erin Center Boulevard, Mississauga, Ontario identified (herein after referred to as the 'Site' or 'RSC Property'). The Phase Two CSM has been developed from observations made during the Phase Two ESA dated March 27, 2024. The Phase Two CSM provides a narrative and graphical interpretation of the RSC property surface features, near surface geologic and hydrogeological conditions, potential contaminants of concern, contaminant fate and transport mechanisms and relevant receptors and exposure pathways. These components are discussed in the following sections.

1 Introduction

The Site is an irregular shaped parcel of land that measures approximately 4.28 hectares (10.58 acres) in size. The Site is located on the southeast corner of Glen Erin Drive and Erin Center Boulevard in the city of Mississauga, Ontario, as indicated on Figures 1 and 2. The RSC Property described above is an irregularly shaped parcel of land with an area of approximately 4.85 hectares (11.98 acres).

Refer to Table 1 for the Site identification information.

Table 1: Site Identification Information

Municipal Address	2670 & 2690 Erin Centre Boulevard, Mississauga, Ontatio
Current Land Use	Commercial Use (Retail Stores, Restaurants, Medical Walk-in Clinic)
Proposed Land Use	Mixed- Use Residential
Legal Description	Blk 1, Pl 43m823 Save And Except Pts 1, 2, Pl 43r16736, Pts 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, Pl 43r35938; Blk 20, Pl 43m823; Pt Blk 5, Pl 43m823 (0.30 Reserves) Des As Pt 6, Pl 43r20654; Pt Of Eglinton Avenue Being Pt Lt 2, Registrar's Compiled Pl 1003, Des As Pts 2, 4, 7, 8, 11, 13 Pl 43r20654, Save And Except Pts 19, 20, Pl 43r35938 As Closed By Bylaw No. Lt1580524; Pt Blk 18, Pl 43m823 (0.30 Reserve) Des As Pt 14, Pl 43r20654; Blk 4, Pl 43m823 (0.30 Reserve); Blk 17, Pl 43m823 (0.30 Reserve), Save And Except Pts 16, 17, 18, Pl 43r35938; Blk 6, Pl 43m823 Subject To An Easement As In Vs162364 Subject To An Easement Over Pts 9, 10, 11, 12, 13, 14, 15, 24, 26, 27, 28, 29, 30, 31, 32, 33, Pl 43r15700 And Pts 10, 11, 12, 13, 14, 15, 16, 17, Pl 43r16222, Save And Except Pts 1, 2, 3, 9, 10, 11, Pl 43r35938 In Favour Of Mississauga Hydro-electric Commission And Bell Canada As In Lt955564 Subject To An Easement Over Pt 1, Pl 43r16217, Pt 1, Pl 43r16218, Pt 1, Pl 43r16219 In Favour Of The Corporation Of The City Of Mississauga As In Lt955566 Subject To An Easement Over Pts 4, 5, 8, 9, Pl 43r16222, Save And Except Pts 1, 9, Pl 43r35938 In Favour Of The City Of Mississauga As In Lt955568 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r20653 In Favour Of Mississauga Hydro-electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r20653 In Favour Of Mississauga Hydro-electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r206553 In Favour Of Mississauga Hydro-electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r206553 In Favour Of Mississauga Hydro-electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r206553 In Favour Of Mississauga Hydro-electric Commission As In Lt1

	3, 4, Pl 43r20654 In Favour Of The Regional Municipality Of Peel As In Lt1741002 Subject To An Easement Over Pts 2, 3, 7, Lpl 43r16222 And Pts 16, 17, 18, 21, 22, 34, 35, 38, Pl 43r15700, Save And Except Pts 1, 4, 5, 6, 7, 8, 9, 12, 13, 14, 15, pl 43r35938 In Favour Of The Regional Municipality Of Peel As In Lt1007104 Subject To An Easement Over Pt 5, Pl 43r16736 In Favour Of Mississauga Hydro-electric Commission As In Lt1130471 Together With An Easement As In Ro551159 Subject To An Easement In Gross Over Parts 1,2,3,4 And 5, 43r39842 As In Pr3811615 City Of Mississauga
Property Identification Number (PIN)	Part of PIN 1351 – 20035 (LT)
Approximate Universal Transverse Mercator (UTM) coordinates	NAD83 17T 603714.75 m E m 4823575.93 m N Google Earth
Accuracy Estimate of UTM	10-15 m
Measurement Method	Google Earth
Site Area	4.28 hectares (10.58 acres)
Property Owners, Owner Contact and Address	Mr. Barry Stern EMTC Holdings Inc. c/o The Muzzo Group of Companies 50 Confederation Parkway Concord, ON, L4K 4T8

2 Phase One Conceptual Site Model

This section presents the Phase One Conceptual Site Model (P1CSM), as it relates to the RSC Property. It has been developed from observations made during the Phase One ESA, dated March 26, 2024. The P1CSM provides a narrative and graphical description integrating information related to the areas of potential environmental concern/potential contaminating activities and the presence and distribution of potential contaminants of concern. The P1CSM was completed in accordance with Ontario Regulation 153/04, as amended (O.Reg.153/04), as defined by the Ontario Ministry of the Environment, Conservation and Parks (MECP).

2.1 Background

Based on a review of historical aerial photographs, chain of title information, historical maps, and other records, the Site was first developed circa 1990s. The Site was developed with two (2) Site buildings – A & B and a parking lot, inferred to be used for commercial use. At the time of the Site visit, the Site buildings were developed with slab-on grade single-storey buildings. Site Building A has a footprint of approximately 4,357 m2 (46,899 ft2) and Site Building B has a footprint of approximately 1,621 m2 (17,447 ft2). It is to be noted that only tenant - Home sense located in Site Building A has a mezzanine in the northern part of the building that is used for office space and employees lounge / lunch area. The mezzanine is equipped with basic home appliances such as refrigerator, microwave and coffee makers. The northeastern portion of the Site is asphalt-paved with a narrow-landscaped area located on the Site boundaries. The southwestern portion of the Site is landscaped with grass area.

2.2 Geological and Hydrogeological Conditions

Based on a review of the topographic map, the inferred groundwater flow at the Site and surrounding properties is anticipated to flow in an easterly direction. A permanent stream is located approximately 150 m east of the Site, and it flows in a southeasterly direction towards Millet Creek draining in Lake Ontario. The Site is approximately 177 m above sea level and mostly



flat. The Site and surrounding areas are dominated by Till Moraines deposits that consist predominantly of clay to silt-textured till (derived from glaciolacustrine deposits or shale). The bedrock in the general area of the Site is part of a group belonging to the Queenston Formation, primarily consisting of shale, limestone, dolostone and siltstone. Based on a review of "Bedrock Topography and Overburden Thickness Mapping, Southern Ontario, prepared by Ontario Geological Survey, published 2006," the bedrock in the vicinity of the Site is anticipated to be encountered at a depth of approximately 11 metres below ground surface (mbgs).

2.3 Previous Investigations

Several previous environmental reports of the Site were provided to EXP by the Client. It is noted that the previous reports were completed for the larger Erim Mills Town Centre Complex which included the Site. Significant findings are summarized below.

