

Functional Servicing and Stormwater Management Report

Proposed Residential
Development
1225 Dundas Street E,
Mississauga, Ontario



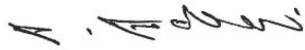
Prepared for:
Dundix Realty Holdings
c/o SmartCentres REIT.

Prepared by:
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Markham ON L3R 0B8

Project No. 160623078

August 7, 2024

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Prepared by _____

(signature)

Payman Fatahi, C.E.T., L.E.L., Project Manager, Community Development



Approved by _____

(signature)

Angelo Ligotti, P. Eng., Senior Principal, Community Development



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1.0 INTRODUCTION

Stantec Consulting Ltd. was retained by Dundix Realty Holdings c/o SmartCentres REIT (the "Client") to provide this Functional Servicing and Stormwater Management Report in support of a proposed residential development in the City of Mississauga, Ontario. The purpose of this report is to provide a servicing opinion regarding the availability of existing municipal infrastructure to support the proposed development on the subject lands.

1.1 SITE LOCATION AND DESCRIPTION

The 1.29 Ha site depicted in the aerial figure below is located at 1225 Dundas Street East in Mississauga, Ontario. The site currently consists of a commercial plaza and associated parking. It is bounded by Dundix Road to the north, a residential property to the east, Dundas Street East to the south, and Arena Road the west.

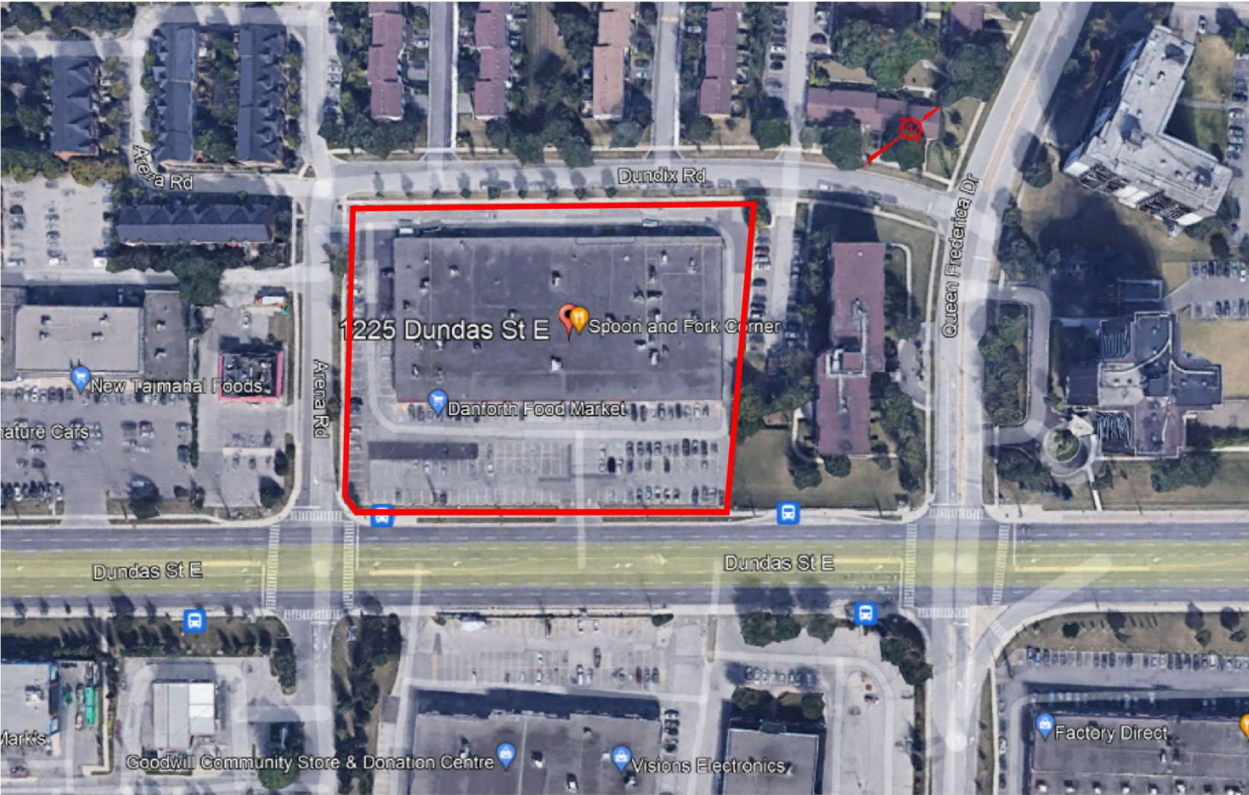


Figure 1-1: Site Location



1.2 SITE PROPOSAL

The site is proposed for conversion to a residential development consisting of 602 apartment and 40 townhouse units with associated amenity and vehicular access areas at grade level. A strip along Dundas Street East will be dedicated to the city as road widening, reducing the site area to 1.24 Ha. Parking will be provided at the underground level which will effectively encompass the entire site footprint below grade. The site concept is depicted below and also provided in **Appendix A**.

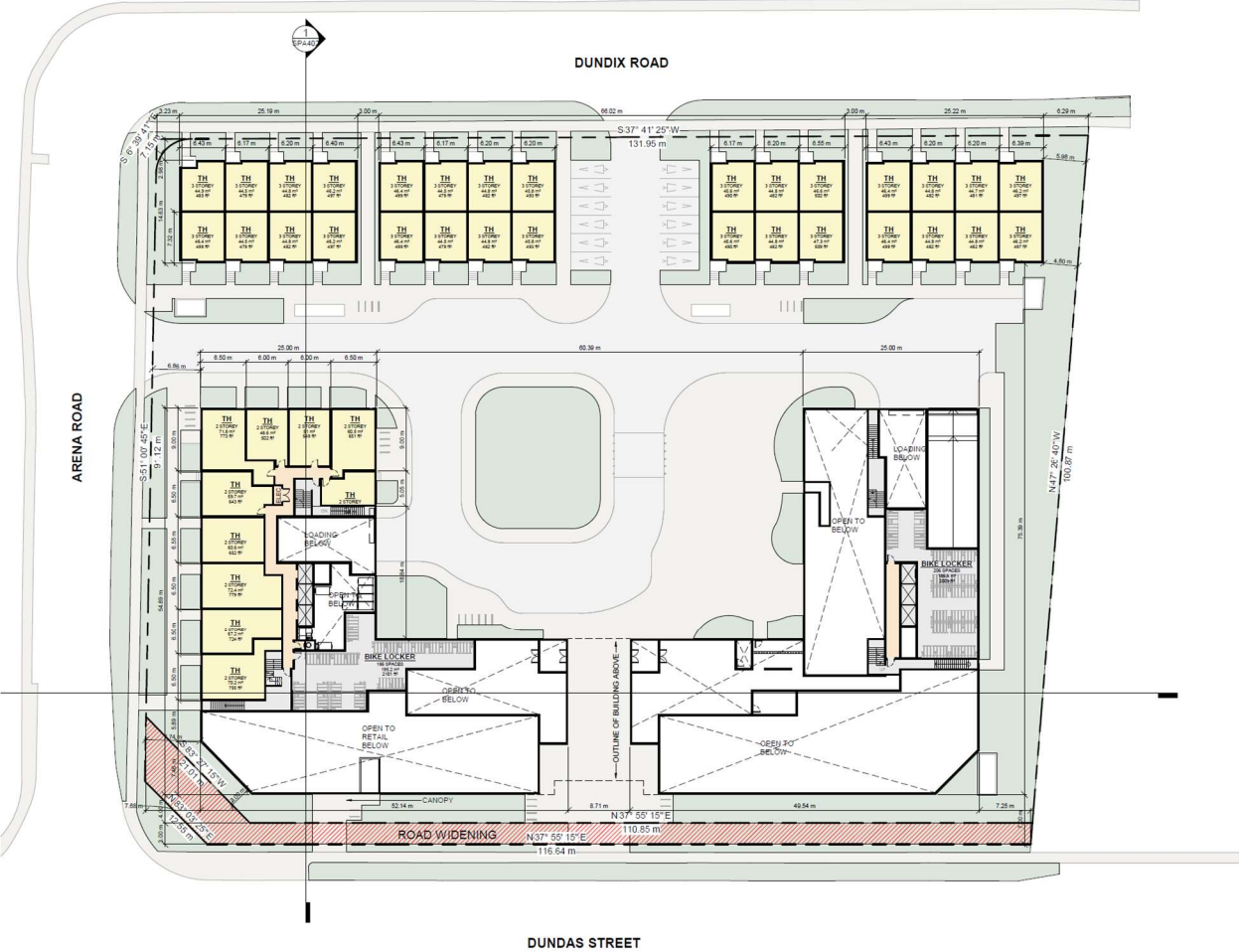


Figure 1-2: Site Concept

Site development statistics are shown on Table 1-1 below and also provided in **Appendix A**.



Table 1-1: Site Development Statistics

Function	Number of Units	Gross Floor Area (m ²)*	Equivalent Population**
Apartments (Unit Size > 750 ft ²)	45	-	135
Apartments (Unit Size =< 750 ft ²)	557	-	892
Townhouses	40	-	136
Commercial	-	626	18
Office	-	-	-
Totals	642	-	1,181
<p>*Gross Floor Area only used in Non-Residential Equivalent Population determination in this report. See site statistics in Appendix A for complete data on Gross Floor Areas.</p> <p>** Equivalent Population based on Region of Peel 2020 Development Charges Background Study: Residential PPU: Townhouse – 3.4, Large Apartment (>750 ft²) – 3.0, Small Apartment (=<750 ft²) – 1.6 Non-Residential: 1.0 employee per 36 m² GFA</p>			

1.3 CRITERIA AND BACKGROUND MATERIAL

The servicing scheme for the site shall be in accordance with guidelines set by the following agencies:

- City of Mississauga
- Region of Peel
- Credit Valley Conservation
- Ontario Provincial Standards
- Ministry of the Environment, Conservation and Parks
- Ministry of Transportation
- Ontario Building Code

The following background reports and materials have been used as reference:

- Region of Peel 2020 Water and Wastewater Master Plan for the Lake-based Systems
- Region of Peel 2020 Development Charges Background Study
- Drainage Area Plans and Plan/Profiles obtained from the city and region
- Draft Hydrogeological Investigation (Project 2202029) by GEI Consultants, June 24, 2022



2.0 STORM DRAINAGE AND STORMWATER MANAGEMENT

2.1 STORM DRAINAGE

A 525 mm to 750 mm storm sewer drains east along Dundix Road. A 600 mm storm sewer drains south along Arena Road and heads west on Dundas Street East. A 750 mm to 825 mm storm sewer drains east along Dundas Street East. There is also a 900 mm storm sewer draining west near the southern boundary of the neighboring property to the east through an easement that terminates at the southeastern corner of the subject site. This sewer is received by a 975 mm storm sewer that crosses Dundas Street East heading south. Topographical information and storm drainage plans obtained from the city indicate the existing site drainage is collected by this 975 mm storm sewer crossing Dundas Street East through a 525 mm site storm connection at the southeast corner. This same outlet will be maintained and used for post-development storm drainage out of the site. The site receives no external flows under existing conditions, and this will remain to be the case under proposed.

As the majority of development will rest on an underground parking level, site drainage will mostly be captured by area drains connected to the building mechanical storm system. A storm sewer will be required along the northern periphery to capture drainage collected by front yard area drains for the townhouse units facing Dundix Road.

The municipality has indicated they prefer permanent groundwater dewatering to be discharged into the municipal storm sewer rather than sanitary. Preliminary investigation by the hydrogeological consultant indicates the site groundwater is not suitable for discharge into the city storm sewer system without pretreatment, and therefore pretreatment measures will be required. These measures will be reviewed and established at Site Plan Application in coordination with the mechanical and hydrogeological consultants. Long-term dewatering peak flow rate is estimated by the hydrogeologist to be 14,100 L/day (See excerpt in **Appendix B**) which translates to 0.2 L/s.

A stormwater storage tank located at the basement level will provide quantity control and retention volumes. A preliminary overview of the storm servicing is provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed site servicing design will be provided at Site Plan Application which will include final tank location and orifice sizing. Background materials on site drainage obtained from the city are provided in **Appendix E**.

2.2 STORMWATER MANAGEMENT

The site is situated within the Applewood Creek watershed under the jurisdiction of Credit Valley Conservation. The following City of Mississauga and Credit Valley Conservation stormwater management criteria are applicable:



- Quantity Control: The 100-year post-development flow shall be controlled to the 2-year predevelopment level. An adjustment factor of 1.25 is applied to the post-development runoff coefficient.
- Runoff Volume Reduction: The first 5mm of runoff shall be retained on-site and managed by way of infiltration, evapotranspiration, or re-use.
- Quality Control: Long term 80% removal of Total Suspended Solids (TSS) on an average annual basis is to be provided.

2.2.1 Quantity Control

Storm events up to and including the 100-year storm will be controlled to the 2-year predevelopment level. As the site is already a developed property, a maximum runoff coefficient of 0.50 was used to determine the 2-year predevelopment level to be used as the target controlled release rate for the site. The total release rate will be set to ensure the sum of controlled stormwater release rate plus long-term groundwater dewatering discharge rate remains within the target release rate. Preliminary calculations indicate approximately 270 m³ of storage will be required for quantity control. The following tables summarize the quantity control parameters for the site. Pre and post-development storm drainage plans and preliminary quantity control calculations are provided in **Appendix B**. A high-level overview of the storm servicing and location of the underground storm tank are provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed storm servicing design including orifice sizing and location will be provided at Site Plan Application.

Table 2-1: Pre-development 2-Year Flow Targets

Storm	Drainage Area	Area (Ha)	Runoff Coefficient (C)	Peak Flow (L/s)
2-year	A1-PRE	1.24	0.50	103.1

Table 2-2 – Post-development 100 Year Flows

Drainage Area	Area (Ha)	Adjusted Runoff Coefficient (C)	Target Release Rate (L/s)	Controlled Release rate (L/s)	Groundwater Release Rate (L/s)	Total Release Rate (L/s)	Required Storage (m3)	Provided Storage (m3)
A1-POST	1.24	0.79	103.1	100.0	0.2	100.2	267	267



2.2.2 Water Balance

To address the city's stormwater runoff volume reduction criteria, as a minimum, the first 5mm of runoff is to be retained on-site and managed by way of infiltration, evapotranspiration, or re-use. The total retention volume is determined by multiplying total impervious area by 5mm. Therefore, $0.73\text{ha} \times 5\text{mm} = 37 \text{ m}^3$ of runoff needs to be retained on-site. A passive reservoir below the outlet at the base of the stormwater tank at the basement level can be an option for storing this volume.

As the entirety of the site essentially sits on an underground parking level, infiltration is not an available option for processing the retained stormwater. Rainwater re-use by way of pumping this reservoir to various uses around the site (such as irrigation) is recommended for achieving this goal. As landscape and mechanical designs advance on this project, this report will be amended at Site Plan Application to provide details on the available re-use mechanisms.

2.2.3 Quality Control

Long-term average removal of 80% of total suspended solids (TSS) is indicated for meeting the city runoff quality control requirements. A CDS PMSU2025-5 stormwater treatment unit (or approved equivalent) upstream of the site storm outlet can provide the required treatment to satisfy this criteria. The supplier unit sizing sheet has been provided in **Appendix B**.

Quality treatment will also be applied to groundwater prior to discharge into the municipal storm sewer. This system will be inside the building and sizing and selection is normally done by the mechanical and hydrogeological consultants. This component of the design will be captured at Site Plan Application in coordination with the wider consultant team.

An overview of the stormwater management measures are provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed storm servicing design will be provided at Site Plan Application.

3.0 SANITARY SERVICING

A 250 mm sanitary sewer drains east along Dundix Road. A 375 mm sanitary sewer drains south along Arena Road and is received by a 375 mm sanitary sewer on the north side of Dundas Street East that drains East. A 300 mm sanitary sewer on the south side of Dundas Street East also drains east. Records obtained from the city and region indicate the existing site drains into the 375 mm sanitary sewer along Arena Road through a 150 mm site sanitary outlet. This outlet will be capped and abandoned in place, and a new connection will be established into the Arena Road sewer system with a 200 mm sanitary pipe outlet.

Based on the site statistics provided in Table 1-1 in Section 1.2, the estimated site peak sanitary flow will be **15.2 L/s**. A capacity analysis of five sewer runs downstream of the site connection was completed based on existing and proposed conditions to assess impact of the added flow on the system. Calculations indicate the 15% flow increase does not surcharge the five analyzed



downstream sewers. The sanitary drainage area plan and sanitary design sheets have been provided in **Appendix C**. Drainage area and sewershed plans based on which surrounding drainage patterns were determined and upstream areas and populations were extrapolated are provided in **Appendix E**. An overview of the sanitary servicing is provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed servicing design will be provided at Site Plan Application.

4.0 WATER SERVICING

A 250 mm watermain is available along both Dundix Road and Arena Road. Both roads also accommodate within their right-of-way a 2100 mm feedermain to which connection is not permitted by the region. A 300 mm watermain is available along Dundas Street East. Plans obtained from the city and region indicate the existing site connects into the 250 mm watermain along Arena Road with a 200 mm water connection for fire and domestic. This connection will be capped and abandoned in place. Due to the proposed function of the development as high density residential, the site is required by the region to connect into a minimum municipal watermain size of 300 mm. A new 250 mm connection will be established into the Dundas Street East 300 mm watermain and split near the property line into fire and domestic as per region standard 1-8-3.

Based on the site statistics provided in Table 1-1 in Section 1.2, the estimated site Total Peak Flow + Fire Demand will be **194.4 L/s**. A hydrant flow test conducted on a nearby hydrant serviced by the 300 mm watermain on Dundas Street East on June 15, 2022, indicates a flow of **6413 USGPM (404.6 L/s)** is available at 20 psi, which demonstrates the available municipal water infrastructure has the capacity to support the development.

The water demand calculation sheet and hydrant flow test report have been provided in **Appendix D**. An overview of the water servicing is provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed servicing design will be provided at Site Plan Application.