- A report entitled "Phase I Environmental Site Assessment, 5100 Erin Mills Parkway & 2690 Erin Centre Boulevard, Mississauga, Ontario" dated April 28, 2014, was prepared for OPB (EMTC) Inc. by its agent and manager 20 Vic Management Inc. by Pinchin Environmental. Pertinent information from the report is as follows:
 - The subject property consisted of a multi-tenant commercial building at 5100 Erin Mills Parkway, and a one -storey multi-tenant commercial building at 2690 Erin Centre Boulevard.
 - Three (3) generator rooms with AST containing diesel fuel were identified at 5100 Erin Mills Parkway. These ASTs are located off-Site.
 - 5100 Erin mills Parkway was constructed in 1989, and 2690 Erin Centre Boulevard in 1993.
 - The tenant, London Cleaners, located on-Site, was reportedly a dry-cleaning drop-off depot.
 - The property at 2670 Erin Centre Boulevard was listed as being occupied by a dry cleaner, however according to Site Representative it was a dry-cleaning depot.
 - Based on the previous reports, the Site had a geotechnical investigation which noted the stratigraphy as clayey silt fill, compact to dense till to approximately 1.7 to 5.5 m bgs, followed by red shale bedrock with limestone interbeds. Groundwater ranged between 3.4 to 4.2 m bgs.
 - Based on the findings of the Phase I ESA, no potential environmental concerns were identified.
- A report entitled "Phase I Environmental Site Assessment Update, Erin Mills Shopping Centre, Mississauga, Ontario" dated November 5, 2018, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Golder. Pertinent information from the report is as follows:
 - The subject property was noted to consist of the municipal address 5100 Erin Mills Parkway.
 - Based on the ERIS report, Loblaws Properties Ltd. At the Pumps, located approximately 30 m southwest of the Site at 5010 Glen Erin Drive, was listed as a gasoline service station with two (2) liquid fuel tanks, with expiration November 11, 2010.
 - Based on the findings, no issues of potential environmental concern were identified for the Site.
- A report entitled "Waste Audit Report, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, ON, L5M 4Z5", by Green for Life Environmental (GFL) in January 2019. No pertinent information related to an environmental concern for the Site was identified.
- A report entitled "Investigation of Indoor Air Quality, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 26, 2019, was prepared for Cushman & Wakefield Ltd. by Pinchin. Pertinent information from the report is as follows:
 - The survey addressed all common areas in the shopping mall.
 - Elevated dust measurements were recorded in several areas of the building.



- Total Volatile Organic Compounds (TVOC) measurements were slightly elevated on the lower level in the vicinity of a fragrance shop and an eyeglass clinic.
- A report entitled "Asbestos Reassessment, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 28, 2019, was prepared for Cushman & Wakefield Asset Services by Pinchin. Pertinent information from the report is as follows:
 - No samples were collected as part of the assessment. The purpose of the report was to assess changes in the condition and quantity of previously reported asbestos-containing materials.
 - Asbestos cement (transite) rainwater leaders are present in various locations in the building, in good condition.
- A report entitled "Water Sample Collection and Analysis for Lead, Erin Mills Town Centre", dated March 4, 2019, was
 prepared for Cushman & Wakefield by Ontario Environmental & Safety Network Ltd. No pertinent information related to
 an environmental concern for the Site was identified.
- A report entitled "Test Pit Program, Erin Mills Town Centre (Former Sears), 5100 Erin Mills Parkway, Mississauga, Ontario", dated March 6, 2020, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Pinchin. Pertinent information from the report is as follows:
 - Sixteen (16) bulk soil samples were submitted for PHCs, VOCs, PAH, and metal and inorganics to a maximum depth of 4.27 m bgs using an excavator.
 - The soil samples submitted met Table 3 SCS with the exception of Electric Conductivity (EC) and Sodium Adsorption Ratio (SAR) exceedance. These exceedances can be discounted based on the recent exemption that was passed by the MECP associated with areas that have been subject to the application of de-icing products to walkways and driveways (eg. Road salt).
- A report entitled *"Phase I Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).
 - The Phase I ESA was prepared for Muzzo Group with the objective to identify issues of environmental concern to the Site in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
 - Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good condition. Floor drains were noted to be in close proximity to the AST's. These ASTs are located off-Site.
 - Based on the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants identified from the ERIS report and the potential fill materials used during development on-Site were considered a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for groundwater at the Site.
 - Based on the findings of the Phase I ESA, a Phase II was recommended.
- A report entitled *"Phase II Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.



- The report was prepared for Muzzo Group to support identified on-Site concerns of waste generation, and off-Site concerns of a nearby gas station and hydro one facility. Based on recommendations made in the Phase I ESA (EXP, 2020).
- Between March 2 and 6, 2020, nine (9) boreholes were advanced at the Site (BH1, BH2, BH5, BH6, BH9, BH11, , BH13, BH17 and BH20). Five (5) of the boreholes were equipped with monitoring wells (BH1, BH5, BH9, BH17 and BH20).
- Soil samples were collected and submitted for the analysis of one or more of the following: Petroleum Hydrocarbons (PHCs), Volatile Organic Compounds (VOCs), Metals and Inorganics (M&I), Polycyclic Aromatic Hydrocarbons (PAHs), Polychlorinated Biphenyls (PCBs), pH, and grain-size (75-micron sleeve).
- Groundwater samples were collected from one (1) monitoring well (MW17) and analyzed for PHC, Metals and VOCs. It was noted that the collection of groundwater from MW1, MW5, MW9 and MW20 were attempted on two (2) separate occasions, however, groundwater was not encountered at these locations.
- Groundwater flow direction could not be determined as three (3) water levels screened within the sample geological formation are required to determine groundwater flow at the Site. Based on local topography, groundwater was anticipated to flow in a northeast direction towards a tributary of the Credit River.
- The concentrations of all parameters submitted for the soil samples from the boreholes were either below the applicable MECP (2011) Table 7 Site Condition Standards (SCS) for residential/parkland/institutional property with medium to fine soil, or not detected above the laboratory detection limits, with the exception of the following:
 - Electrical Conductivity (EC) and Sodium Adsorption Ratio (SAR) in BH5 SS2 (0.8 1.5 m bgs), BH9 SS2 (0.8 1.5 m bgs), BH11 SS2 (0.8 1.5 m bgs), and BH13 SS2 (0.8 1.5 m bgs). It was inferred the high concentrations were due to de-salting activities for road safety or for pedestrian walkways. As such, the elevated EC/SAR in soil was not considered an exceedance based on section 49.1 of O.Reg. 407/19.
- The concentrations of all parameters submitted for groundwater for one (1) monitoring well were either below the applicable MECP (2022) Table 7 SCS or not detected above laboratory detection limits.
- Based on the results of the Phase II ESA (EXP, 2020), no further work was recommended at the time. It was noted that, should an RSC be required for the Site or smaller portions of the Site, additional environmental work would be required to support those submissions.
- A report entitled *"Phase I Environmental Site Assessment Update, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated October 18, 2022, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).
 - The Phase I ESA Update was prepared for Muzzo Group for due diligence purposes in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
 - Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good condition. Floor drains were noted to be in close proximity to the AST's. These ASTs are not located on-Site.
 - According to information provided by the Site Representative and based on a reviewed Environmental Control Incident Report provided by the MECP, the spill occurred on November 27, 2021, during the construction activities in the corridor of mall entrance F in the west portion of 5100 Erin Mills Parkway, a diesel-filled construction heater was temporarily stationed on the concrete walkway adjacent to the building.
 - EXP was made aware that since the previous Phase I ESA, there was a fire On-Site Fire at 2575 Erin Mills Parkway in the building in 2021.

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- Similar to the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants
 identified from the ERIS report and the potential fill materials used during development on-Site were considered
 a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent
 to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for
 groundwater at the Site.
- Based on the Phase I ESA Update (EXP, 2022) findings, the fire and spill are considered a low concern and immediate action is not required at this time. However, it was noted that this will need to be identified as an Area of Potential Environmental Concern (APEC) in any future O. Reg 153/04 compliant Phase One ESA and investigated through a Phase Two ESA both of which will be required for the purpose of filing an RSC.
 - $\circ~$ It is noted that based on the nature and distance from the spill and the fire to the Site, neither we considered to contribute to an APEC.

2.4 Underground Utilities

The Site utilities and services were identified at the Site based on information provided in environmental records, relevant utility infrastructure observed during the Site reconnaissance. The Site utilities are summarized in the table below and noted on Figure 3, where available. It is noted that the precise underground location of the utilities cannot be determined without professional locate services.

Utility	Source	Location	Site Entry
Natural Gas	Enbridge Gas	Underground	The natural gas meter was observed on the eastern wall of the Site building A and northern wall of Site Building B.
Sanitary Sewer	City of Mississauga	Underground	Unknown
Storm Sewer	City of Mississauga	Underground	Various storm sewers observed in the asphalt portion of the Site
Water	City of Mississauga	Underground	Unknown
Electricity	Unknown	Underground	Unknown
Telecommunications	Unknown	Underground	Unknown

2.5 Potentially Contaminating Activities and Areas of Potential Environmental Concern

3 Phase One ESA Conceptual Site Model – 2670 & 2690 Erin Center Boulevard, Mississauga, ON

This section presents the Phase One Conceptual Site Model (P1CSM), as it relates to the Site designated as 2670 & 2690 Erin Center Boulevard in Mississauga, Ontario providing a narrative and graphical description integrating information related to the areas of potential environmental concern/potential contaminating activities and the presence and distribution of potential contaminants of concern. The P1CSM was completed in accordance with Ontario Regulation 153/04, as amended (O.Reg.153/04), as defined by the Ontario Ministry of the Environment, Conservation and Parks (MECP).

3.1 Introduction

The Site includes municipal addresses 2670 & 2690 Erin Center Boulevard but is hereinafter collectively referred to as the 'Site'. The Site is located on the southeast corner of Glen Erin Drive and Erin Center Boulevard in the city of Mississauga, Ontario. The

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Site is an irregular shaped parcel of land that measures approximately 4.28 hectares (10.58 acres) in size. At the time of this investigation, the Site consisted of the following:

- The northeastern portion of the Site is asphalt-paved with a narrow-landscaped area located on the Site boundaries.
- The southwestern portion of the Site is landscaped with grass area.
- The central portion of the Site is developed with two (2) Site buildings that have the municipal addresses 2670 Erin Center Boulevard (Site Building A) and 2690 Erin Center Boulevard (Site Building B) and asphalt-paved with parking area towards central-east. The tenants include:
 - 2670 Erin Center Boulevard (Site Building A): Dollarama, Bouclair Home, and Home Sense.
 - 2690 Erin Center Boulevard (Site Building B): Pizza Pizza, Hero's Burger, Walk-in Medical Clinic, Cosmetic Dentistry, Studio 10 Salon, Chiropractor Practice Center.

The areas surrounding the Site buildings have been paved with asphalt and landscaped areas are located along the perimeter of the Site.

Details of the Site are as follows:

2670 & 2690 Erin Centre Boulevard, Mississauga, Ontatio
Commercial Use (Retail Stores, Restaurants, Medical Walk-in Clinic)
Mixed-Use Residential
Blk 1, Pl 43m823 Save And Except Pts 1, 2, Pl 43r16736, Pts 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, Pl 43r35938; Blk 20, Pl 43m823; Pt Blk 5, Pl 43m823 (0.30 Reserves) Des As Pt 6, Pl 43r20654; Pt Of Eglinton Avenue Being Pt Lt 2, Registrar's Compiled Pl 1003, Des As Pts 2, 4, 7, 8, 11, 13 Pl 43r20654, Save And Except Pts 19, 20, Pl 43r35938 As Closed By Bylaw No. Lt1580524; Pt Blk 18, Pl 43m823 (0.30 Reserve) Des As Pt 14, Pl 43r20654; Blk 4, Pl 43m823 (0.30 Reserve); Blk 17, Pl 43m823 (0.30 Reserve), Save And Except Pts 16, 17, 18, Pl 43r35938; Blk 6, Pl 43m823 Subject To An Easement As In Vs162364 Subject To An Easement Over Pts 9, 10, 11, 12, 13, 14, 15, 24, 26, 27, 28, 29, 30, 31, 32, 33, Pl 43r15700 And Pts 10, 11, 12, 13, 14, 15, 16, 17, Pl 43r16222, Save And Except Pts 1, 2, 3, 9, 10, 11, Pl 43r35938 In Favour Of Mississauga Hydro-electric Commission And Bell Canada As In Lt955564 Subject To An Easement Over Pt 1, Pl 43r16217, Pt 1, Pl 43r16218, Pt 1, Pl 43r16219 In Favour Of The Corporation Of The City Of Mississauga As In Lt955566 Subject To An Easement Over Pts 4, 5, 8, 9, Pl 43r16222, Save And Except Pts 1, 9, Pl 43r35938 In Favour Of The Corporation Of The City Of Mississauga As In Lt955568 Subject To An Easement Over Pts 1, 2, 3, 4, 7, 8, 11, 13, 43r20654 And Pts 5, 6, 7, 10, Pl 43r20653 In Favour Of Mississauga Hydro- electric Commission As In Lt1741001 Subject To An Easement Over Pts 1, 3, 4, Pl 43r20654 In Favour Of The Regional Municipality Of Peel As In Lt1741002 Subject To An Easement Over Pts 2, 3, 7, Lpl 43r16222 And Pts 16, 17, 18, 21, 22, 34, 35, 38, Pl 43r15700, Save And Except Pts 1, 4, 5, 6, 7, 8, 9, 12, 13, 14, 15, pl 43r35938 In Favour Of The Regional Municipality Of Peel As In