5.0 GRADING

The natural topography of the site is in a southerly direction at a vertical relief of approximately 1.2 m. A retaining wall is utilized along the northern boundary of the existing site to interface with Dundix Rd while maintaining a relatively flat site. Under post-development conditions, the grading scheme ensures maintenance of existing drainage patterns and containment of drainage within the property boundary. Steps along the southern face of the townhouse blocks and stepped finished floor elevations allow for creation of suitable grading and accessibility conditions throughout the site as well as safe overland flow exit onto Dundas Street East through the street level corridor between the apartment buildings. A retaining wall will not be required to accommodate the proposed development plan. An overview of the site grading is provided on C-101 – Grading and Servicing Plan in **Appendix A**. A detailed grading design will be provided at Site Plan Application.



FUNCTIONAL SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix A SITE PROPOSAL AND ENGINEERING PLANS

Appendix A **SITE PROPOSAL AND ENGINEERING PLANS**

CONDO BLOCK 19135
PEEL CONDOMINIUM PLAN 135

PLAN OF SURVEY
SHOWING TOPOGRAPHIC FEATURES OF
PART OF LOT 7
CONCESSION 1
NORTH AND SOUTH OF DUNDAS STREET
CITY OF MISSISSAUGA
REGIONAL MUNICIPALITY OF PEEL

SCALE 1:500
10 0 10 20 Metres

MAURO GROUP INC.
ONTARIO LAND SURVEYORS
© 2022

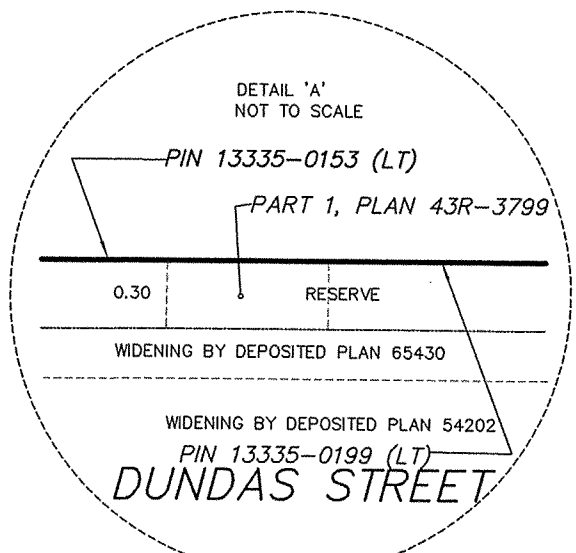
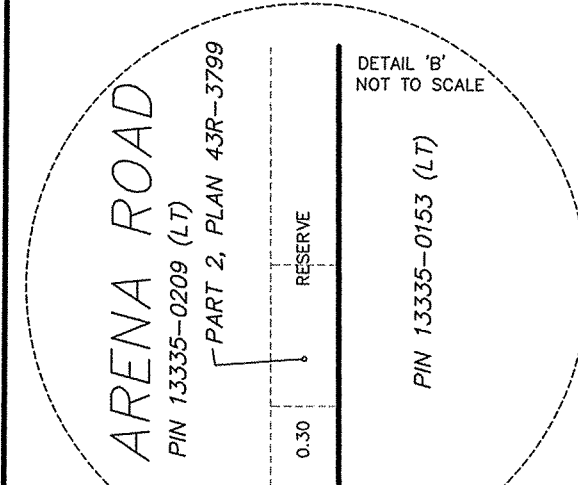
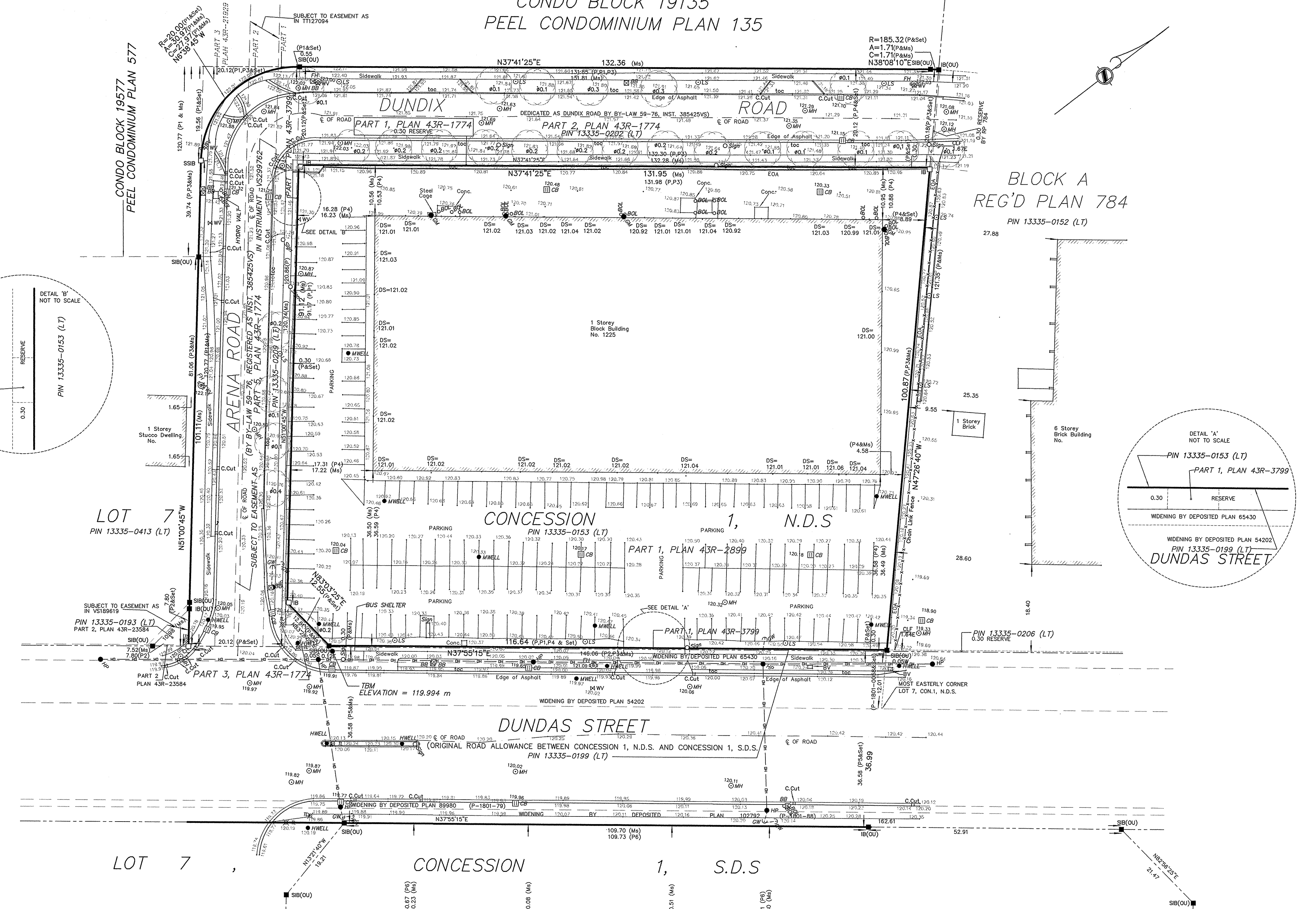
METRIC
DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048.

BEARING NOTE
BEARINGS ARE UTM GRID, DERIVED FROM OBSERVED REFERENCE POINTS A AND B, BY REAL TIME NETWORK (TOPNET) OBSERVATIONS, UTM ZONE 17, NAD83 (CSRS) (2010).

BEARING ROTATION NOTE
ADJUST FOR BEARING COMPARISONS, A ROTATION OF 1'01"00" COUNTER-CLOCKWISE WAS APPLIED TO ASTRONOMIC BEARINGS ON PLAN 43R-2899 TO CONVERT TO UTM ZONE 17, NAD 83 (CSRS).

DISTANCE NOTE
DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.999746

- LEGEND**
- DENOTES FOUND BAR
 - DENOTES PLANTED BAR
 - SIB DENOTES STANDARD IRON BAR
 - CC DENOTES CUT CROSS
 - IB DENOTES IRON BAR
 - PB DENOTES PLASTIC BAR
 - REG'D DENOTES REGISTERED PLAN
 - Ms DENOTES MEASURED
 - WIT DENOTES WITNESS
 - OU DENOTES ORIGIN UNKNOWN
 - P DENOTES 43R-2899
 - P1 DENOTES 43R-21929
 - P2 DENOTES 43R-23584
 - P3 DENOTES 43R-1774
 - P4 DENOTES DATED PLAN OF SURVEY BY UNWIN, MURPHY AND ESTEN, LTD DATED AUGUST 14, 1975
 - P5 DENOTES PLAN P-1801-88
 - P6 DENOTES PLAN OF SURVEY BY MARSHALL MACKLIN MONAGHAN LIMITED DATED JULY 22, 1976
 - NDS DENOTES NORTH OF DUNDAS STREET
 - SDS DENOTES SOUTH OF DUNDAS STREET
 - CB DENOTES CATCH BASIN
 - HP DENOTES HYDRO POLE
 - TL DENOTES TRAFFIC LIGHT
 - BOL DENOTES BOLLARD
 - OLS DENOTES LIGHT STANDARD
 - HYD DENOTES TOP OF HYDRANT
 - MWELL DENOTES MONITORING WELL
 - HWELL DENOTES HAND WELL
 - DS DENOTES DOOR SILL
 - OMH DENOTES MANHOLE
 - GW DENOTES GUY WIRE
 - MWV DENOTES WATER VALVE
 - BB DENOTES BELL BOX
 - GM DENOTES GAS METER
 - OWH DENOTES OVERHEAD WIRE
 - OCut DENOTES CURB CUT
 - loc DENOTES TOP OF CURB
 - EOA DENOTES EDGE OF ASPHALT
 - TBM DENOTES TEMPORARY BENCHMARK
 - BV DENOTES BELL VALT



NOTE
THE SUBSURFACE UTILITIES SHOWN ON THIS PLAN HAVE BEEN PROVIDED

REVISION NOTE
THIS SURVEY WAS REVISED ON TO SHOW TOPOGRAPHIC FEATURES AND SUBUTILITY LOCATES.

BENCHMARK NOTE
ELEVATIONS HEREON ARE GEODETIC IN ORIGIN AND WERE DERIVED FROM THE CITY OF MISSISSAUGA BENCHMARK # 693 HAVING AN ELEVATION 115.521 METRES.

SITE BENCHMARK
CUT CROSS HAS BEEN MARKED ON SIDEWALK NORTH OF DUNDAS STREET, HAVING ELEVATION 119.994 m.

DRAFT

DATE _____ JAMES A. AGYEMANG B.Eng.
ONTARIO LAND SURVEYOR



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PHONE 905.951.6000 FAX 905.857.4911
www.youngsurveying.ca - info@youngsurveying.ca
PARTY CHIEF: BP DRAWN BY: IG CHECKED BY: JA
CLIENT: SMART CENTRES
PATH=F:\PROJECTS\2022\87880\MSCAD\87880_P05_T.DWG

PROJECT No. 22-B7880

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BLOCK A REG'D PLAN 784

PIN 13335-0152 (LT)



LEGEND

- PRIMARY RESIDENTIAL ENTRANCE
- SECONDARY RESIDENTIAL ENTRANCE
- RETAIL ENTRANCE
- EXIT
- FIRE HYDRANT
- SIAMESE CONNECTION
- CONVEX MIRROR
- TRANSFORMER WITH CLEARANCES
- FIRE ROUTE SIGN
- SPOT ELEVATION
- GAS/HYDRO METER
- ROAD WIDENING

#	DATE	DESCRIPTION	BY
1	July 08, 2022	OPA, ZBA & SPA Submission	

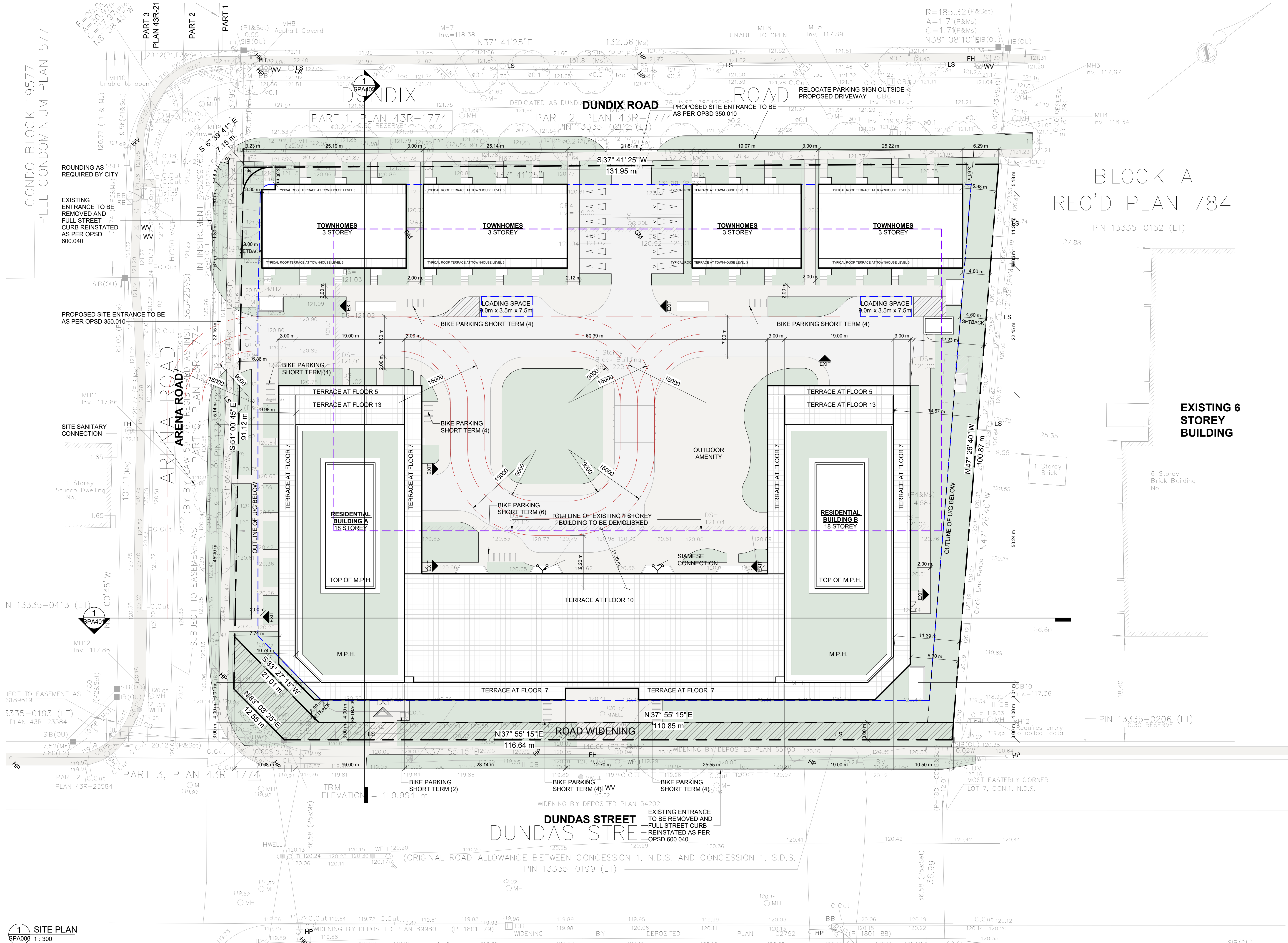
PROJECT
PROPOSED RESIDENTIAL MIXED-USE DEVELOPMENT

1225 Dundas Street E, Mississauga, ON

DRAWING
SITE PLAN / ROOF PLAN

PROJECT NO. 22.117P01
PROJECT DATE
DRAWN BY WJB
CHECKED BY Checker
SCALE As indicated

DRAWING NO. SPA006	REV 1
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1 SITE PLAN
SPA006 1:300

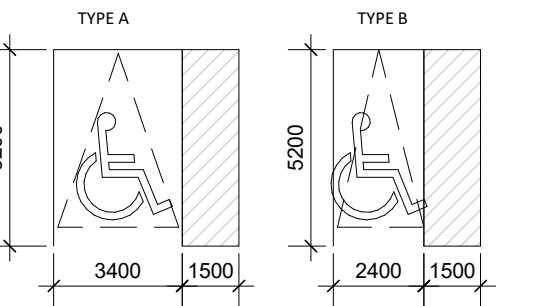
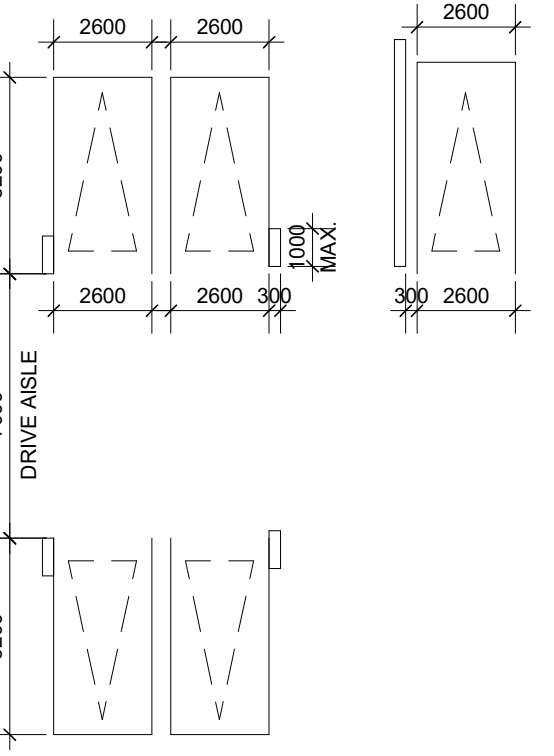
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TYPICAL PARKING DIMENSIONS:

AISLE WIDTH: MIN 7m

TYPICAL PARKING SPACE:
 MIN 2.6 x 5.2 x 2.0m HIGH

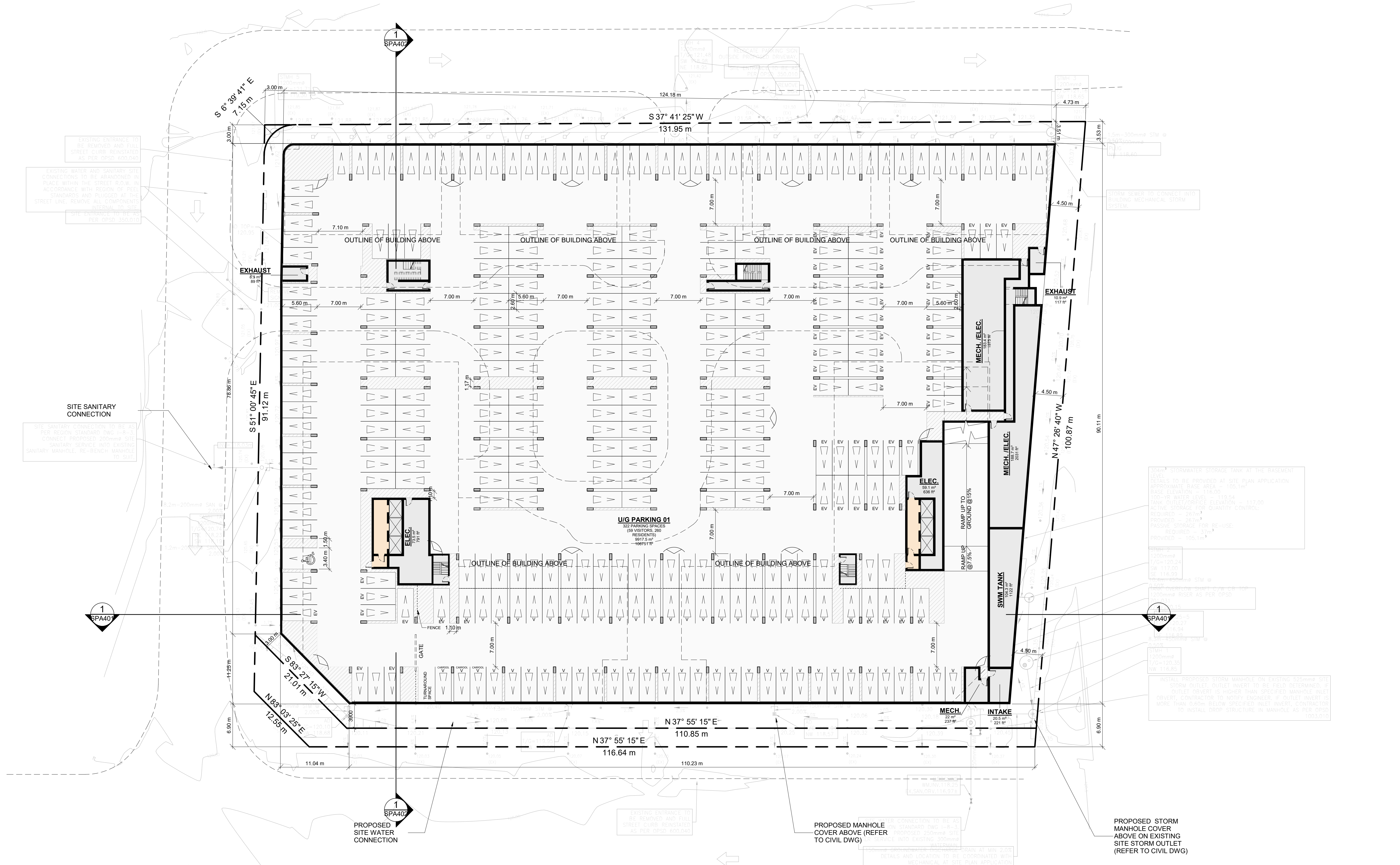


LEGEND

- WALL/COLUMN-MOUNTED CONVEX MIRRORS
- V VISITOR PARKING

The minimum width of a parking space, other than an accessible parking space or parallel parking space, shall be increased to 2.8 m where the length of one side of the parking space abuts a building, structure or part thereof, except for a building, structure or part thereof, that extends 1.0 m or less into the front and/or rear of the parking space.

TOWNHOUSE FRONTYARDS FACING WINDY RD TO BE GRADED TO ENSURE ALL DRAINAGE IS CAPTURED IN AREA DRAINS AND CONVEYED TO THE SITE STORMWATER TANK WITHOUT ANY SPILLAGE INTO THE MUNICIPAL R.O.W. DETAILS TO BE PROVIDED AT SITE PLAN APPLICATION.



14m³ STORMWATER STORAGE TANK AT THE BASEMENT LEVEL.

SPALLS TO BE PROVIDED AT SITE PLAN APPLICATION.

APPROXIMATE BASE AREA = 105.1m²

BASE ELEVATION = 116.00

100-10 WATER LEVEL = 118.54

TANK SLOPE (HORIZONTAL) = 1:17.00

ACTIVE STORAGE FOR QUANTITY CONTROL:

- REQUIRED = 25.0m³
- PROVIDED = 25.0m³
- PROVIDED FOR RE-USE = 17m³
- PROVIDED = 100.1m³

1.200mm x 4.75m x 225.24

SW 112.05

1.200mm x 4.75m x 225.24

SW 118.93

1.200mm x 4.75m x 225.24

SW 116.53

1.200mm x 4.75m x 225.24

SW 118.53

1.200mm x 4.75m x 225.24

SW 118.93

1.200mm x 4.75m x 225.24

SW 118.53

1.200mm x 4.75m x 225.24

SW 118.93

1.200mm x 4.75m x 225.24

SW 118.53

1.200mm x 4.75m x 225.24

SW 118.93

1.200mm x 4.75m x 225.24

SW 118.53

EXISTING ENTRANCE TO BE REMOVED AND FULL STREET CURB REINSTATED AS PER OPSD 600.040.

EXISTING WATER AND SANITARY SITE CONNECTIONS TO BE ABANDONED IN PLACE WITHIN THE STREET R.O.W. IN ACCORDANCE WITH REGION OF PEEL STANDARDS AND PLUGGED AT THE STREET LINE. REMOVE ALL COMPONENTS (MATERIAL OR SITE) PER OPSD 100.010.

SITE SANITARY CONNECTION

SITE SANITARY CONNECTION TO BE AS PER REGION STANDARD TWO (1-8-3) CONNECT PROPOSED 250mm R/W SANITARY SERVICE INTO EXISTING SANITARY MANHOLE, RE-BENCH MANHOLE TO SHIT.

EXISTING ENTRANCE TO BE REMOVED AND FULL STREET CURB REINSTATED AS PER OPSD 600.040.

PROPOSED MANHOLE COVER ABOVE (REFER TO CIVIL DWG)

PROPOSED CONNECTION TO BE AS PER REGION STANDARD TWO (1-8-3) SHALL BE INSTALLED INTO EXISTING 250mm MANHOLE TO INSTALL PROP. STRUCTURE IN MANHOLE AS PER OPSD 100.010.

OTHER CONNECTION TO BE AS PER REGION STANDARD TWO (1-8-3) DETAILS AND LOCATION TO BE COORDINATED WITH MECHANICAL AT SITE PLAN APPLICATION.

PROPOSED STORM MANHOLE COVER ABOVE ON EXISTING SITE STORM OUTLET (REFER TO CIVIL DWG)

PROPOSED SITE WATER CONNECTION

U/G 1
SPA102 1: 300

#	DATE	DESCRIPTION	BY
1	July 08, 2022	OPA,ZBA & SPA Submission	

PROPOSED RESIDENTIAL MIXED-USE DEVELOPMENT

1225 Dundas Street E, Mississauga, ON

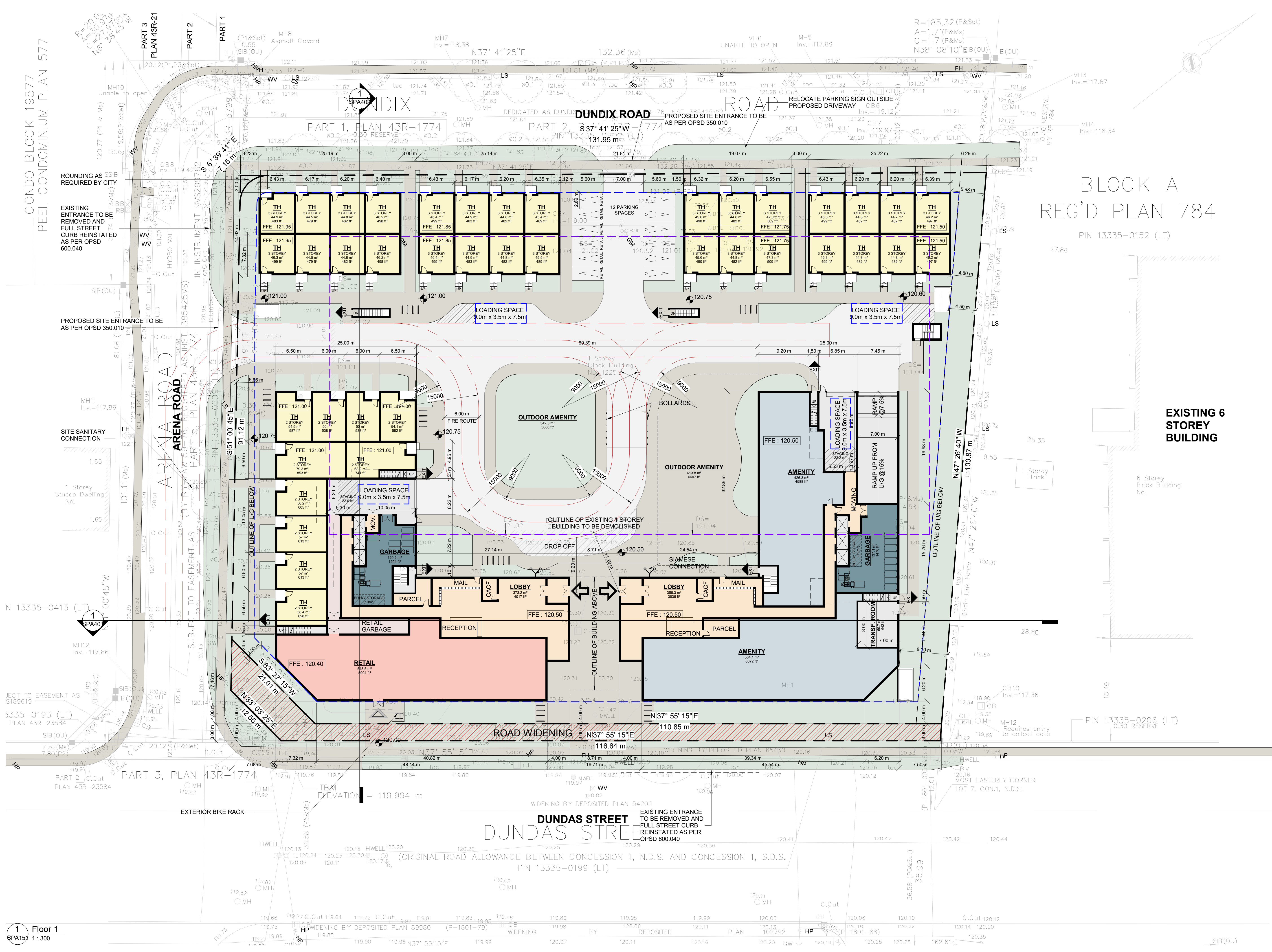
UNDERGROUND LEVEL 1

PROJECT NO: 22.117P01
PROJECT DATE:
DRAWN BY: WJB
CHECKED BY:

SCALE:
As indicated

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LEGEND	
	PRIMARY RESIDENTIAL ENTRANCE
	SECONDARY RESIDENTIAL ENTRANCE
	RETAIL ENTRANCE
	EXIT
	FIRE HYDRANT
	SIAMESE CONNECTION
	CONVEX MIRROR
	TRANSFORMER WITH CLEARANCES
	FIRE ROUTE SIGN
	0.000.00 SPOT ELEVATION
	GAS/HYDRO METER
	ROAD WIDENING



BLOCK A
REG'D PLAN 784

EXISTING 6
STOREY
BUILDING

DUNDAS STREET

1 Floor 1
SPA151 1:300

#	DATE	DESCRIPTION	BY
1	July 08, 2022	CPA, ZBA & SPA Submission	

PROJECT
PROPOSED RESIDENTIAL MIXED-USE DEVELOPMENT
1225 Dundas Street E, Mississauga, ON

DRAWING
FLOOR 1

PROJECT NO.	22.117P01
PROJECT DATE	
DRAWN BY	WJB
CHECKED BY	Checker
SCALE	As indicated

DRAWING NO.	SPA151	REV	1
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UNIT MIX		SALEABLE							AVG. UNIT SIZE	
BLDG	FLOOR	1B	1B+D	2B	2B+D	3B	TH	TOTAL	m ²	ft ²
BLDG BUILDING 1	Floor 1						10	10	118.2	1,273
	Floor 2	2	19	16	5	3		45	69.6	750
	Floor 3		21	20	4	3		48	68.9	742
	Floor 4		21	20	4	3		48	68.9	742
	Floor 5		21	24	2	1		48	67.2	724
	Floor 6		21	24	2	1		48	67.2	724
	Floor 7	6	32	4	1	2		45	55.9	601
	Floor 8	6	32	4	1	2		45	55.9	601
	Floor 9	6	32	4	1	2		45	55.9	601
	Floor 10	6	12	4				22	53.0	570
	Floor 11	9	14	5	1			29	53.6	577
	Floor 12	9	14	5	1			29	53.6	577
	Floor 13	6	13	5	1			25	55.3	596
	Floor 14	6	13	5	1			25	55.3	596
	Floor 15	6	13	5	1			25	55.3	596
	Floor 16	6	13	5	1			25	55.3	596
	Floor 17	6	13	5	1			25	55.3	596
	Floor 18	6	13	5	1			25	55.3	596
	SUBTOTAL		80	317	160	28	17	10	612	
TOTAL UNITS		397		188		17		612		
UNIT MIX		13.1%	51.8%	26.1%	4.6%	2.8%	1.6%	100.0%		
UNIT MIX TOTAL		64.9%		30.7%		2.8%		1.6%		100.0%
AVG UNIT SIZE (m ²)		42.8	55.3	72.0	85.1	86.6	118.2	61.3		
AVG UNIT SIZE (ft ²)		460	595	775	916	932	1,273	660		
AVG UNIT SIZE TOTAL (m ²)		52.8		73.9		86.6		118.2		61.3
AVG UNIT SIZE TOTAL (ft ²)		568		796		932		1,273		660

Last Updated: Tuesday, 23 July 2024 12:50:18 PM

UNIT MIX		SALEABLE		AVG. UNIT SIZE		
BLDG	FLOOR	TH	TOTAL	m ²	ft ²	
BLDG BUILDING 2	Floor 1	30	30	126.6	1,362	
	SUBTOTAL		30	30		
	TOTAL UNITS		30	30		
	UNIT MIX		100.0%	100.0%		
	UNIT MIX TOTAL		100.0%	100.0%		
	AVG UNIT SIZE (m ²)		126.6	126.6	126.6	1,362
	AVG UNIT SIZE (ft ²)		1,362	1,362		
	AVG UNIT SIZE TOTAL (m ²)		126.6	126.6		
AVG UNIT SIZE TOTAL (ft ²)		1,362	1,362			

Last Updated: Tuesday, 23 July 2024 12:50:36 PM

UNIT MIX		SALEABLE							AVG. UNIT SIZE	
BLDG	FLOOR	1B	1B+D	2B	2B+D	3B	TH	TOTAL	m ²	ft ²
BLDG BUILDING 1 + BUILDING 2	Floor 1						40	40	124.5	1,340
	Floor 2	2	19	16	5	3		45	69.6	750
	Floor 3		21	20	4	3		48	68.9	742
	Floor 4		21	20	4	3		48	68.9	742
	Floor 5		21	24	2	1		48	67.2	724
	Floor 6		21	24	2	1		48	67.2	724
	Floor 7	6	32	4	1	2		45	55.9	601
	Floor 8	6	32	4	1	2		45	55.9	601
	Floor 9	6	32	4	1	2		45	55.9	601
	Floor 10	6	12	4				22	53.0	570
	Floor 11	9	14	5	1			29	53.6	577
	Floor 12	9	14	5	1			29	53.6	577
	Floor 13	6	13	5	1			25	55.3	596
	Floor 14	6	13	5	1			25	55.3	596
	Floor 15	6	13	5	1			25	55.3	596
	Floor 16	6	13	5	1			25	55.3	596
	Floor 17	6	13	5	1			25	55.3	596
	Floor 18	6	13	5	1			25	55.3	596
	SUBTOTAL		80	317	160	28	17	40	642	
TOTAL UNITS		397		188		17		642		
UNIT MIX		12.5%	49.4%	24.9%	4.4%	2.6%	6.2%	100.0%		
UNIT MIX TOTAL		61.8%		29.3%		2.6%		6.2%		100.0%
AVG UNIT SIZE (m ²)		42.8	55.3	72.0	85.1	86.6	124.5	64.3		
AVG UNIT SIZE (ft ²)		460	595	775	916	932	1,340	692		
AVG UNIT SIZE TOTAL (m ²)		52.8		73.9		86.6		124.5		64.3
AVG UNIT SIZE TOTAL (ft ²)		568		796		932		1,340		692

Last Updated: Tuesday, 23 July 2024 16:24:28 PM

#	DATE	DESCRIPTION	BY

PROJECT
PROPOSED RESIDENTIAL MIXED-USE DEVELOPMENT
1225 Dundas Street E, Mississauga, ON

DRAWING
STATISTICS

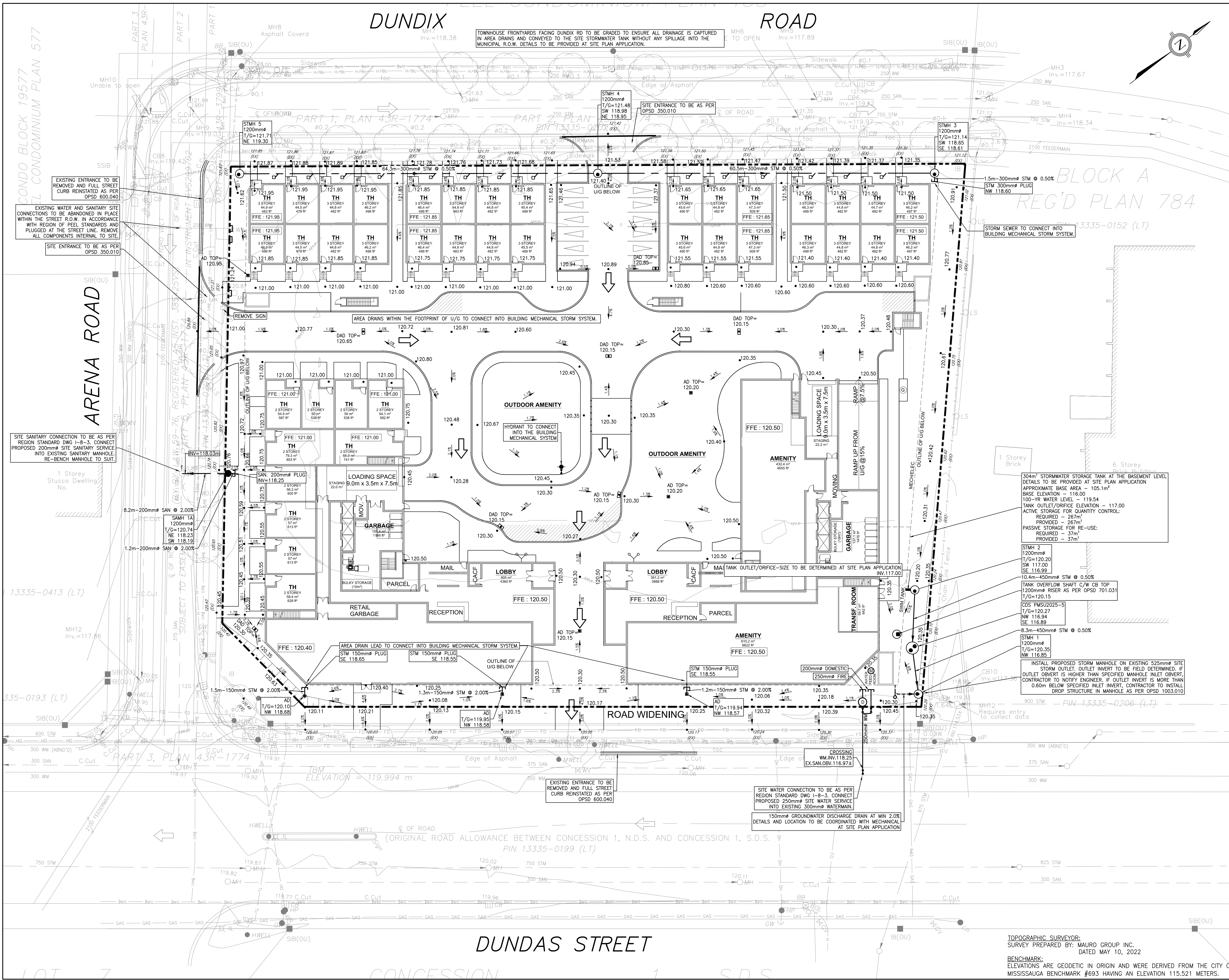
PROJECT NO.
22.117P01
PROJECT DATE
DRAWN BY
MLE
CHECKED BY
NMC
SCALE
1 : 1

DRAWING NO. REV.
SPA003

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- Legend**
- 120.00 PROPOSED ELEVATION
 - 120.95 EXISTING ELEVATIONS
 - OVERLAND FLOW
 - EXISTING OVERLAND FLOW
 - SANITARY MANHOLE
 - STORM MANHOLE
 - CATCH BASIN
 - DOUBLE CATCH BASIN
 - CATCH BASIN MANHOLE
 - AREA DRAIN
 - DOUBLE AREA DRAIN
 - VALVE AND VALVE BOX
 - ◆ HYDRANT AND VALVE
 - ===== CONCRETE CURB
 - DROPPED CURB
 - PROPERTY BOUNDARY
 - WATERMAIN
 - STORM SEWER
 - SANITARY SEWER



304m³ STORMWATER STORAGE TANK AT THE BASEMENT LEVEL
 DETAILS TO BE PROVIDED AT SITE PLAN APPLICATION
 APPROXIMATE BASE AREA - 105.1m²
 100-YR WATER LEVEL - 119.54
 TANK OUTLET/ORIFICE ELEVATION - 117.00
 ACTIVE STORAGE FOR QUANTITY CONTROL:
 REQUIRED - 267m³
 PROVIDED - 267m³
 PASSIVE STORAGE FOR RE-USE:
 REQUIRED - 37m³
 PROVIDED - 37m³

STMH 2
 1200mm^Ø
 T/G=120.20
 SW 117.00
 SE 118.99

10.4m-450mm^Ø STM @ 0.50%
 TANK OVERFLOW SHAFT C/W CB TOP
 1200mm^Ø RISER AS PER OPSD 701.031
 T/G=120.15

CDS PMSU2025-5
 T/G=120.27
 NW 116.94
 SE 116.89

8.3m-450mm^Ø STM @ 0.50%

STMH 1
 1200mm^Ø
 T/G=120.35
 NW 116.85

INSTALL PROPOSED STORM MANHOLE ON EXISTING 525mm^Ø SITE
 STORM OUTLET. OUTLET INVERT TO BE FIELD DETERMINED. IF
 OUTLET ORIFICE IS HIGHER THAN SPECIFIED MANHOLE INLET ORIFICE,
 CONTRACTOR TO NOTIFY ENGINEER. IF OUTLET INVERT IS MORE THAN
 0.60m BELOW SPECIFIED INLET INVERT, CONTRACTOR TO INSTALL
 DROP STRUCTURE IN MANHOLE AS PER OPSD 1003.010

ISSUED FOR IBA/OA	PF	AL	24.08.02
ISSUED FOR IBA/OA/SPA	PF	AL	22.07.08
Issued	By	Appd.	YYMMDD
File Name:	PF	BC	PF
	Dwn.	Chkd.	Dgn.
			YYMMDD

Permit-Seal

Client/Project
DUNDIX REALTY HOLDINGS
 c/o SmartCentres REIT.

PROPOSED RESIDENTIAL DEVELOPMENT
 1225 DUNDAS ST E
 Mississauga, Ontario

Title
GRADING AND SERVICING PLAN

Project No. 160623078
 Drawing No. C-101

Scale 1:300
 Sheet 0 3 9 15m
 Revision

TOPOGRAPHIC SURVEYOR:
 SURVEY PREPARED BY: MAURO GROUP INC.
 DATED MAY 10, 2022

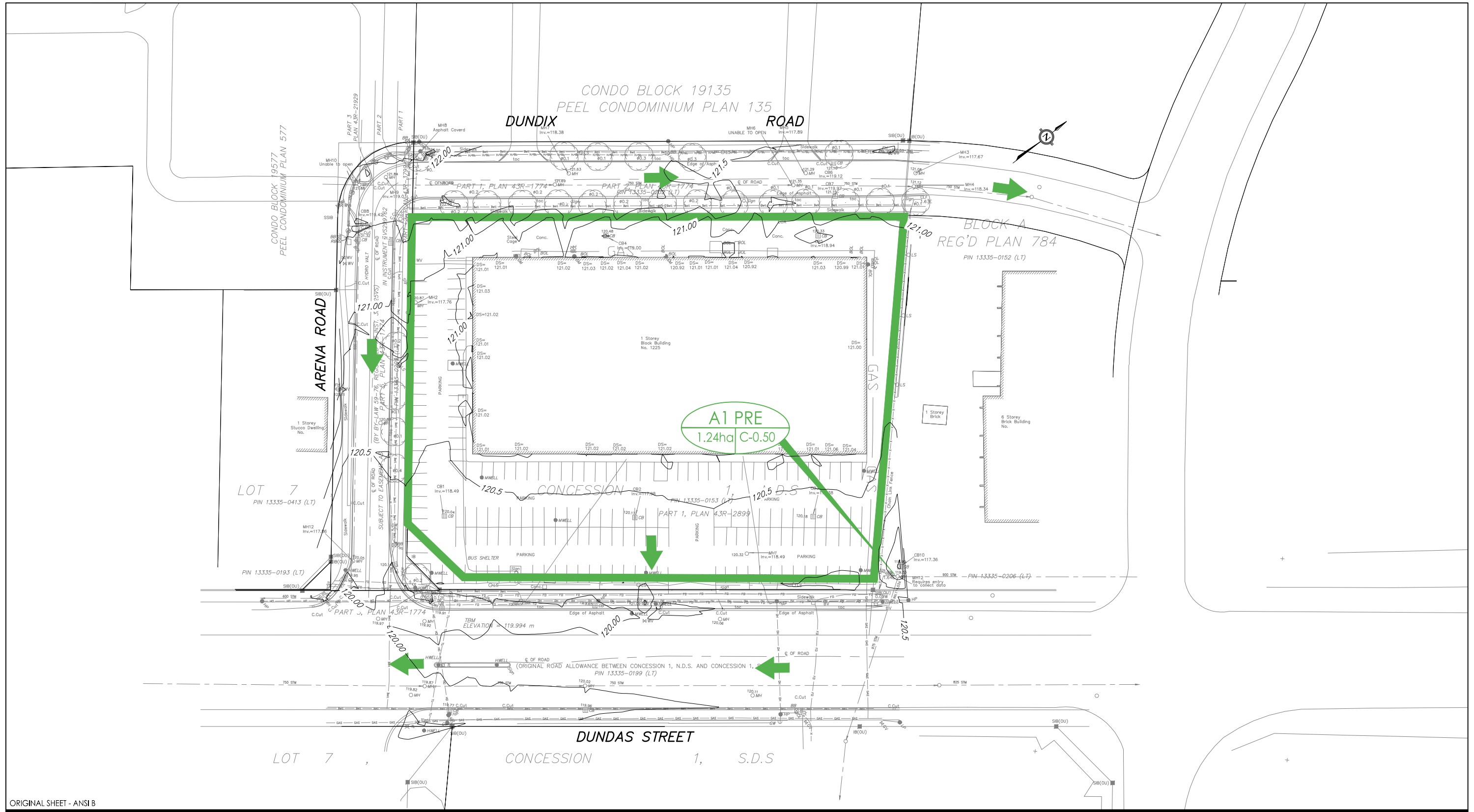
BENCHMARK:
 ELEVATIONS ARE GEODETIC IN ORIGIN AND WERE DERIVED FROM THE CITY OF
 MISSISSAUGA BENCHMARK #693 HAVING AN ELEVATION 115.521 METERS.

V:\01_65A\A\Drawings\160623078\Drawing\Sheet\Main\160623078-C-101-UGP.dwg
 2024/08/01 1:07 PM by: [Name]
 ORIGINAL SHEET - ARCH-D

FUNCTIONAL SERVICING AND STORMWATER MANAGEMENT REPORT

Appendix B STORM

Appendix B **STORM**



ORIGINAL SHEET - ANSI B

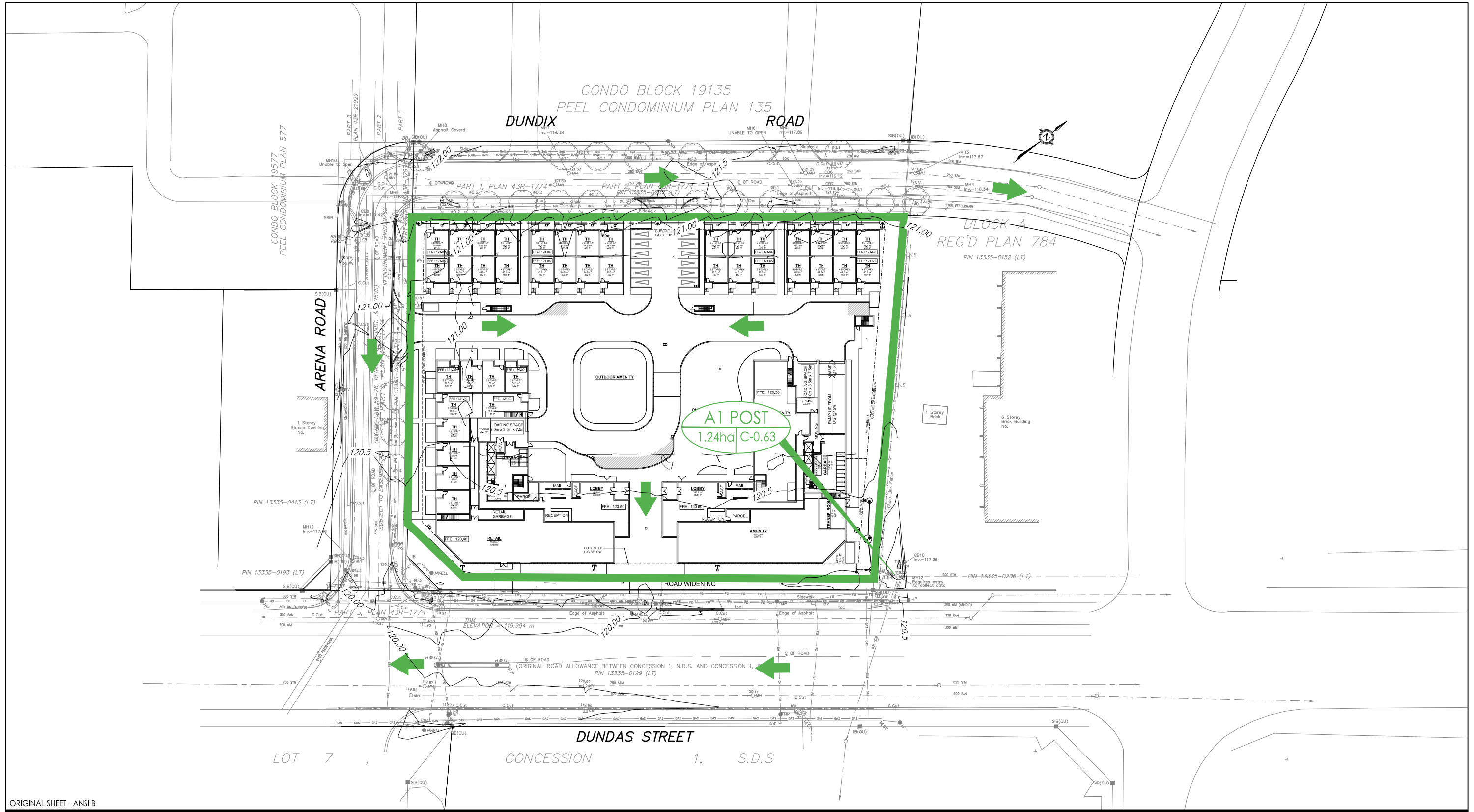


300 - 675 Cochrane Drive West Tower
 Markham, Ontario L3R 0B8
 www.stantec.com

Legend

Notes

Project
 Proposed Residential Development
 1225 Dundas St E
 Figure No.
 2.1
 Title
 PRE-DEVELOPMENT STORM DRAINAGE PLAN



ORIGINAL SHEET - ANSI B

Legend

Notes

Project

Proposed Residential Development
1225 Dundas St E

Figure No.

2.2

Title

POST-DEVELOPMENT STORM DRAINAGE PLAN



300 - 675 Cochrane Drive West Tower
Markham, Ontario L3R 0B8
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floor slab and/or near the foundation and the groundwater would passively drain into these sub drains and discharge directly to sumps. Due to the nature of overburden material, the groundwater will flow through the natural gradient that exists on the site and passively flow into the foundation sub-drains and will not be actively pumped.

Based on the Copper-Jacob equation, the ROI is approximately 85 m, calculation details are provided in Appendix F.

5.2.3 Long-Term Perimeter Drain Flow Rate Estimate

The Dupuit-Forcheimer equation for radial flow from an unconfined aquifer for a fully penetrating excavation was used to obtain a flow rate estimate, and is expressed as follows:

$$Q_w = \frac{\pi K(H^2 - h^2)}{\ln\left(\frac{R_0}{r_e}\right)}$$

Based on the assumptions provided in this report (outlined in Section 5.1 and 5.2), the results of the long-term discharge volume estimate are summarized below and detailed calculations are provided in Appendix F:

Location	Long-Term Peak Flow Rate (L/day)	Notes
Flow into sub-drain after initial dewatering stages	14,100	Long term sub-drain flow value rounded based on Dupuit-Forcheimer's equation. A Safety factor of 2 was used.

The maximum flow rate estimates represent short term events and are not indicative of long-term continuous contributions to the drainage system. Intermittent cycling of sump pumps and seasonal fluctuation in groundwater regimes should be considered for pump specifications. Given that the predicted dewatering volume does not exceed the 50,000 L/day limit, a PTTW is not required.

It should be noted that the dewatering estimates provided in this report are based on the proposed building information available at this time.

If the groundwater encountered during long-term dewatering is discharged to the City of Mississauga and/or Region of Peel Sanitary and Combined sewer, no treatment will likely be required; however, discharge directed to the City of Mississauga and/or Region of Peel Storm Sewers will likely require treatment.

In the event that the long-term foundation drainage is not allowed to discharge into the City's sewer system, the proposed building may be designed and supported by "tanked" water-proofed continuous raft foundation without permanent dewatering (i.e., avoiding permanent perimeter and under-floor drainage system).