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	Of Peel As In Lt1007104 Subject To An Easement Over Pt 5, Pl 43r16736 In Favour Of Mississauga Hydro-electric Commission As In Lt1130471 Together With An Easement As In Ro551159 Subject To An Easement In Gross Over Parts 1,2,3,4 And 5, 43r39842 As In Pr3811615 City Of Mississauga
Property Identification Number (PIN)	Part of PIN 1351 – 20035 (LT)
Approximate Universal Transverse Mercator (UTM) coordinates	NAD83 17T 603714.75 m E m 4823575.93 m N Google Earth
Accuracy Estimate of UTM	10-15 m
Measurement Method	Google Earth
Site Area	5.01 hectares (12.4 acres)
Property Owners, Owner Contact and Address	Mr. Barry Stern EMTC Holdings Inc. c/o The Muzzo Group of Companies 50 Confederation Parkway Concord, ON, L4K 4T8

3.2 Background

Based on a review of historical aerial photographs, chain of title information, historical maps, and other records, the Site was first developed circa 1990s. The Site was developed with two (2) Site buildings – A & B and a parking lot, inferred to be used for commercial use. At the time of the Site visit, the Site buildings were developed with slab-on grade single-storey buildings. Site Building A has a footprint of approximately 4,357 m² (46,899 ft²) and Site Building B has a footprint of approximately 1,621 m² (17,447 ft²). It is to be noted that only tenant - Home sense located in Site Building A has a mezzanine in the northern part of the building that is used for office space and employees lounge / lunch area. The mezzanine is equipped with basic home appliances such as refrigerator, microwave and coffee makers. The northeastern portion of the Site is asphalt-paved with a narrow-landscaped area located on the Site boundaries. The southwestern portion of the Site is landscaped with grass area.

3.3 Geological and Hydrogeological Conditions

The following physiographic, geological and soil maps were reviewed:

- "Toporama"; Natural Resources Canada. Map 030M11. Scale 1:15,000. 2008.
- Quaternary Geology of Ontario geology_ll.shp [computer file], Ontario: Ontario Geological Survey, 2000.
- Bedrock Geology of Ontario geology_ll.shp [computer file], Ontario: Ontario Geological Survey, 2000.
- Bedrock Topography and Overburden Thickness Mapping, Southern Ontario, prepared by Ontario Geological Survey, published 2006

Based on the review of the above maps, the following information was obtained:

• There is a decrease in elevation towards the north and east in the Phase I Study Area. A permanent stream is located approximately 150 m east of the Site, and it flows in a southeasterly direction towards Millet Creek draining in Lake

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Ontario. Based on a review of the topographic map, the inferred groundwater flow at the Site and surrounding properties is anticipated to flow in an easterly direction.

- The Site is approximately 177 m above sea level and mostly flat.
- The Site and surrounding areas are dominated by Till Moraines deposits that consist predominantly of clay to silttextured till (derived from glaciolacustrine deposits or shale).
- The bedrock in the general area of the Site is part of a group belonging to the Queenston Formation, primarily consisting of shale, limestone, dolostone and siltstone.
- Based on a review of "Bedrock Topography and Overburden Thickness Mapping, Southern Ontario, prepared by Ontario Geological Survey, published 2006," the bedrock in the vicinity of the Site is anticipated to be encountered at a depth of approximately 11 metres below ground surface (mbgs).

3.4 Previous Investigations

The following reports were available for review at the time of this Phase One ESA.

- A report entitled *"Phase I Environmental Site Assessment, 5100 Erin Mills Parkway & 2690 Erin Centre Boulevard, Mississauga, Ontario"* dated April 28, 2014, was prepared for OPB (EMTC) Inc. by its agent and manager 20 Vic Management Inc. by Pinchin Environmental. Pertinent information from the report is as follows:
 - The subject property consisted of a multi-tenant commercial building at 5100 Erin Mills Parkway, and a one -storey multi-tenant commercial building at 2690 Erin Centre Boulevard.
 - Three (3) generator rooms with AST containing diesel fuel were identified at 5100 Erin Mills Parkway. Please note these ASTs are located off-Site.
 - 5100 Erin mills Parkway was constructed in 1989, and 2690 Erin Centre Boulevard in 1993.
 - The tenant, London Cleaners, located on-Site, was reportedly a dry-cleaning drop-off depot.
 - The property at 2670 Erin Centre Boulevard was listed as being occupied by a dry cleaner, however according to Site Representative it was a dry-cleaning depot.
 - Based on the previous reports, the Site had a geotechnical investigation which noted the stratigraphy as clayey silt fill, compact to dense till to approximately 1.7 to 5.5 m bgs, followed by red shale bedrock with limestone interbeds. Groundwater ranged between 3.4 to 4.2 m bgs.
 - Based on the findings of the Phase I ESA, no potential environmental concerns were identified.
- A report entitled "Phase I Environmental Site Assessment Update, Erin Mills Shopping Centre, Mississauga, Ontario" dated November 5, 2018, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Golder. Pertinent information from the report is as follows:
 - The subject property was noted to consist of the municipal address 5100 Erin Mills Parkway.
 - Based on the ERIS report, Loblaws Properties Ltd. At the Pumps, located approximately 30 m southwest of the Site at 5010 Glen Erin Drive, was listed as a gasoline service station with two (2) liquid fuel tanks, with expiration November 11, 2010. Please note these ASTs are located off-Site.



- Based on the findings, no issues of potential environmental concern were identified for the Site.
- A report entitled "Waste Audit Report, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, ON, L5M 4Z5", by Green for Life Environmental (GFL) in January 2019. No pertinent information related to an environmental concern for the Site was identified.
- A report entitled "Investigation of Indoor Air Quality, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 26, 2019, was prepared for Cushman & Wakefield Ltd. by Pinchin. Pertinent information from the report is as follows:
 - The survey addressed all common areas in the shopping mall.
 - Elevated dust measurements were recorded in several areas of the building.
 - Total Volatile Organic Compounds (TVOC) measurements were slightly elevated on the lower level in the vicinity of a fragrance shop and an eyeglass clinic.
- A report entitled "Asbestos Reassessment, Erin Mills Town Centre, 5100 Erin Mills Parkway, Mississauga, Ontario", dated February 28, 2019, was prepared for Cushman & Wakefield Asset Services by Pinchin. Pertinent information from the report is as follows:
 - No samples were collected as part of the assessment. The purpose of the report was to assess changes in the condition and quantity of previously reported asbestos-containing materials.
 - Asbestos cement (transite) rainwater leaders are present in various locations in the building, in good condition.
- A report entitled "Water Sample Collection and Analysis for Lead, Erin Mills Town Centre", dated March 4, 2019, was prepared for Cushman & Wakefield by Ontario Environmental & Safety Network Ltd. No pertinent information related to an environmental concern for the Site was identified.
- A report entitled "Test Pit Program, Erin Mills Town Centre (Former Sears), 5100 Erin Mills Parkway, Mississauga, Ontario", dated March 6, 2020, was prepared for OPB (EMTC) Inc. c/o Cushman & Wakefield Asset Services ULC by Pinchin. Pertinent information from the report is as follows:
 - Sixteen (16) bulk soil samples were submitted for PHCs, VOCs, PAH, and metal and inorganics to a maximum depth of 4.27 m bgs using an excavator.
 - The soil samples submitted met Table 3 SCS with the exception of Electric Conductivity (EC) and Sodium Adsorption Ratio (SAR) exceedance. These exceedances can be discounted based on the recent exemption that was passed by the MECP associated with areas that have been subject to the application of de-icing products to walkways and driveways (eg. Road salt).
- A report entitled *"Phase I Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).