Project Name - 1225 Dundas St E
Project Number - 160623078
Date - Jul-24



COMPOSITE RUNOFF COEFFICIENT CALCULATION SHEET

Surface Type	Runoff Coefficient (C)
Roof	0.90
Pavement	0.90
Landscape	0.25
Green Roof	0.50

Drainage Area	Roof (Ha)	Pavement (Ha)	Landscape (Ha)	Green Roof (Ha)	Total (Ha)	Composite C
A1 POST	0.52	0.21	0.51	0.00	1.24	0.63

Note: Areas obtained from Site Statistics in Appendix A

Project Name - 1225 Dundas St E
Project Number - 160623078
Date - Jul-24



PRE-DEVELOPMENT FLOWS

Drainage Area	Area (Ha)	Runoff Coefficient (C)	Time of Concentration (Tc) (min)
A1 PRE	1.24	0.50	15

Storm Event	a	b	c	I (mm/hr)	Target Flow (L/s)
Two Year	610	4.6	0.78	59.9	103.1
Five Year	820	4.6	0.78	80.5	138.7
Hundred Year	1450	4.9	0.78	140.7	242.3

Project Name - 1225 Dundas St E
 Project Number - 160623078
 Date - Jul-24



TARGET RELEASE RATE -	103.1	L/s
RELEASE RATE -	100.2	L/s

100 YEAR FLOWS

A1 POST					Uncontrolled Groundwater Discharge		
Area (Ha) - 1.24		<====Adjustment factor of 1.25 applied		Unadjusted C- 0.63	Area (Ha) - N/A		
C - 0.79					C - N/A		
Control Type - Underground Storage					Control Type - Uncontrolled		
Tc (min) - 15					Tc (min) - N/A		
Available Active Storage (m ³) - 267		Tank Base Area (m ²) - 105.1			Max. Release Rate (L/s) - 0.2		
Release Rate (L/s) - 100		Active Storage Base Elevation (m) - 117.00					
Required Storage (m ³) - 267		Headwater elevation in the tank (m) - 119.54					

Time (Min)	I (mm/hr)	Runoff Volume (m ³)	Released Volume (m ³)	Stored Volume (m ³)	Time Min	I (mm/hr)	Release Rate (L/s)
15	140.7	343	90	253	N/A	N/A	N/A
20	118.1	384	120	264	N/A	N/A	N/A
25	102.4	417	150	267	N/A	N/A	N/A
30	90.8	443	180	263	N/A	N/A	N/A
35	81.8	466	210	256	N/A	N/A	N/A
40	74.6	486	240	246	N/A	N/A	N/A
45	68.7	503	270	233	N/A	N/A	N/A
50	63.8	519	300	219	N/A	N/A	N/A
55	59.6	533	330	203	N/A	N/A	N/A
60	56.0	546	360	186	N/A	N/A	N/A
65	52.8	559	390	169	N/A	N/A	N/A
70	50.0	570	420	150	N/A	N/A	N/A
75	47.6	581	450	131	N/A	N/A	N/A
80	45.4	591	480	111	N/A	N/A	N/A
85	43.4	600	510	90	N/A	N/A	N/A
90	41.6	609	540	69	N/A	N/A	N/A
95	40.0	618	570	48	N/A	N/A	N/A
100	38.5	626	600	26	N/A	N/A	N/A
105	37.1	634	630	4	N/A	N/A	N/A
110	35.8	642	660	0	N/A	N/A	N/A
115	34.7	649	690	0	N/A	N/A	N/A
120	33.6	656	720	0	N/A	N/A	N/A
125	32.6	662	750	0	N/A	N/A	N/A
130	31.6	669	780	0	N/A	N/A	N/A
135	30.7	675	810	0	N/A	N/A	N/A
140	29.9	681	840	0	N/A	N/A	N/A
145	29.1	687	870	0	N/A	N/A	N/A
150	28.4	693	900	0	N/A	N/A	N/A
155	27.7	699	930	0	N/A	N/A	N/A
160	27.0	704	960	0	N/A	N/A	N/A
165	26.4	709	990	0	N/A	N/A	N/A
170	25.8	714	1020	0	N/A	N/A	N/A



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: 1225 Dundas St E
Location: Mississauga, ON
OGS #: 1

Engineer: Stantec
Contact: P. Fatahi, CET
Report Date: 14-Jun-22

Area	1.32	ha	Rainfall Station #	204
Impervious	95	%	Particle Size Distribution	FINE
CDS Model	2025		CDS Treatment Capacity	45 l/s

<u>Rainfall Intensity¹</u> (mm/hr)	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
0.5	9.4%	9.4%	1.6	1.6	3.5	97.9	9.2
1.0	11.0%	20.4%	3.2	3.2	7.0	96.9	10.6
1.5	10.1%	30.5%	4.7	4.7	10.4	95.9	9.7
2.0	9.6%	40.1%	6.3	6.3	13.9	94.9	9.1
2.5	7.9%	48.0%	7.9	7.9	17.4	93.9	7.5
3.0	6.4%	54.4%	9.5	9.5	20.9	92.9	5.9
3.5	4.4%	58.8%	11.0	11.0	24.4	91.9	4.0
4.0	4.2%	63.0%	12.6	12.6	27.9	90.9	3.8
4.5	3.7%	66.7%	14.2	14.2	31.3	89.9	3.3
5.0	3.3%	70.0%	15.8	15.8	34.8	88.9	2.9
6.0	5.6%	75.6%	18.9	18.9	41.8	86.9	4.8
7.0	4.0%	79.6%	22.1	22.1	48.8	84.9	3.4
8.0	3.5%	83.1%	25.2	25.2	55.7	82.9	2.9
9.0	2.2%	85.3%	28.4	28.4	62.7	80.9	1.8
10.0	1.7%	87.0%	31.6	31.6	69.6	78.9	1.3
15.0	6.3%	93.3%	47.3	45.3	100.0	67.2	4.2
20.0	2.3%	95.6%	63.1	45.3	100.0	50.4	1.1
25.0	1.8%	97.3%	78.9	45.3	100.0	40.3	0.7
30.0	0.8%	98.2%	94.7	45.3	100.0	33.6	0.3
35.0	0.9%	99.0%	110.5	45.3	100.0	28.8	0.2
40.0	0.3%	99.3%	126.2	45.3	100.0	25.2	0.1
45.0	0.5%	99.8%	142.0	45.3	100.0	22.4	0.1
50.0	0.2%	100.0%	157.8	45.3	100.0	20.2	0.0

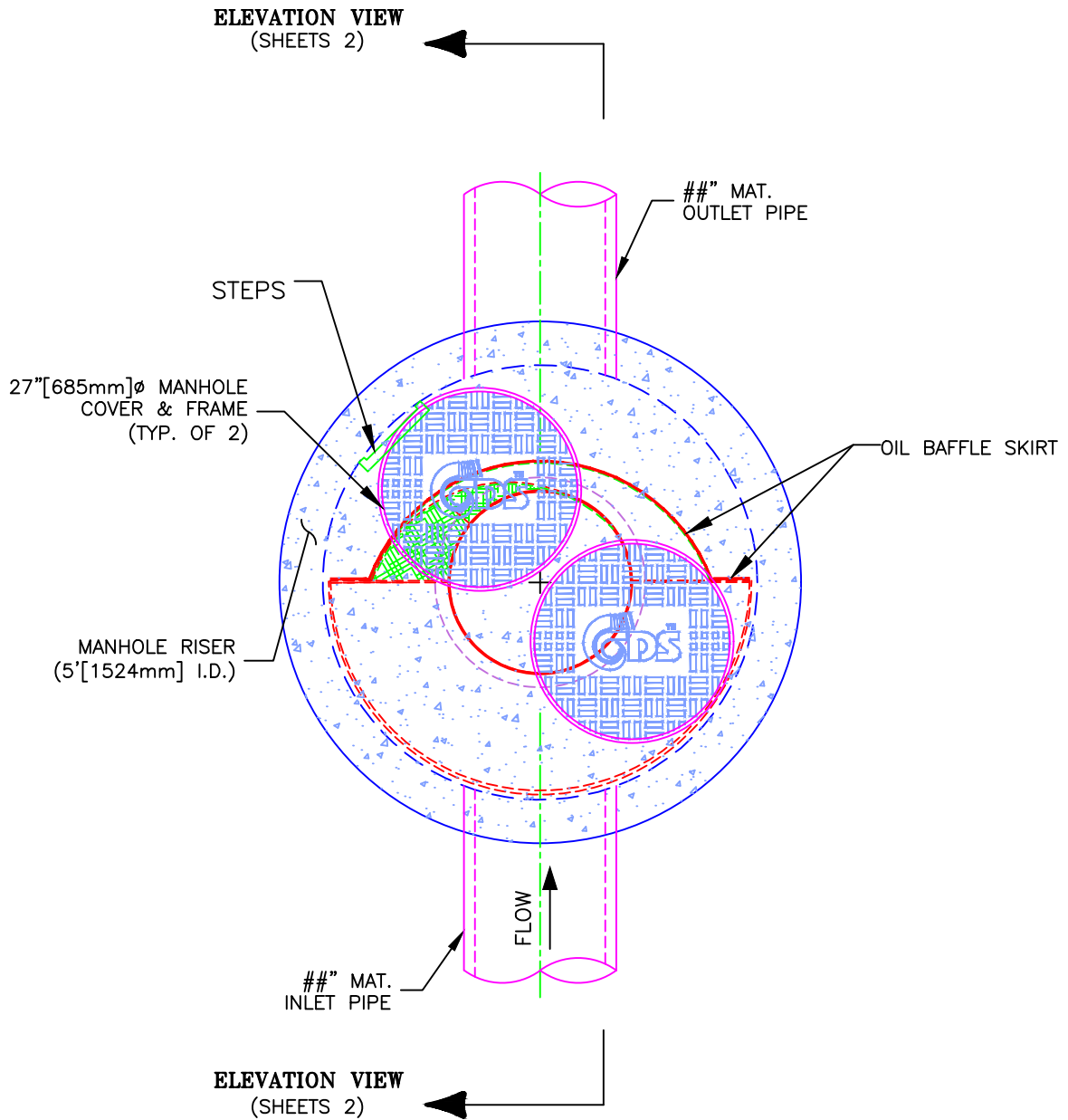
87.2

Removal Efficiency Adjustment² = 6.5%
Predicted Net Annual Load Removal Efficiency = 80.7%
Predicted % Annual Rainfall Treated = 96.7%

1 - Based on 44 years of hourly rainfall data from Canadian Station 6158733, Toronto ON (Airport)
 2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
 3 - CDS Efficiency based on testing conducted at the University of Central Florida
 4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications



PLAN VIEW

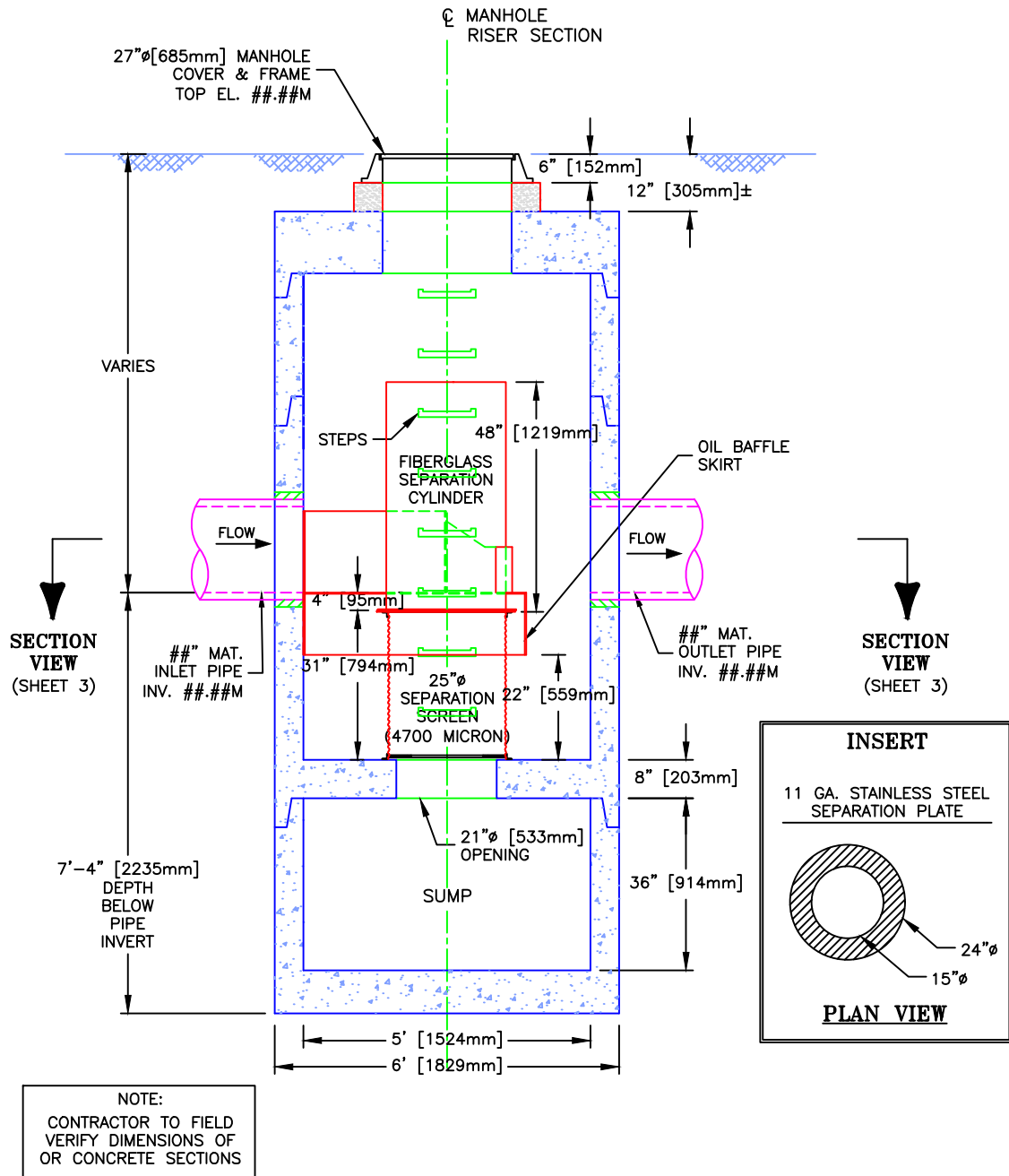


CDS MODEL PMSU20_25m, 1.6 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT

	PROJECT NAME CITY, PROVINCE	JOB# XX-##-###	SCALE 1" = 2'
		DATE ##/##/##	SHEET
		DRAWN INITIALS	1
		APPROV.	



ELEVATION VIEW



CDS MODEL PMSU20_25m, 1.6 CFS TREATMENT CAPACITY STORM WATER TREATMENT UNIT



PROJECT NAME
CITY, PROVINCE

JOB# XX-##-###

DATE ##/##/##

DRAWN INITIALS

APPROV.

SCALE
1" = 3'

SHEET

2

Appendix C **SANITARY**

Project Name - 1225 Dundas St E
 Project Number - 160623078
 Date - July-24



PEAK SANITARY FLOWS CALCULATION SHEET

Criteria Used: Region of Peel 2020 Development Charges Background Study

Function	Population	Units	Flow	Units
Residential - Townhouse	3.4	PPU	290	L/Capita/Day
Residential - Large Apartment (>750 ft ²)	3.0	PPU	290	L/Capita/Day
Residential - Small Apartment (=<750 ft ²)	1.6	PPU	290	L/Capita/Day
Non-Residential	1.0	Per 36 m ² of GFA	270	L/Capita/Day
Extraneous	-	-	0.26	L/s/Ha

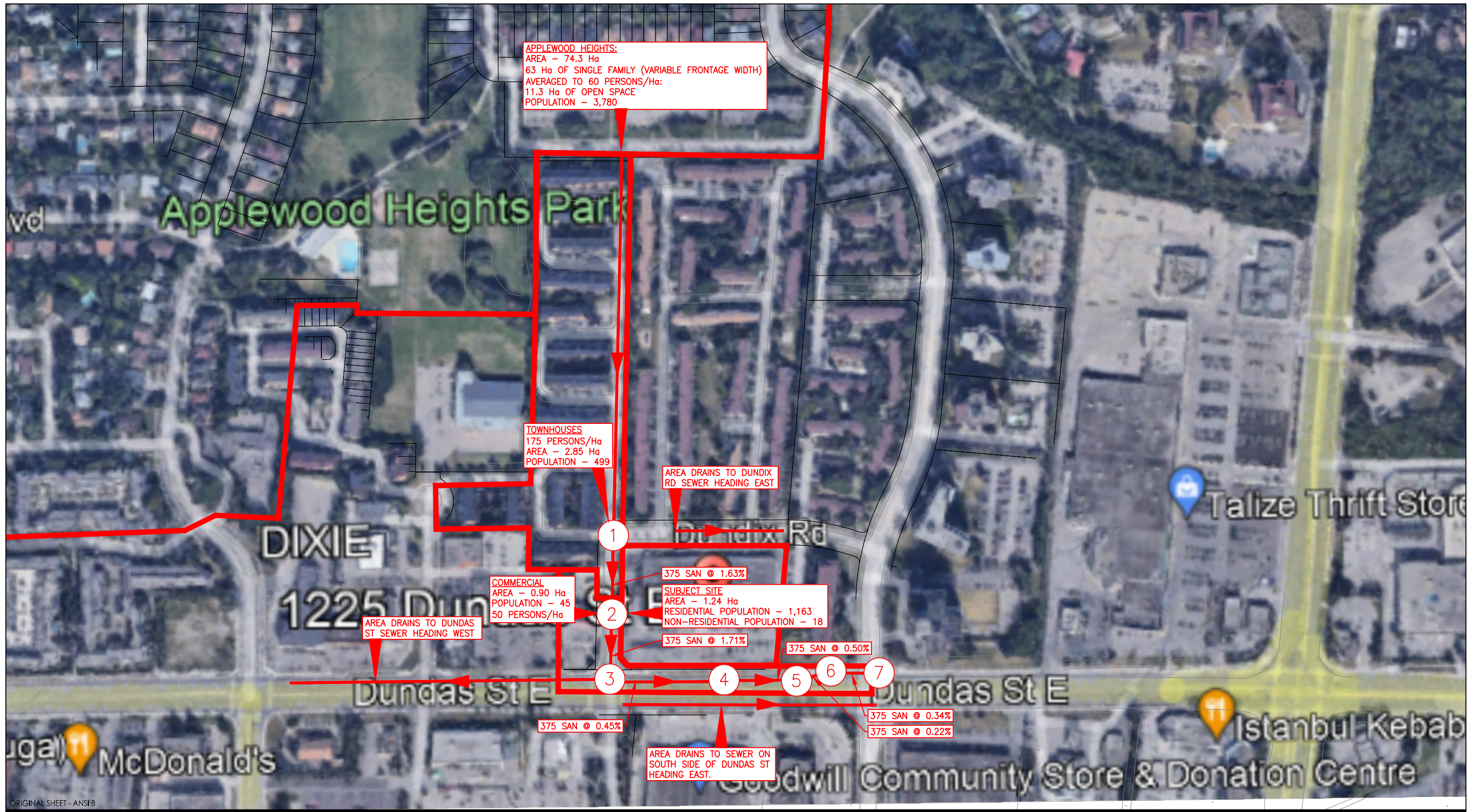
Site Area	1.24	Ha
-----------	------	----

Average Dry Weather Flow

Function	Number of Units	GFA (m ²)	Population	Flow (L/s)
Townhouse	40	-	136	0.46
Residential - Large Apartment (>750 ft ²)	45	-	135	0.45
Residential - Small Apartment (=<750 ft ²)	557	-	892	2.99
Non-Residential	-	626	18	0.06
Total				3.96

Harmon Peaking Factor	3.75
-----------------------	------

Total Site	14.86
Extraneous	0.32
Total Peak Flow (L/s)	15.2



ORIGINAL SHEET - ANSI B



Legend

Notes

Project

Proposed Residential Development
 1225 Dundas St E

Figure No.

3.1

Title

SANITARY DRAINAGE AREA PLAN

Project Name - 1225 Dundas St E
 Project Number - 160623078
 Date - Jun-22



SANITARY CALCULATION SHEET - EXISTING CONDITIONS

Criteria Used: Region of Peel 2020 Development Charges Background Study

Function	Population	Units	Flow	Units	Peaking Factor
Residential - Townhouse	3.4	PPU	290	L/Capita/Day	Harmon Peaking Factor
Residential - Large Apartment (>750 ft ²)	3.0	PPU	290	L/Capita/Day	Harmon Peaking Factor
Residential - Small Apartment (≤750 ft ²)	1.6	PPU	290	L/Capita/Day	Harmon Peaking Factor
Non-Residential	1.0	Per 36 m ² of GFA	270	L/Capita/Day	Harmon Peaking Factor
Extraneous	-	-	0.26	L/s/Ha	-

Location				Site Area (Ha)		Residential Population*		Non-Residential Population*		Peaking Factor	Average Flow (L/s)	Peak Flow (L/s)	Infiltration (L/s)	Total Peak Flow (L/s)	Pipe Diameter (mm)	Pipe Slope (%)	Velocity (m/s)	Capacity (L/s)	% Capacity Used
Block	Area Tag	From	To	Local Area	Cumulative Area	Local Population	Cumulative Population	Local Population	Cumulative Population										
Applewood Heights + Townhouses northwest of site + Subject Site	-	Node 1	Node 2	78.47	78.47	4279	4279	66	66	3.30	14.6	48.1	20.4	68.5	375	1.63	2.03	223.8	31%
Commercial west of Arena Rd + North half of Dundas St E	-	Node 2	Node 3	0.90	79.37	0	4279	45	111	3.30	14.7	48.5	20.6	69.1	375	1.71	2.08	229.3	30%
Dundas St E	-	Node 3	Node 4	0.00	79.37	0	4279	0	111	3.30	14.7	48.5	20.6	69.1	375	0.45	1.06	117.6	59%
Dundas St E	-	Node 4	Node 5	0.00	79.37	0	4279	0	111	3.30	14.7	48.5	20.6	69.1	375	0.50	1.12	124.0	56%
Dundas St E	-	Node 5	Node 6	0.00	79.37	0	4279	0	111	3.30	14.7	48.5	20.6	69.1	375	0.22	0.74	82.2	84%
Dundas St E	-	Node 6	Node 7	0.00	79.37	0	4279	0	111	3.30	14.7	48.5	20.6	69.1	375	0.34	0.93	102.2	68%

* See Figure 3.1 in this appendix for criteria used for external populations. Existing population for subject site = 1.32 Ha x 50 persons/Ha as per region standards = 66



SANITARY CALCULATION SHEET - PROPOSED CONDITIONS

Criteria Used: Region of Peel 2020 Development Charges Background Study

Function	Population	Units	Flow	Units	Peaking Factor
Residential - Townhouse	3.4	PPU	290	L/Capita/Day	Harmon Peaking Factor
Residential - Large Apartment (>750 ft ²)	3.0	PPU	290	L/Capita/Day	Harmon Peaking Factor
Residential - Small Apartment (≤750 ft ²)	1.6	PPU	290	L/Capita/Day	Harmon Peaking Factor
Non-Residential	1.0	Per 36 m ² of GFA	270	L/Capita/Day	Harmon Peaking Factor
Extraneous	-	-	0.26	L/s/Ha	-

Location				Site Area (Ha)		Residential Population*		Non-Residential Population*		Peaking Factor	Average Flow (L/s)	Peak Flow (L/s)	Infiltration (L/s)	Foundation Discharge (L/s)		Total Peak Flow (L/s)	Pipe Diameter (mm)	Pipe Slope (%)	Velocity (m/s)	Capacity (L/s)	% Capacity Used
Block	Area Tag	From	To	Local Area	Cumulative Area	Local Population	Cumulative Population	Local Population	Cumulative Population					Local	Cumulative						
Applewood Heights + Townhouses northwest of site	-	Node 1	Node 2	77.15	77.15	4279	4279	0	0	3.31	14.4	47.5	20.1	0.0	0.0	67.6	375	1.63	2.03	223.8	30%
Subject Site + Commercial west of Arena Rd + North half of Dundas St E	-	Node 2	Node 3	2.14	79.29	1163	5442	63	63	3.21	18.5	59.2	20.6	0.0	0.0	79.8	375	1.71	2.08	229.3	35%
Dundas St E	-	Node 3	Node 4	0.00	79.29	0	5442	0	63	3.21	18.5	59.2	20.6	0.0	0.0	79.8	375	0.45	1.06	117.6	68%
Dundas St E	-	Node 4	Node 5	0.00	79.29	0	5442	0	63	3.21	18.5	59.2	20.6	0.0	0.0	79.8	375	0.50	1.12	124.0	64%
Dundas St E	-	Node 5	Node 6	0.00	79.29	0	5442	0	63	3.21	18.5	59.2	20.6	0.0	0.0	79.8	375	0.22	0.74	82.2	97%
Dundas St E	-	Node 6	Node 7	0.00	79.29	0	5442	0	63	3.21	18.5	59.2	20.6	0.0	0.0	79.8	375	0.34	0.93	102.2	78%

* See Figure 3.1 in this appendix for criteria used for external populations. See Table 1-1: Site Development Statistics in Section 1.2 of the report for criteria used for determining population for subject site

Appendix D WATER

Project Name - 1225 Dundas St E
 Project Number - 160623078
 Date - Jul-24



DOMESTIC WATER DEMAND CALCULATION

CRITERIA SUMMARY

Criteria Used: **Region of Peel 2020 Development Charges Background Study**

Function	Population	Units	Flow	Units	Max Day Factor	Peak Hour Factor
Residential - Townhouse	3.4	PPU	270	L/Capita/Day	1.8	3.0
Residential - Large Apartment (>750 ft ²)	3.0	PPU	270	L/Capita/Day	1.8	3.0
Residential - Small Apartment (=<750 ft ²)	1.6	PPU	270	L/Capita/Day	1.8	3.0
Non-Residential	1.0	Per 36 m ² of GFA	250	L/Capita/Day	1.4	3.0

Function	Number of Units	GFA (m2)	Population	Average Day (L/d)	Max Day (L/d)	Peak Hour (L/hr)	Peak Domestic Flow (L/s)
Townhouse	40	-	136	36720.0	66096.0	4590.0	1.3
Residential - Large Apartment (>750 ft ²)	45	-	135	36450.0	65610.0	4556.3	1.3
Residential - Small Apartment (=<750 ft ²)	557	-	892	240840.0	433512.0	30105.0	8.4
Non-Residential	-	626	18	4500.0	6300.0	562.5	0.2
Total							11.1

FIRE FLOW DEMAND CALCULATION

Assumptions:

- Type of Construction- Fire Resistive
- Protection Rating- One Hour Rating
- Occupancy Type- Limited Combustible
- Sprinkler Protection- NFPA 13
- E- Distance to closest structure on the east side (m)
- S- Distance to closest structure on the south side (m)
- W- Distance to closest structure on the west side (m)
- N- Distance to closest structure on the north side (m)

Location	C	Largest Floor Area (m ²)	Above Floor Area (m ²)	Below Floor Area (m ²)	F1 (L/min)	Occupancy Factor	F2 (L/min)
Site	0.6	5162	5162	5162	11615	-15%	9873

Sprinkler Protection Factor	F2 (L/min)	E	S	W	N	Exposure Factor	F (L/min)
-30%	6911	3	44	28	24	50%	11000

F (L/s)	F (USGPM)
183.3	2906

Peak Domestic Flow + Fire Demand (L/s) 194.4

521 Piercey Road, Unit 6
Bolton, ON, L7E 5B5



T: 905.951.1877 F: 905.951.1878
E: office@vtfireprotection.com

HYDRANT FLOW TEST REPORT

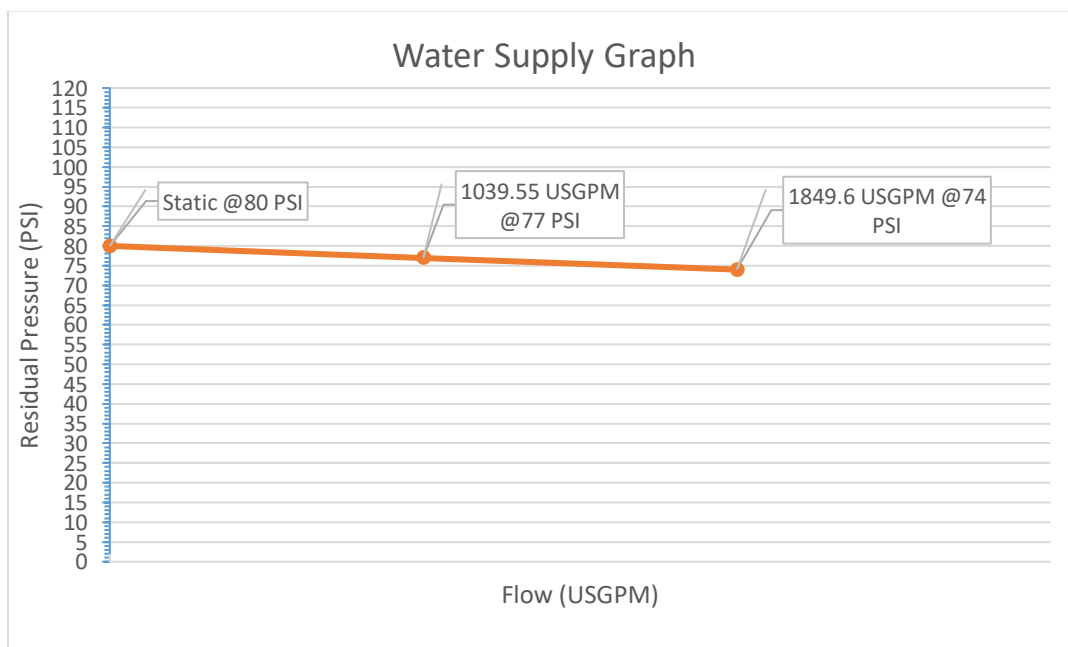
SITE INFORMATION

Test Location:	Dundas St. East / Blundell, Mississauga	Underground W/M Size:	12" (300 mm)
Date of Test:	June 15, 2022	Pipe Material:	PVC
Time of Test:	11:00 am		
Flow Hydrant ID:	1225 Dundas St. East	Flow Hyd. Co-Efficient:	0.85
Res. Hydrant ID:	1214 Dundas St. East (@ Arena Rd.)	Static Reading:	80 PSI

FIELD DATA

Test No.	Outlet Size (inches)	Pitot Reading (PSI)	Flow Adjustment (USGPM)	Total Flow (USGPM)	Residual (PSI)	Field Notes (if applicable)
1	1 – 1¼"	-	-	-	-	-
2	1 – 2½"	43	1,223	1,039.55	77	-
3	2 – 2½"	34, 34	2,176	1,849.60	74	-
4	-	-	-	-	-	-

WATER SUPPLY GRAPH



ADDITIONAL COMMENTS

- All readings are true at the time of actual hydrant test.
- 1¼" playpipe was not conducted due to the site condition/unsafe (traffic)

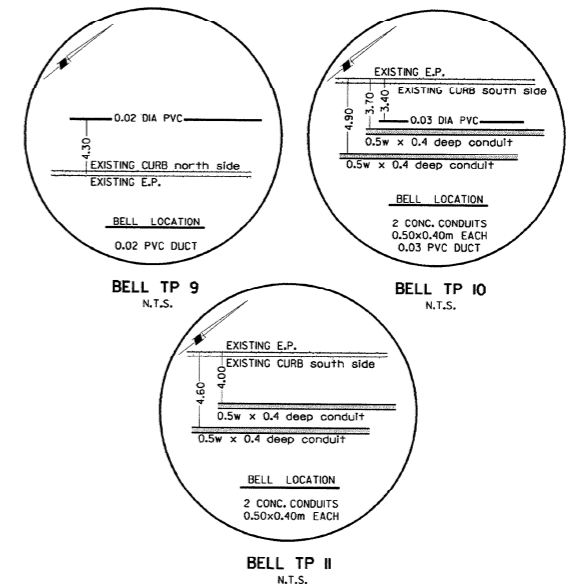
Appendix E BACKGROUND MATERIALS

SERVICE DATA					
SERVICE	DATE	INIT.	SERVICE	DATE	INIT.
SAN. SEWERS	97 II	B.G.M.	GAS MAINS	97 II	B.G.M.
SIM. SEWERS	97 II	B.G.M.	BELL U/G CABLE	97 II	B.G.M.
WATERMANS	97 II	B.G.M.	HYDRO U/G CABLE	97 II	B.G.M.
O.C.W.A.	97 II	B.G.M.			

REVISIONS		
DATE	DETAILS	INIT.

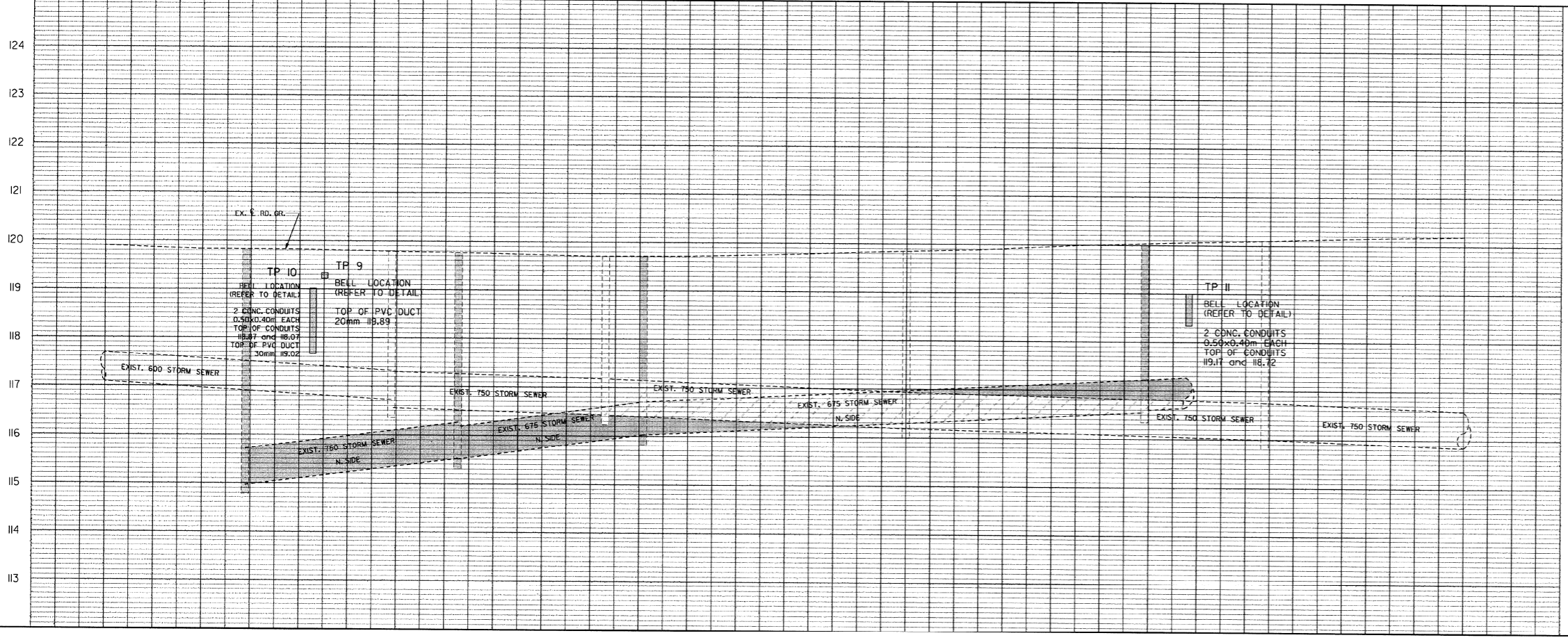
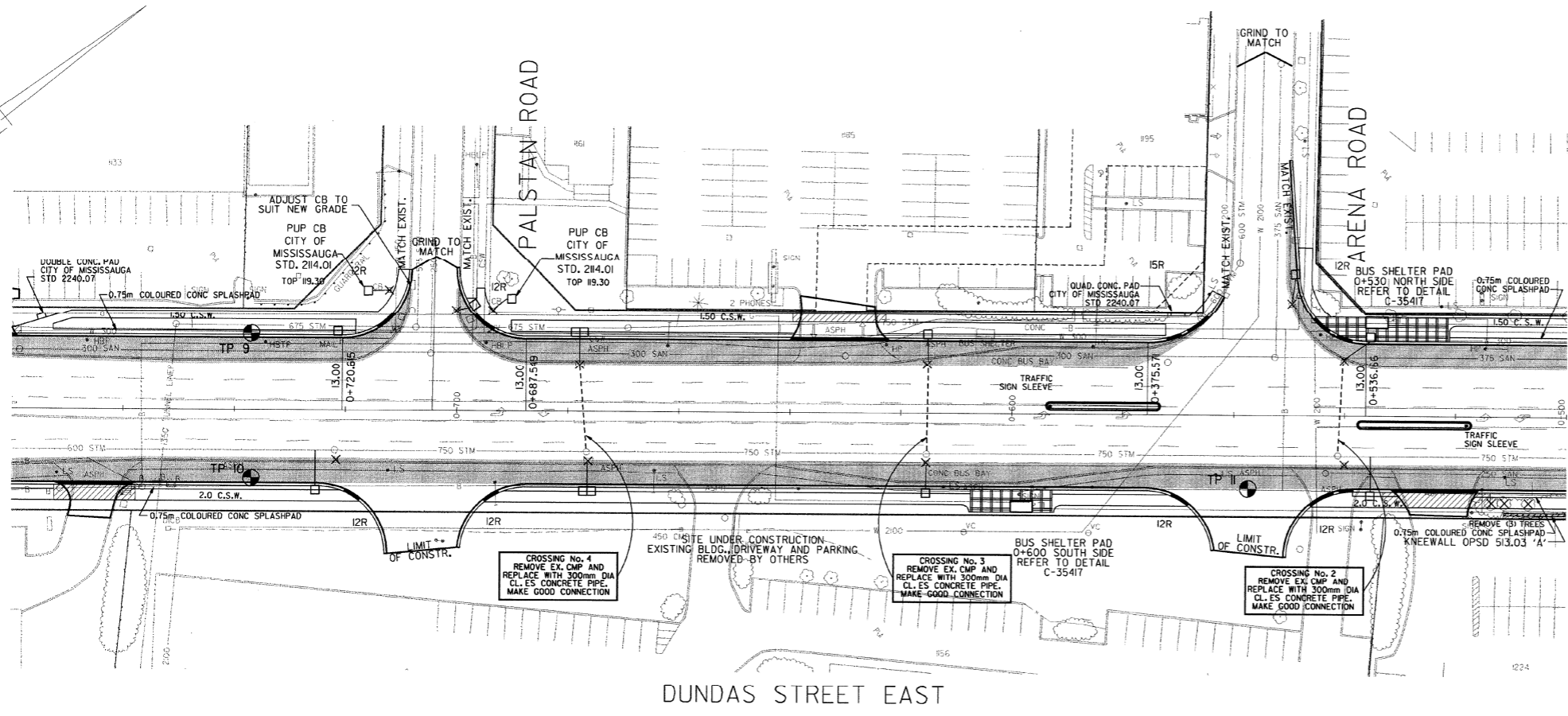
LEGEND

- FULL DEPTH EXCAVATION REQUIRED
- 180mm DEPTH CONCRETE SIDEWALK



REFER TO C-35407

REFER TO C-35405



DESIGN BY: *D. Galati*
DOMENIC GALATI C.E.T.