- The Phase I ESA was prepared for Muzzo Group with the objective to identify issues of environmental concern to the Site in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
- Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building
 addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up
 generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good
 condition. Floor drains were noted to be in close proximity to the AST's. Please note these ASTs are located offSite.
- Based on the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants identified from the ERIS report and the potential fill materials used during development on-Site were considered a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for groundwater at the Site.
- Based on the findings of the Phase I ESA, a Phase II was recommended.
- A report entitled *"Phase II Environmental Site Assessment, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated December 1, 2020, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was prepared for Muzzo Group to support identified on-Site concerns of waste generation, and off-Site concerns of a nearby gas station and hydro one facility. Based on recommendations made in the Phase I ESA (EXP, 2020).
 - Between March 2 and 6, 2020, nine (9) boreholes were advanced at the Site (BH1, BH2, BH5, BH6, BH9, BH11, , BH13, BH17 and BH20). Five (5) of the boreholes were equipped with monitoring wells (BH1, BH5, BH9, BH17 and BH20).
 - Soil samples were collected and submitted for the analysis of one or more of the following: Petroleum Hydrocarbons (PHCs), Volatile Organic Compounds (VOCs), Metals and Inorganics (M&I), Polycyclic Aromatic Hydrocarbons (PAHs), Polychlorinated Biphenyls (PCBs), pH, and grain-size (75-micron sleeve).
 - Groundwater samples were collected from one (1) monitoring well (MW17) and analyzed for PHC, Metals and VOCs. It was noted that the collection of groundwater from MW1, MW5, MW9 and MW20 were attempted on two (2) separate occasions, however, groundwater was not encountered at these locations.
 - Groundwater flow direction could not be determined as three (3) water levels screened within the sample geological formation are required to determine groundwater flow at the Site. Based on local topography, groundwater was anticipated to flow in a northeast direction towards a tributary of the Credit River.
 - The concentrations of all parameters submitted for the soil samples from the boreholes were either below the applicable MECP (2011) Table 7 Site Condition Standards (SCS) for residential/parkland/institutional property with medium to fine soil, or not detected above the laboratory detection limits, with the exception of the following:

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- Electrical Conductivity (EC) and Sodium Adsorption Ratio (SAR) in BH5 SS2 (0.8 1.5 m bgs), BH9 SS2 (0.8 1.5 m bgs), BH11 SS2 (0.8 1.5 m bgs), and BH13 SS2 (0.8 1.5 m bgs). It was inferred the high concentrations were due to de-salting activities for road safety or for pedestrian walkways. As such, the elevated EC/SAR in soil was not considered an exceedance based on section 49.1 of O.Reg. 407/19.
- The concentrations of all parameters submitted for groundwater for one (1) monitoring well were either below the applicable MECP (2022) Table 7 SCS or not detected above laboratory detection limits.
- Based on the results of the Phase II ESA (EXP, 2020), no further work was recommended at the time. It was noted that, should an RSC be required for the Site or smaller portions of the Site, additional environmental work would be required to support those submissions.
- A report entitled *"Phase I Environmental Site Assessment Update, 5100 Erin Mills Parkway, Mississauga, Ontario"* dated October 18, 2022, was prepared for Muzzo Group by EXP. Pertinent information from the report is as follows:
 - The subject property included a multi-tenant commercial shopping plaza at 5100 Erin Mills Parkway, and surrounding buildings with the following municipal addresses: 2670 & 2690 Erin Centre Boulevard, 5175, 5155, 5145, 5035 & 5025 Glen Erin Drive, and 2635, 2655 & 2675 Eglinton Avenue West.
 - The report was completed in general accordance with CSA standards Z768-01 (R2016).
 - The Phase I ESA Update was prepared for Muzzo Group for due diligence purposes in support of a potential acquisition involving the Site. It was noted that an RSC was not required at the time.
 - Six (6) diesel above ground storage tanks (ASTs) were observed in the mechanical rooms of the Site building
 addressed as 5100 Erin Mills Parkway. The AST's were noted to be 1,135 L and associated with the back-up
 generators for the shopping centre. Secondary containment was noted, and concrete appeared to be in good
 condition. Floor drains were noted to be in close proximity to the AST's.
 - According to information provided by the Site Representative and based on a reviewed Environmental Control Incident Report provided by the MECP, the spill occurred on November 27, 2021, during the construction activities in the corridor of mall entrance F in the west portion of 5100 Erin Mills Parkway, a diesel-filled construction heater was temporarily stationed on the concrete walkway adjacent to the building.
 - EXP was made aware that since the previous Phase I ESA, there was a fire On-Site Fire at 2575 Erin Mills Parkway in the building in 2021.
 - Similar to the Phase I ESA (EXP, 2020) findings, the multiple registered waste generators from commercial tenants identified from the ERIS report and the potential fill materials used during development on-Site were considered a potential concern to groundwater and soil at the Site. In addition, the presence of a hydro building west adjacent to the Site and a gasoline station within close proximity to the Site were considered off-Site concerns for groundwater at the Site.
 - Based on the Phase I ESA Update (EXP, 2022) findings, the fire and spill are considered a low concern and immediate action is not required at this time. However, it was noted that this will need to be identified as an Area of Potential Environmental Concern (APEC) in any future O. Reg 153/04 compliant Phase One ESA and investigated through a Phase Two ESA both of which will be required for the purpose of filing an RSC.

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• It is noted that based on the nature and distance from the spill and the fire to the Site, neither we considered to contribute to an APEC.

3.5 Underground Utilities

The Site utilities and services were identified at the Site based on information provided in environmental records, relevant utility infrastructure observed during the Site reconnaissance. The Site utilities are summarized in the table below and noted on Figure 3, where available. It is noted that the precise underground location of the utilities cannot be determined without professional locate services.

Utility	Source	Location	Site Entry
Natural Gas	Enbridge Gas	Underground	The natural gas meter was observed on the eastern wall of the Site building A and northern wall of Site Building B.
Sanitary Sewer	City of Mississauga	Underground	Unknown
Storm Sewer	City of Mississauga	Underground	Various storm sewers observed in the asphalt portion of the Site
Water	City of Mississauga	Underground	Unknown
Electricity	Unknown	Underground	Unknown
Telecommunications	Unknown	Underground	Unknown

3.6 Potentially Contaminating Activities and Areas of Potential Environmental Concern:

Refer to Table 2 for the list of potentially contaminating activities (PCAs) that have occurred within the Phase One Study Area, which includes the Site and properties within 250 m radius of the Site boundaries. The approximate locations of the PCAs are shown on Figure 2. The following PCAs were determined to contribute towards and APEC on-Site:

PCA Identifier	Address	Location of Activity (in relation to Site) ⁽¹⁾	Potentially Contaminating Activity (PCA) ⁽²⁾	Approximate timeline that PCA occurred	Contributes to APEC (Yes or No)?
On- Site					
1a	2 670 and		PCA 'Other' – Waste Generation	Based on the ERIS report, the property was registered as generating photo processing wastes between 1992 – 2001.	No, based on the previous report Phase I ESA (Pinchin, 2014) - according
1b	2670 and 2690 Erin Center Boulevard	Entire Site	PCA#40 – Pesticides (including Herbicides, Fungicides and Anti- Fouling Agents) Manufacturing, Processing, Bulk Storage and Large-Scale Applications	Based on the ERIS report, the property was registered as was registered as a limited vendor for pesticides.	to Site representative the dry-cleaner was used as a drop off depot. The review of the current ERIS showed a lack of waste generation reports at the Site.