APPROVED BY: *D. Marchese*
REGISTERED PROFESSIONAL ENGINEER
D. MARCHESE
PROVINCIAL ENGINEERING BOARD

DEPARTMENTAL APPROVAL: *W. Scott Anderson*
W. SCOTT ANDERSON P. ENG.

MISSISSAUGA
Transportation and Works

DUNDAS STREET EAST
DIXIE ROAD TO CAWTHRA ROAD
STN. 0+500 TO STN. 0+780

119.900	119.830	119.830	119.790	119.760	119.710	119.720	119.770	119.840	119.890	119.990	120.050	120.100	120.160	120.200	EX. C. RD. GR.
0+780	0+760	0+740	0+720	0+700	0+680	0+660	0+640	0+620	0+600	0+580	0+560	0+540	0+520	0+500	CHAINAGE

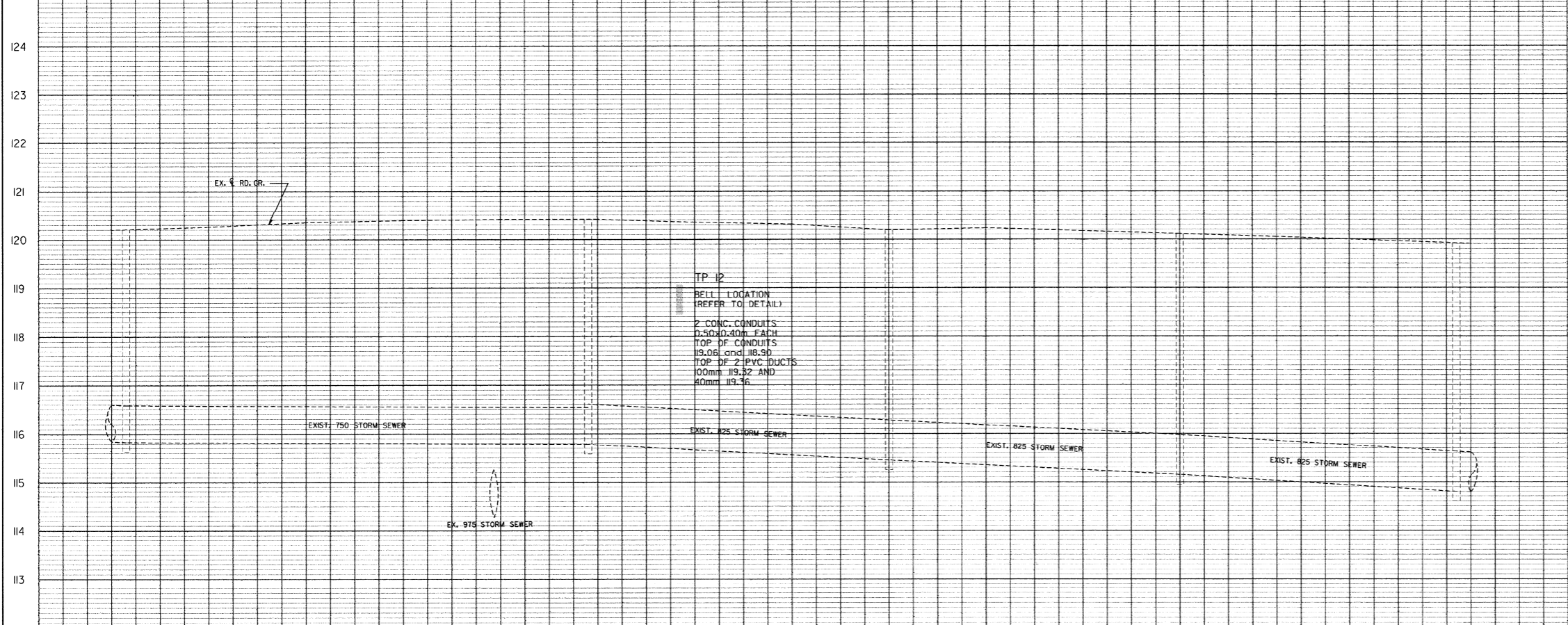
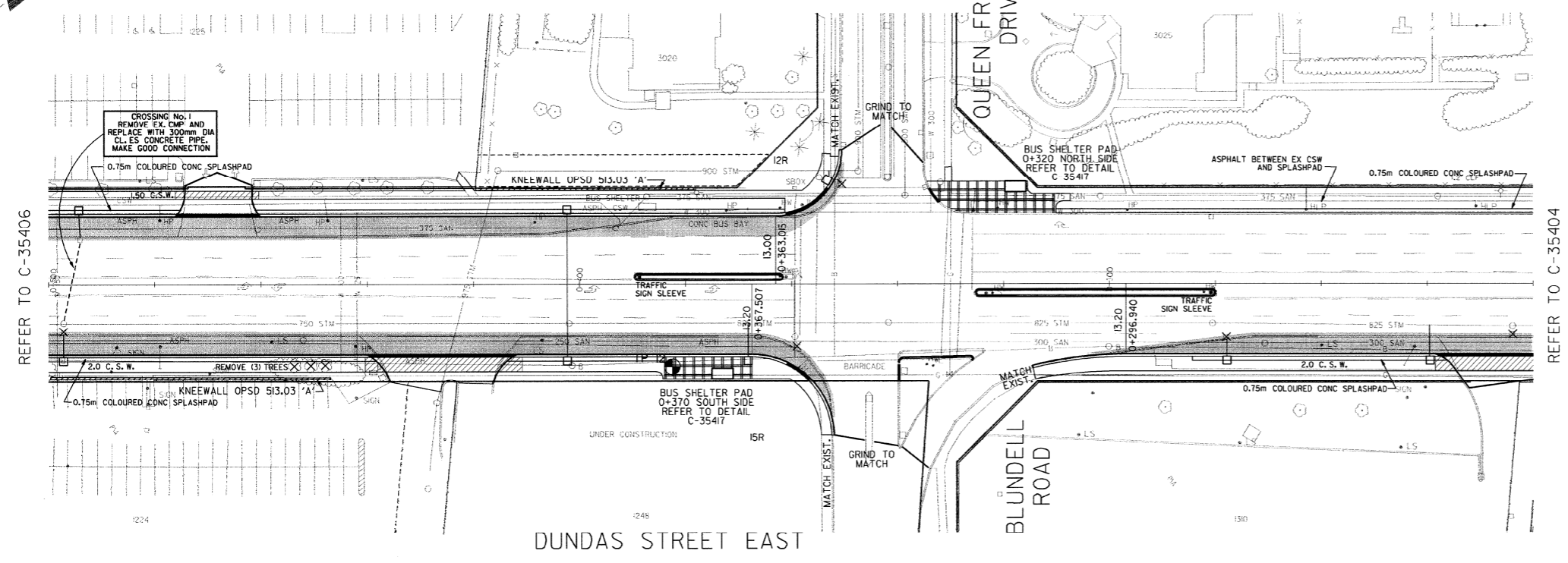
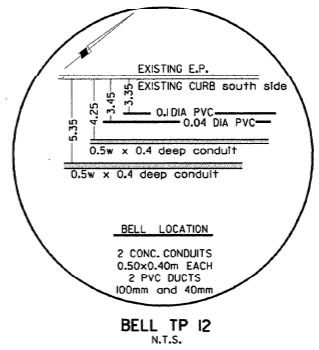
SCALE: HORIZ. 1:500 VERT. 1:50	AREA: 2-15, 2-20	PROJECT No. 99-105
C.A.D.D. BY: B.G.M.	CHECKED BY:	PLAN No.
DATE: 99 01	SHEET 3 OF 10	C-35406

SERVICE DATA					
SERVICE	DATE	INIT.	SERVICE	DATE	INIT.
SAN. SEWERS	97 II	B.G.M.	GAS MAINS	97 II	B.G.M.
STM. SEWERS	97 II	B.G.M.	BELL U/G CABLE	97 II	B.G.M.
WATERMANS	97 II	B.G.M.	HYDRO U/G CABLE	97 II	B.G.M.
O.C.W.A.	97 II	B.G.M.			

REVISIONS		
DATE	DETAILS	INIT.

LEGEND

- FULL DEPTH EXCAVATION REQUIRED
- 180mm DEPTH CONCRETE SIDEWALK



DESIGN BY: *S. Galati*
 DOMENIC GALATI C.E.T.

APPROVED BY: *D. Marchese*
 REGISTERED PROFESSIONAL ENGINEER
 PROVINCE OF ONTARIO
 M.A. 12/99

DEPARTMENTAL APPROVAL: *W. Scott Anderson*
 W. SCOTT ANDERSON P. ENG.

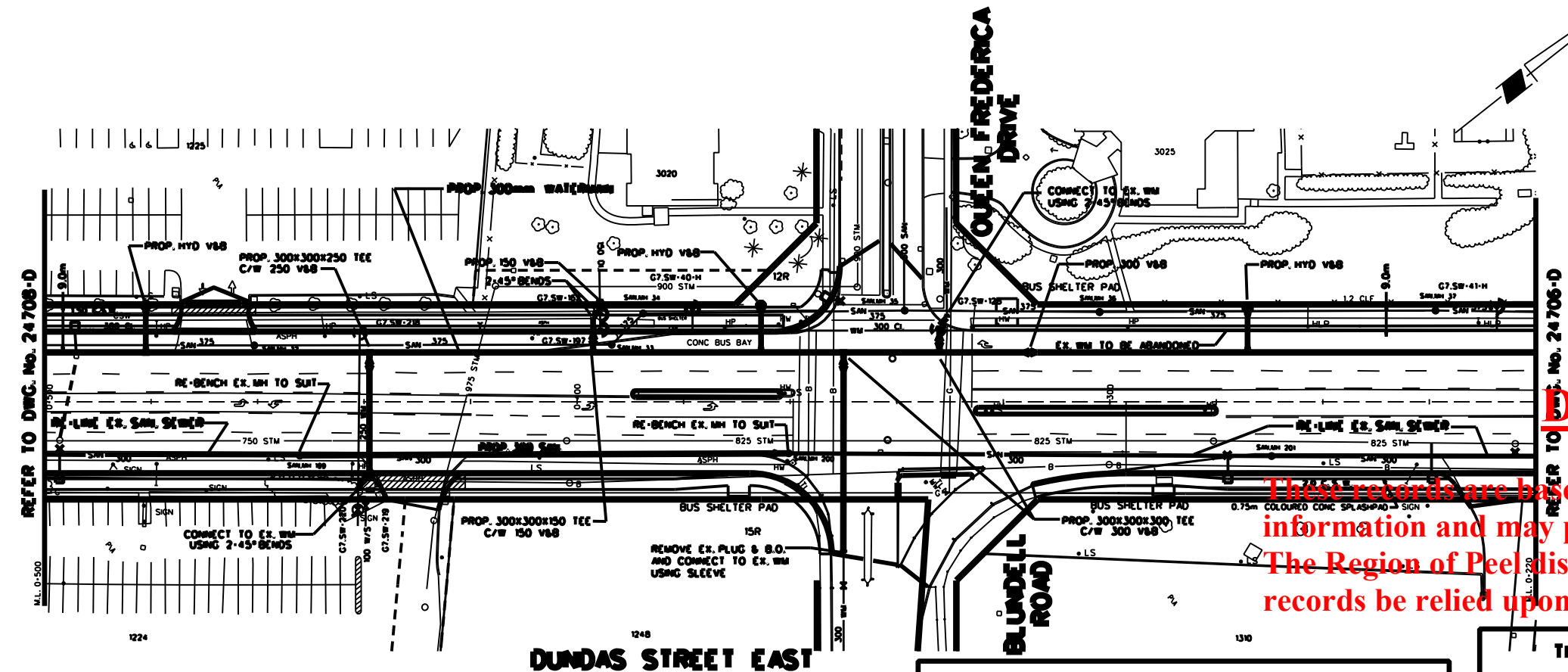
MISSISSAUGA
 Transportation and Works

DUNDAS STREET EAST
 DIXIE ROAD TO CAWTHRA ROAD

120.200	120.260	120.350	120.390	120.410	120.410	120.350	120.310	120.200	120.230	120.170	120.110	120.050	119.980	119.910	EX. C. RD. GR.
0+500	0+480	0+460	0+440	0+420	0+400	0+380	0+360	0+340	0+320	0+300	0+280	0+260	0+240	0+220	CI/AINAGE

SCALE: HORIZ. 1:500 VERT. 1:500	AREA: 2-15, 2-20	PROJECT No. 99-105
C.A.D.D. BY: B.G.M.	CHECKED BY:	PLAN No.
DATE: 22.01	SHEET 2 OF 10	C-35405

SERVICE DATA					
SAN SEWERS	DATE	INT.	SERVICE	DATE	INT.
STW SEWERS			GAS MAINS		
WATERMANS			BELL W/C CABLE		
O.C.W.A.			HYDRO W/C CABLE		
REVISIONS					
DATE	DETAILS			INT.	
DEC. 1999	AS SHOWN			J.P.	



REFER TO DWG. No. 24708-D

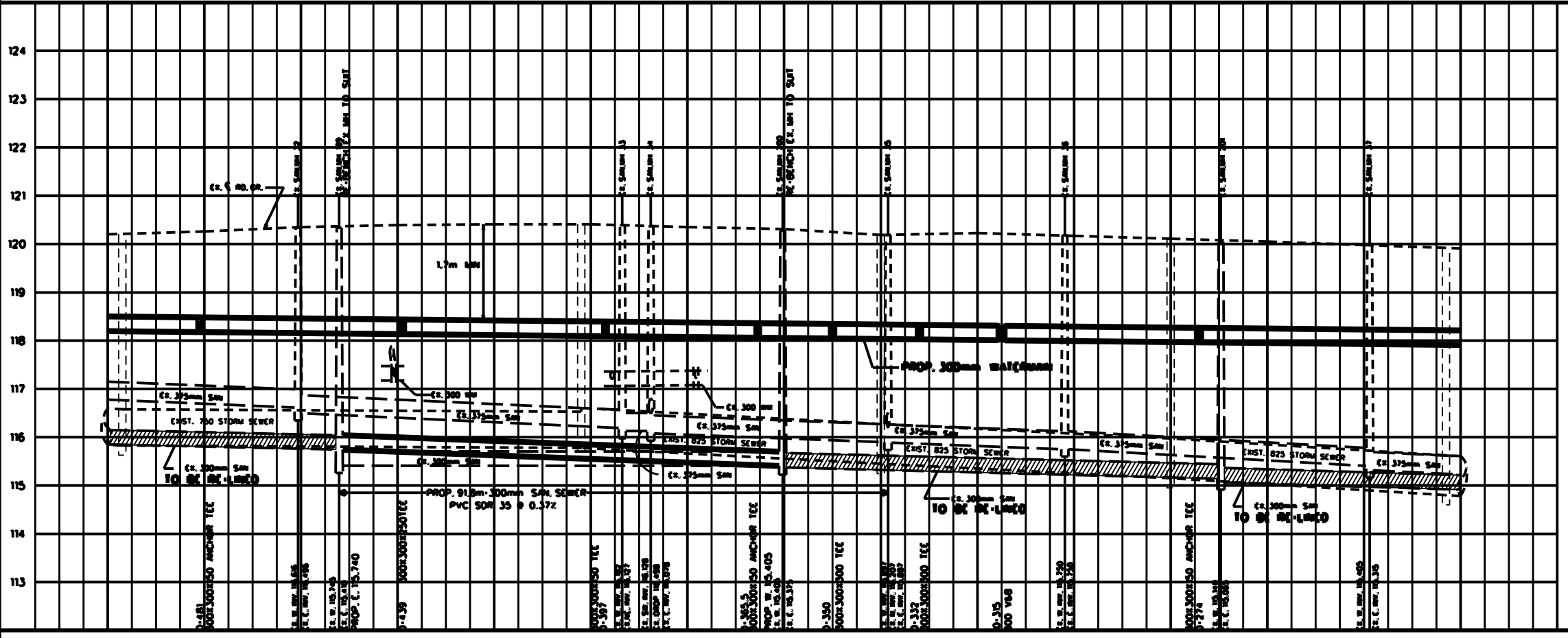
REFER TO DWG. No. 24706-D

DISCLAIMER

These records are based upon available and unverified information and may prove inaccurate. The Region of Peel disclaims any responsibility should these records be relied upon to the detriment of any person.