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PCA Identifier	Address	Location of Activity (in relation to Site) ⁽¹⁾	Potentially Contaminating Activity (PCA) ⁽²⁾	Approximate timeline that PCA occurred	Contributes to APEC (Yes or No)?
1c			PCA#37- Operation of Dry-Cleaning Equipment (where chemicals are used)	Based on the reviewed City Directories and previous reports, the property was occupied by Dry Cleaners – The Loblaws Supermarkets Limited between 2000 - 2001.	
1d			PCA#30-Importation of Fill Material of Unknown Quality.	Based on Aerials, between 1989 and 1997, Site Building A, Site Building B, a paved parking lot (northeastern and central), and landscaping (southern) were developed. It's possible that unknown- quality fill materials were brought in for grading during construction.	APEC - 1
1e		Central and north eastern portion of the Site - Parking lot	PCA#48- Salt Manufacturing, Processing and Bulk Storage	During the Site visit, bulk salt storage and snow removal equipment were seen in the northeastern parking lot. The Parking areas in the north and central portion of the Site have utilized de-icing measures for public safety	APEC - 2
1f		North portion of the Site	PCA#55-Transformer Manufacturing, Processing and Use.	During the Site visit, (2) pad-mounted transformers were observed at the north portion of the Site along the Site boundary.	APEC - 3
Off-Site					
2	5100 Erin Mills	East adjacent	PCA#28 – Gasoline and Associated Products Storage in Fixed Tanks	Based on the ERIS report, The property was listed as "digital printing", "manufacturer of medical	No based on the trans- gradient location and distance to the buildings associated with the 5100
	Parkway		PCA#19 – Electronic and Computer Equipment Manufacturing	equipment and supplies manufacturing", "manufacturer of coating,	Erin Mills Parkway site as well as the previous reports and nature of the



PCA Identifier	Address	Location of Activity (in relation to Site) ⁽¹⁾	Potentially Contaminating Activity (PCA) ⁽²⁾ Approximate timeline that PCA occurred		Contributes to APEC (Yes or No)?	
			PCA#31 – Ink Manufacturing, Processing and Bulk Storage	engraving, heat treating and allied activities". The property generated organic/inorganic	operations (shopping mall).	
		PCA#33 – Metal Treatment, Coating, Plating and Finishing PCA#40 – Pesticides (including Herbicides, Fungicides and Anti- Fouling Agents) Manufacturing, Processing, Bulk Storage and Large-Scale Applications		waste compressed gases, detergents/soaps,		
			PCA 'Other' – Waste Generation			
3	Glen Erin Drive & Erin Centre Blvd	West adjacent	PCA 'Other' – Spill	Based on the ERIS report, the intersection was listed for a spill of approximately 125 L of herbicides to the road.	No, considering that it was washed off into the catch basins on the road and the low mobility of the contaminants of concern (herbicides).	
4	2800 Erin Centre Boulevard	30 m west – across Glen Erin Drive	PCA 'Other' – Waste Generation	Based on the ERIS report, the property generated halogenated solvents, alkaline wastes, acid waste, inorganic laboratory chemicals, aliphatic solvents, petroleum distillates from 2002 to 2013	No, given the nature of operations of a library.	
5	2665 Erin Centre Boulevard	30 m west – across Glen Erin Drive	PCA 'Other' - Waste Generation	Based on the ERIS report, the property generated inorganic laboratory chemicals, petroleum distillates, oils skimmings & sludges, waste oils & lubricants, organic laboratory chemicals and photoprocessing wastes	No, based on the nature of the operations (a secondary school) and the ERIS report which notes that there were no environmental impacts, no sub surface contamination, and no	

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PCA Identifier	Address	Location of Activity (in relation to Site) ⁽¹⁾	Potentially Contaminating Activity (PCA) ⁽²⁾	Approximate timeline that PCA occurred	Contributes to APEC (Yes or No)?	
				from 1992 to October 2022	contamination migration associated with the spill.	
			PCA 'Other' – Spill	Based on the ERIS report, the property was listed for fuel oil spill of liquid petroleum (quantity unknown) in 2009.		
6	5010 Glen Erin Drive	30 m southwest – across Glen Erin Drive	PCA#28 – Gasoline and Associated Products Storage in Fixed Tanks	Based on the ERIS report, the property was listed for two (2) expired liquid fuel tank of 40,000 L regular fuel and 20,000L diesel fuel and associated piping in 2012 and as an expired facility in 2013.		
			PCA#58 – Waste Disposal and Waste Management, including thermal treatment, landfilling and transfer of waste, other than use of biosoils as soil conditioners	Based on the ERIS report, the property generated halogenated pesticides, organic and inorganic laboratory chemicals, paint/pigment/coating residues, detergents/soaps, misc. waste organic chemicals waste oils and lubricants, aliphatic solvents and acid wastes – heavy metals in 2015 to October 2022.	No, based on its location downgradient from the Site. Furthermore, it is noted that the gas station on the 5010 Erin Mills Drive Site is located at the far south portion of the property which is approximately 250 m	
			PCA#40 – Pesticides (including Herbicides, Fungicides and Anti- Fouling Agents) Manufacturing, Processing, Bulk Storage and Large-Scale Applications	Based on the ERIS report, the property was registered as was registered as a limited vendor for pesticides.	south of the Site.	
			PCA 'Other' – Spill	Based on the reviewed ERIS report, the property reported 60 L of transformer non-PCB oil spill onto the grass from a		



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PCA Identifier	Address	Location of Activity (in relation to Site) ⁽¹⁾	Potentially Contaminating Activity (PCA) ⁽²⁾ Approximate timeline that PCA occurred		Contributes to APEC (Yes or No)?
				pad-mounted transformer in 2020 to MOE.	
			PCA#37- Operation of Dry-Cleaning Equipment (where chemicals are used)	Based on the reviewed City Directories, the property was listed as drycleaner in 2021.	
7	5045 Glen Erin Drive	138 m south	PCA#55-Transformer Manufacturing, Processing and Use.	Based on the ERIS report, the property is registered as standby power system.	No, given the inferred hydraulically down- gradient location relative to the Site and the low mobility of the contaminants of concern (PCBs).
8	Major Reinvention, Mississauga, ON L5M 7E1, Canada [43.557201, -79.712680]	163 m east	PCA#28 – Gasoline and Associated Products Storage in Fixed Tanks	Based on the ERIS report, approximately 600L of diesel fuel was spilled and reported to MOE due to equipment failure / malfunction during some other heavy and civil engineering construction. Soil contamination was considered possible.	No, given the spill was considered trans gradient and at a considerable distance to the Site.

(1) These are approximate distances taken from the Site boundaries

(2) Potentially contaminating activity means a use or activity set out in Column A of Table 2 of Schedule D (O.Reg 153/04, as amended) that is occurring or has occurred in a phase one Study area.

3.6 Areas of Potential Environmental Concern

Based on the evaluation of the PCAs located within the Phase One Study Area, the following areas of potential environmental concern (APECs) were identified at the RSC Property, as presented in Figure 4.

Table 3: Areas of Potential Environmental Concern

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Area of Potential Environmental Concern (APEC)	Location of APEC on Phase One Property	Potentially Contaminating Activity (PCA) ¹	Location of PCA (on-Site or off- Site)	Contaminants of Potential Concern	Media Potentially Impacted (Groundwater, soil and/or sediment)
APEC 1: Entire Site (PCA- 1d)	Entire Site	PCA#30-Importation of Fill Material of Unknown Quality.	On-Site	PHCs, VOCs, PAHs, Metals, Inorganics	Soil
APEC 2: Seasonal application of de-icing salt agents (PCA-1e)	Central and northeastern portion of the Site - Parking lot	PCA#48- Salt Manufacturing, Processing and Bulk Storage	On-Site	рН, EC, SAR	Soil
APEC 3: Transformers on Site (PCA-1f)	North portion of the Site	PCA#55-Transformer Manufacturing, Processing and Use.	On-Ste	PCBs	Soil

(1) Potentially contaminating activity means a use or activity set out in Column A of Table 2 of Schedule D (O.Reg.153/04, as amended) that is occurring or has occurred in a phase one Study area.

The seasonal salt storage area located on the Site that is used solely for the purpose of de-icing activities in the winter months. Paved areas where the salt has been applied are also present at the Site. Although seasonal salt storage and salt application has occurred on the Site, it is the QP's opinion that the seasonal on-site salt storage is not a PCA as there is no salt manufacturing, processing and/or bulk storage taking place on-site. Further, as the salt is seasonally stored and used solely for the purpose of application to the paved surfaces for the safety of vehicular or pedestrian traffic under conditions of snow or ice or both, it is the QP's opinion that the exemption outlined in O.Reg153/04, Part IX, Subsection 49.1, regarding the classification of Electrical Conductivity (EC) and Sodium Adsorption Ratio (SAR) as COCs, applies.

Based on the findings of the Phase One ESA and conclusions, a Phase Two ESA was required to assess the soil conditions at the Site prior to submitting an RSC.

4. Physical Site Description

4.1 Stratigraphy

The current soil investigation conducted at the Site consisted of the advancement of fourteen (14) boreholes into the surficial fill material and the underlying native materials to a maximum depth of 15.9 m bgs. Boundaries of soil indicated on the log sheets are intended to reflect transition zones for the purpose of environmental assessment and should not be interpreted as exact planes of geological change.

The general stratigraphy at the Site, as observed in the boreholes, consists of topsoil or asphalt underlain by sand and gravel. Fill material was observed beneath the topsoil and sand and gravel. Fill material generally consisted of clayey silt and/or silty sand which overlies native soil. Native material at the Site generally consists of silt, sandy silt, and/or silt till. Shale bedrock of the Queenston Formation was encountered below the silt till at all borehole locations. The approximate elevation for the bedrock encountered in each borehole is presented in the individual borehole and core logs. A brief description of the soil stratigraphy

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at the Site, in order of depth, is summarized in the following sections. Refer to borehole logs provided in Appendix C for details of soil stratigraphy. The interpreted Site geology is also shown on the enclosed cross section shown on Figure 13.

4.1.1 Surface Material

Asphalt with thickness ranging from about 70 to 140 mm was encountered at the surface of Boreholes 201, 202, 203, 204, 205, 206 and 209D.

At the ground surface of Boreholes 207, 210, 211, 212, 213 and 214D, topsoil cover with thickness ranging from about 100 to 200 mm was encountered.

At Borehole 208, a topsoil layer of about 125 mm thick was encountered below the surficial pebble layer.

4.1.2 Fill Material

Fill, comprising sand and gravel, silty sand and clayey silt was encountered below the asphalt or topsoil at all borehole locations. The fill extends to depths ranging from about 0.7 to 2 m below existing ground surface (El. ~176.1 to 173.3 m).

4.1.3 Native Soils

The fill in Borehole 205 is underlain by a silt deposit which is brown to grey in colour, contains a trace of clay and gravel. It is in a dense to very dense state of compactness (recorded 'N'-values of 41 to over 100) and extends to a depth of about 8.5 m below existing grade (El. ~166.8 m).

A sandy silt to silty sand deposit was encountered below the fill in Borehole 206. This deposit is brown in colour and is in a compact state of compactness. The sandy silt to silty sand extends to a depth of about 2.5 m below existing ground surface (El. \sim 173.5 m).

A silt till deposit was encountered below the silt in Borehole 205, below the sandy silt to silty sand in Borehole 206 and below the fill in the remaining boreholes. The silt till is generally reddish brown in colour, contains some clay, a trace of sand with a trace of shale fragments at lower level of the deposit. It has moisture contents of about 5 to 11 percent of dry mass and is in a dense to very dense state of compactness (recorded 'N'-values of 38 to over 100). The silt till extends to depths of about 2.6 to 10.1 m below existing ground surface (El. ~174.2 to 165.2 m).

4.1.4 Bedrock

Shale bedrock with some interbedded sandstone and limestone layers were encountered at all the borehole locations. The top of the shale bedrock was encountered between 2.6 and 10.1 mbgs (El. ~174.2 to 165.2 m) across the boreholes 201 to 214.

4.2 Areas on, in or Under the Phase Two Property Where Excess Soil is Finally Placed

Based on the reviewed information, it appears that fill material may have been brought to the site at the time of the previous development for grading purposes. No fill material was brought to the Site during the Phase Two ESA.

4.3 Land Use

It is EXP's understanding that the Site will be redeveloped with nine (9) residential towers of 20 to 44 storeys. The development will include three (3) levels of underground car parking.

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5 Contaminants of Concern

For assessment purposes, EXP selected the MECP (2011) Table 3: Full Depth Generic Site Condition Standards (SDS) in a Non-Potable Ground Water Condition for Residential/Parkland/Institutional Property Use and medium-fine textured soil conditions was considered applicable for determining contaminants of concern (COCs), based on the rationale presented in the following Table.

Table 4: Site Condition Standards

Description	Site Specific Condition
Section 35 Site Sensitivity	 Applicable The full depth generic site condition standards in a non-potable groundwater condition. The Site is serviced by the City of Mississauga water distribution system which obtains its municipal water supply from Lake Ontario. The Site is not proposed for agricultural or other property use for filing RSC. The Site was not located within wellhead protection area.
Section 41 Site Sensitivity	 Not applicable The soil at the Site has pH values between 5 and 11 for subsurface soil and between 5 and 9 for surface soils soil. The Site is not located within a Significant Area, and/or located adjacent to an area of natural significance/an environmentally sensitive area.
Section 43.1 Site Sensitivity	 Not applicable The Site is not considered a shallow soil property, based on the recovered soil cores, which indicated that more than two-thirds of the Site has an overburden thickness in excess of 2 m. The Site is note located within 30m of a waterbody
Ground Water	 Potable The Site is serviced by the City of Vaughan water distribution system which obtains its municipal water supply from Lake Ontario. It is noted however that there are potable wells located within a 250 m radius of the Site and as such, the Site is considered to be in a potable groundwater condition.
Land Use	CommercialThe proposed future use of the Site is for residential use.
Soil Texture	 Medium-fine textured The predominant texture of soils at the Site is considered to be medium-fine textured, based on soil characteristics identified in the borehole logs and 75-micron sieve analysis.



5.1 Soil and Groundwater Characterization

A chemical constituent was selected as a COC if it was detected in soil samples obtained from the RSC Property at a concentration in excess of the applicable Table 3 SCS.

Soil samples were submitted for the analysis PHCs, BTEX, VOCs, metals and inorganics, PAHs, and PCBs. All soil parameters were either non-detected or detected below their applicable MECP (2011) Table 3 SCS. Therefore, no COCs were identified in soil.

Analytical results of soil samples collected on the RSC Property are presented in a plan view on Figures 7 through 12.

Groundwater was not identified as a media potentially impacted of the APECs at the Site; therefore, no groundwater investigation was completed.

Given that no COCs were identified on the RSC Property during the Phase Two ESA, no additional delineation work, remediation, and/or risk assessment was considered necessary prior to the filing of an RSC for the RSC Property.

5.2 Contaminant Fate and Transport

Subsurface investigations were completed to assess the impact of the PCAs on soil and ground water within the APECs identified on the RSC Property. The screening of contaminants of concern (COCs) was done by comparing the maximum concentrations of potential contaminants of concern (pCOCs) in soil and ground water with the Table 8 Standards. A summary of the assessment of each APEC is provided in Table 5.

Table 5: Site Condition Standards

APEC	Location of APEC on RSC Property	РСА	Location of PCA	pCOC and Media Potentially Affected	Current Status (Exceedances of SCS)
APEC 1: Entire Site (PCA-1d)	Entire Site	PCA#30-Importation of Fill Material of Unknown Quality.	On-Site	PHCs, VOCs, PAHs, Metals, Inorganics	Six samples were collected for PHCs, VOCs, PAHs, Metals, Inorganics. All Samples meet the Table 3 SCS.
APEC 2: Seasonal application of de-icing salt agents (PCA- 1e)	Central and northeastern portion of the Site - Parking lot	PCA#48- Salt Manufacturing, Processing and Bulk Storage	On-Site	pH, EC, SAR	Six samples collected for ph, EC, SAR. All Samples meet the Table 3 SCS.*
APEC 3: Transformers on Site (PCA- 1f)	North portion of the Site	PCA#55-Transformer Manufacturing, Processing and Use.	On-Ste	PCBs	One (1) soil sample was collected at the location of the transformer and PCBs were not



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* It is anticipated that road salt has been applied across the paved areas of the Site, for the purposes of vehicular and pedestrian safety under conditions of snow and/or ice. As such, it is the QPESA's opinion that the applicable Table 3 SCS for EC and SAR in soil were exceeded solely because salt was used on the road for the purpose of keeping the road safe for traffic under conditions of snow or ice or both, and therefore these parameters are not considered to exceed as per Section 49.1 (1) of O. Reg. 153/04.

5.3 Sediment

Sediment sampling was not considered as part of the Phase Two ESA scope as sediment was not identified as a media of concern in the Phase One ESA. Furthermore, no water bodies are located on-Site.

5.4 Areas Where Contaminants are Present

No soil exceedances of the MECP Table 3 Standards are present at the RSC Property. Given that no contaminants have been identified at the RSC property, a cross-section has only been provided for the purposes of presenting the stratigraphy at the Site.

5.6 Climatic and Meteorological Conditions Affecting Migration

It is noted that climatic or meteorological conditions may influence the distribution and migration of COCs at the Site. Seasonal fluctuations in groundwater due to cyclical increases and decreases in precipitation can affect groundwater recharge. Groundwater levels may be elevated in the spring and fall due to snow melt and/or increases in precipitation; and groundwater levels may be lowered in the winter and summer due to snow storage and/or increased evaporation. Such fluctuations can increase the vertical distribution of COCs in the capillary zone, as well as alter the direction of groundwater flow paths based on changes in infiltration rates. However, based on the conditions observed at the Site, it is not anticipated that the climatic or meteorological changes will result in significant alterations in the distribution of contaminants.

Given that no soil exceedances of the MECP Table 3 Standards were identified at the RSC Property, the distribution and migration of temporal variability in ground water flow due to climatic and meteorological conditions is not a concern for the RSC Property.

6 Exposure Pathways

6.1 Human Health Receptors and Exposure Pathways

The selection of human receptors is based on the future residential use of the RSC Property. Therefore, the receptors chosen for analysis are those standard receptors found at parkland properties and includes property visitors (all ages), property trespassers (child and adult). Subsurface workers (e.g., construction/utility workers) may also be present during redevelopment of the RSC Property and, as such, are also considered.

Given that no COCs were identified on the Site for the purposes of filing a RSC, there are no potential soil gas/indoor/outdoor air exposure pathways for construction workers, long-term (outdoor) workers and property visitors. However, the potential onsite exposure pathways for the human site receptors at the RSC Property include:



- Construction worker:
 - Inadvertent soil ingestion;
 - Soil dermal contact;
 - Soil particulate inhalation;
 - Outdoor air inhalation (trench scenario).
- Property visitor:
 - Soil particulate inhalation;
 - Soil dermal contact;
 - Inadvertent soil ingestion;
 - Indoor air inhalation;
 - Outdoor air inhalation.
- Outdoor long-term (outdoor) worker:
 - Soil particulate inhalation;
 - Soil dermal contact;
 - Inadvertent soil ingest; and,
 - Outdoor air inhalation.

6.2 Ecological Receptors and Exposure Pathways

The RSC Property is proposed to be developed as parkland capable of supporting some terrestrial ecological receptors. Relevant terrestrial receptors include terrestrial vegetation, such as trees, grasses and weeds; soil invertebrates, such as earthworms, millipedes and beetles; terrestrial birds, such as pigeons, sparrows and robins; and, terrestrial mammals, such as moles, voles and mice.

Furthermore, it is noted that the MECP evaluates exposure to aquatic receptors at properties within 5 km of a surface water body. Given that no COCs were identified on the Site for the purposes of filing a RSC, there are no relevant COCs for ecological health. However, the potential on- site exposure pathways for the ecological receptors at the East Parcel include:

- Terrestrial Vegetation
 - root uptake of soil; and,
 - stem and foliar uptake of vapour.
- Soil Invertebrates
 - soil dermal contact;
 - ingestion of soil;
 - soil particulate inhalation;
 - inhalation of vapour; and,
 - tissue residue ingestion.
- Terrestrial Mammals and Birds
 - soil dermal contact;



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- ingestion of soil;
- soil particulate inhalation;
- inhalation of vapour; and,
- tissue residue ingestion.
- Aquatic Receptors
 - direct contact with surface water via soil leaching and groundwater discharge.