FOR GENERAL NOTES, LEGEND AND DETAILS SEE DWG. No. 24708-D

THIS DRAWING TO BE USED FOR WATERMAIN AND SAN. SEWER CONSTRUCTION ONLY



118.20	118.20	118.20	118.15	118.15	118.10	118.05	118.05	118.00	118.00	118.00	118.00	117.95	117.95	117.90	BOT. EL. OF WM.
120.200	120.260	120.350	120.390	120.410	120.410	120.350	120.310	120.200	120.230	120.170	120.110	120.050	119.960	119.910	C/L GRADE
0+500	0+480	0+460	0+440	0+420	0+400	0+380	0+360	0+340	0+320	0+300	0+280	0+260	0+240	0+220	CHAINAGE

General Notes

- ALL DRIVEWAYS ASPHALT UNLESS OTHERWISE NOTED.
- ALL SERVICE LOCATIONS ARE APPROXIMATE AND MUST BE LOCATED ACCURATELY IN THE FIELD.
- DENOTES BUILDING - NOT LOCATED
- DENOTES BUILDING LOCATED
- TYPE 'B' BEDDING UNLESS OTHERWISE NOTED (SAN)

B.M. NO. ELEV.

THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES PRIOR TO AND DURING CONSTRUCTION LOCATION OF EXISTING UTILITIES APPROXIMATE ONLY. TO BE VERIFIED IN FIELD BY CONTRACTOR.

DESIGNED BY: CHKD APPROVED BY: _____

NOTICE TO CONTRACTOR
48 HOURS PRIOR TO COMMENCING WORK NOTIFY THE FOLLOWING:

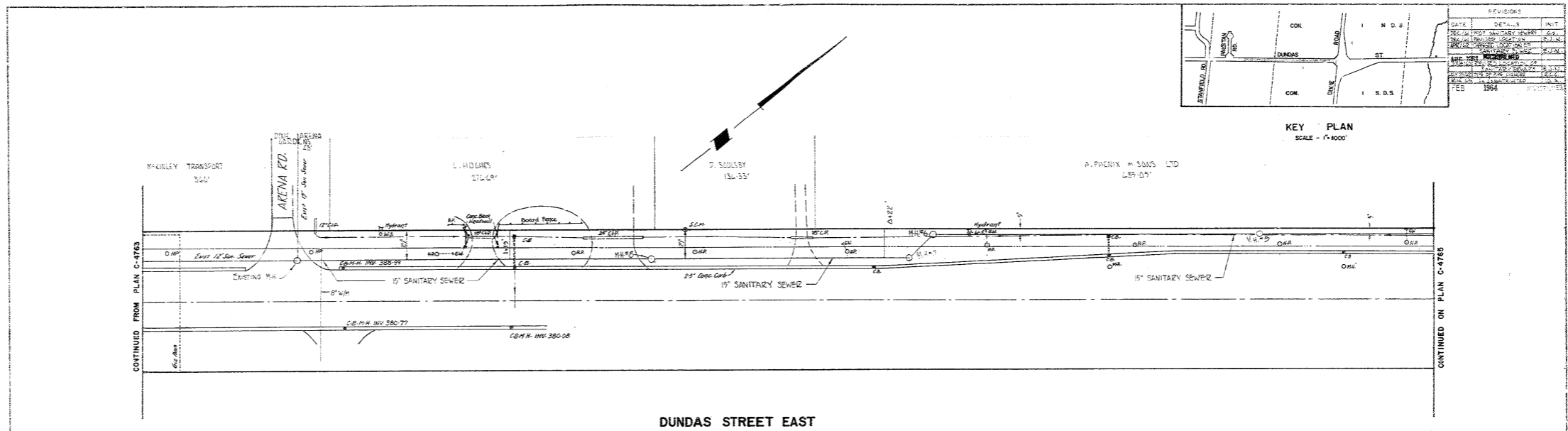
- THE REGIONAL MUNICIPALITY OF PEEL
- CITY OF MISSISSAUGA WORKS DEPT.
- CITY OF BRAMPTON WORKS DEPT.
- TOWN OF CALEDON WORKS DEPT.
- BELL TELEPHONE COMPANY
- CONSUMERS GAS COMPANY
- MINISTRY OF TRANSPORTATION
- MINISTRY OF ENVIRONMENT
- HYDRO ELECTRIC POWER COMM. OF ONTARIO
- HYDRO ELECTRIC COMM. CITY OF MISSISSAUGA
- HYDRO ELECTRIC COMM. CITY OF BRAMPTON
- HYDRO ELECTRIC COMM. TOWN OF CALEDON
- CABLE TELEVISION

Department of Public Works

**DUNDAS STREET EAST
DIXIE ROAD TO CAWTHRA ROAD
PROP. 300mm WATERMAIN
RE-LINING SAN. SEWER**

Sta. 0+220 To Sta. 0+500

LOTS	AREA 2-13.2-20	PROJECT NO. 24708-D
SCALE HOR. 1"=50'	DRAWN BY J.P.	CHECKED BY
DATE FEBRUARY, 99	SHEET 2 OF 8	PLAN NO. 24707-D



DUNDAS STREET EAST

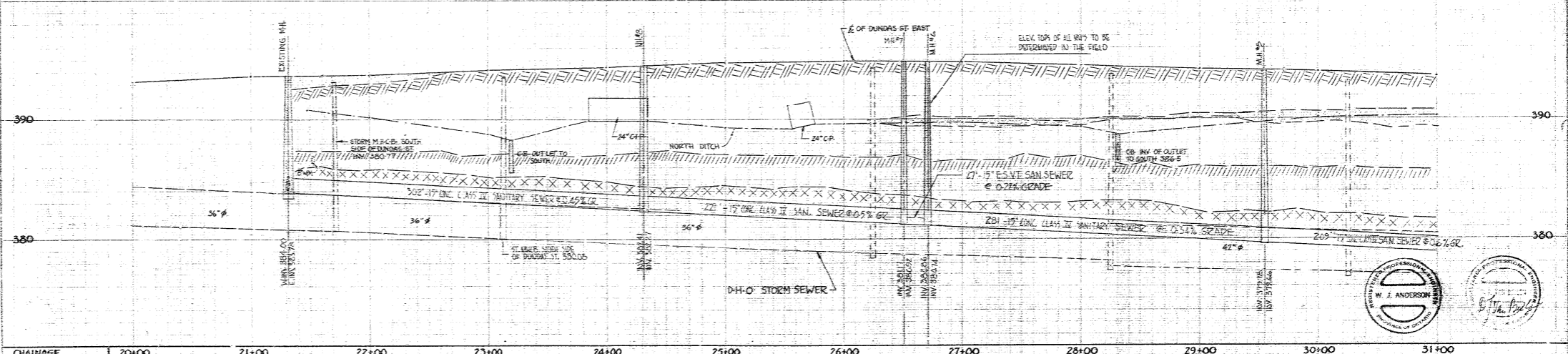
DISCLAIMER

These records are based upon available and unverified information and may prove inaccurate. The Region of Peel disclaims any responsibility should these records be relied upon to the detriment of any person.

- NOTE**
- ALL DRIVEWAYS GRAVEL UNLESS OTHERWISE NOTED
 - ⊙ DENOTES BLDG. - NOT LOCATED
 - ⊙ DENOTES BLDG. - LOCATED
 - ALL SERVICE LOCATIONS ARE APPROXIMATE AND MUST BE LOCATED ACCURATELY IN THE FIELD.
 - ALL PIPE TO HAVE 1/4" 10' BENDING
 - ELEV. OF TOPS OF M.H.'S TO BE DETERMINED IN THE FIELD.

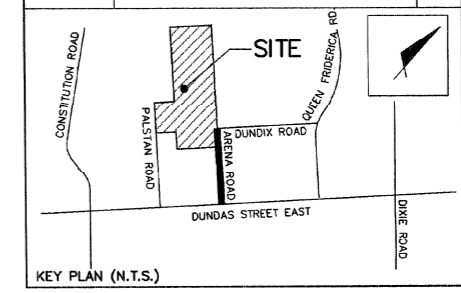
T.T.B.M. NO.	ELEV.
TEMP. E.M. ELEV.	DESC.

SCALE
Hor. - 1" = 40'
Ver. - 1" = 4'



TOWNSHIP OF TORONTO			
COUNTY OF PEEL			
ENGINEERING DEPARTMENT			
PLAN & PROFILE OF			
DUNDAS STREET EAST			
(NORTH SIDE ONLY)			
STN. 20+00 TO STN. 31+00			
LOTS 6, 7, 8, 8	CON. RANGE 1 N.D.S.	AREA 1 X 2	
DESIGN BY D.V.B.	DRAWN BY Z. MELHAM	PROJECT NO. 96-60	
DATE: OCT. 17, 1968	SHEET 1 OF 2	PLAN NO. C-4764	

REVISIONS		
DATE	DETAILS	INIT.
MAR. 20/1996	FIRST SUBMISSION REVISIONS	G.J.
APR. 15/1996	REVISED STORM SEWER	G.J.
APR. 22/1996	2nd SUBMISSION REVISION	G.J.
MAY 06/1996	INTERIM SUBMISSION REVISION	G.J.
MAR. 24/97	AS CONSTRUCTED STM.	J.K.



NOTES
UNLESS OTHERWISE NOTED ON DRAWINGS

1. ALL DIMENSIONS TO BE CHECKED AND VERIFIED ON THE SITE AND ANY DISCREPANCIES REPORTED TO THE ENGINEER.
2. ANY UTILITY RELOCATIONS REQUIRED DUE TO THE SEWER CONSTRUCTION TO BE UNDERTAKEN AT THE EXPENSE OF THE DEVELOPER.
3. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING AND PROTECTING ALL UTILITIES DURING CONSTRUCTION. GAS, HYDRO, TELEPHONE, OR ANY OTHER UTILITIES THAT MAY EXIST WITHIN THE STREETS AND MUST BE LOCATED BY ITS OWN UTILITIES AND VERIFIED PRIOR TO CONSTRUCTION.

- SEWERS**
1. CLASS "B" BEDDING IS TO BE USED AS PER CITY STANDARD 212.08. SEWER BEDDING AND COVER MATERIAL SHALL CONFORM WITH CITY STANDARDS 212.08 AND 212.11. IF WATER IS PRESENT IN THE TRENCH EXCAVATION, THEN 19MM CLEAR STONE IS TO BE USED FOR BEDDING IN ACCORDANCE WITH CITY STANDARD 212.11. IF THE SUB DRAINAGE CONSISTS OF MET AND GRODABLE SOILS, THEN THE BEDDING IS TO BE WRAPPED IN FILTER FABRIC FOR PROTECTION AGAINST THE INTRUSION OF FINES FROM THE SUBGRADE.
 2. TRENCH BACKFILL AS PER DETAIL ON THIS DRAWING.
 3. STANDARD RUBBER GASKET JOINTS TO BE USED THROUGHOUT STORM SEWER SYSTEM.
 4. TRENCH WIDTH AT TOP OF PIPE AS PER CITY OF MISSISSAUGA STD. 212.08.
 5. THE CONTRACTOR IS TO BE RESPONSIBLE FOR SUPPLYING EXTRA BEDDING AND/OR STRONGER PIPE IF ACTUAL TRENCH WIDTH EXCEEDS THE DESIGN WIDTH.
 6. CONCRETE PIPES SMALLER THAN 450mm SHALL BE CL. ES.
 7. CONCRETE PIPES LARGER THAN 450mm SHALL BE CL. ES-D, UNLESS OTHERWISE NOTED.
 8. ALL SEWER AND WATERMAIN CROSSINGS SHALL BE SUPPORTED BY CONCRETE ENCASUREMENT OR GRANULAR MATERIAL AS APPROVED BY THE CITY ENGINEER.

- MANHOLES**
1. STORM MANHOLES AS PER OPSD 701.01 UNLESS OTHERWISE NOTED ON DRAWINGS.
 2. ALUMINUM MANHOLE STEPS AS PER OPSD 405.01.
 3. MANHOLE FRAME AND COVER AS PER OPSD 401.04D.

- WATERMAIN**
1. ALL VALVES IN THE AREA OF CONSTRUCTION SHALL BE LOCATED WITH THE ASSISTANCE OF REGION OF PEEL AND RAISED, IF NECESSARY PRIOR TO ASPHALT WORKS BEING COMPLETED.
 2. ONLY REGION OF PEEL SHALL OPERATE EXISTING HYDRANTS AND VALVES.
 3. ALL SEWERS SHALL BE LOCATED PRIOR TO CONSTRUCTION AND PROTECTED DURING ALL CONSTRUCTION ACTIVITIES SHOULD DAMAGE OCCUR. REGION OF PEEL SHALL UNDERTAKE ANY REQUIRED REPAIRS AT THE EXPENSE OF THE CONTRACTOR.

- ROADS**
1. ALL TRENCHES WITHIN ROAD ALLOWANCE SHALL BE BACKFILLED AS PER DETAIL ON THIS DWG.
 2. THE STABILITY AND COMPACTION OF ALL BACKFILL MATERIALS ARE TO BE CERTIFIED BY A RECOGNIZED SOIL CONSULTANT TO THE CITY ENGINEER PRIOR TO THE INSTALLATION OF ANY ROAD BASE MATERIALS.
 3. CURBS TO BE REINFORCED TO MATCH EXISTING CURB DIMENSIONS WITH 300mm CONC. AT 28 DAYS.
 4. ALL DRIVEWAYS AFFECTED BY SERVICE RECONNECTION WORKS ARE TO BE REPLACED IN TOTAL BETWEEN THE CURB AND SUBPAVEMENT OR STREETLINE, IF NO SUBPAVEMENT. 450mm OF 14.3 50mm H.B. 150mm OF ORAL "A" AND 300mm OF ORAL "B" COMPACTED TO MEET STANDARD PROCTOR DENSITY.

SUBMISSIONS: 1st _____ 2nd _____ Preservicing _____
DATE: _____ Interim _____ Final _____

GENERAL NOTES

ALL DRIVEWAYS ASPHALT UNLESS OTHERWISE NOTED.
ALL SERVICE LOCATIONS ARE APPROXIMATE AND MUST BE LOCATED IN THE FIELD.

● DENOTES BUILDING - NOT LOCATED
□ DENOTES BUILDING - LOCATED

B.M. No. 361 ELEV. 129.069m (1978 RE-ADJUSTMENT)
DESCRIPTION: ON THE W. FACE AT THE S. CORNER OF GARAGE OF A RED BRICK BUNGALOW, NO. 3147 ON THE E. SIDE OF THE CONSTITUTION BLVD., OPPOSITE HOMERIC DR.

DESIGNED BY: CHKD APPROVED BY: _____

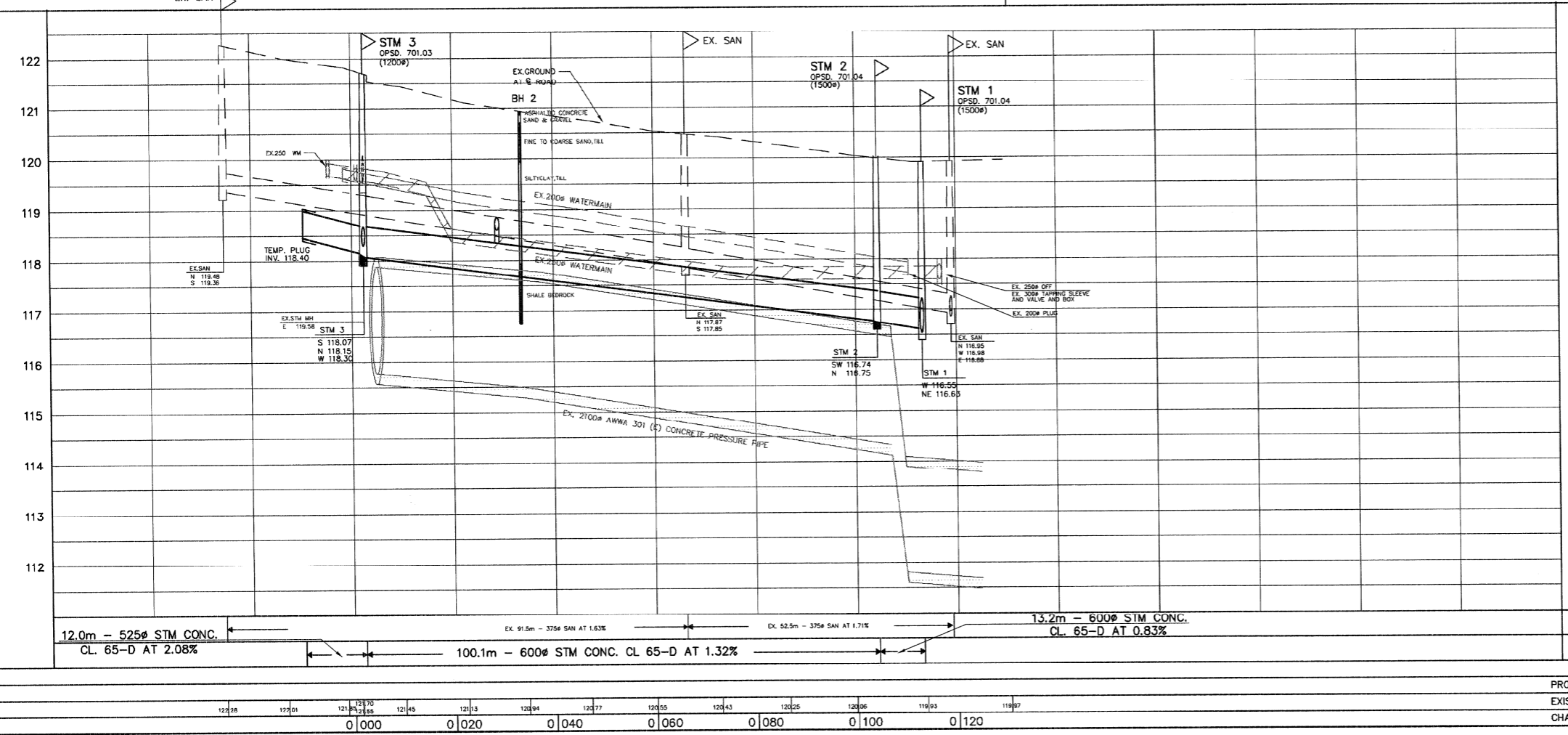
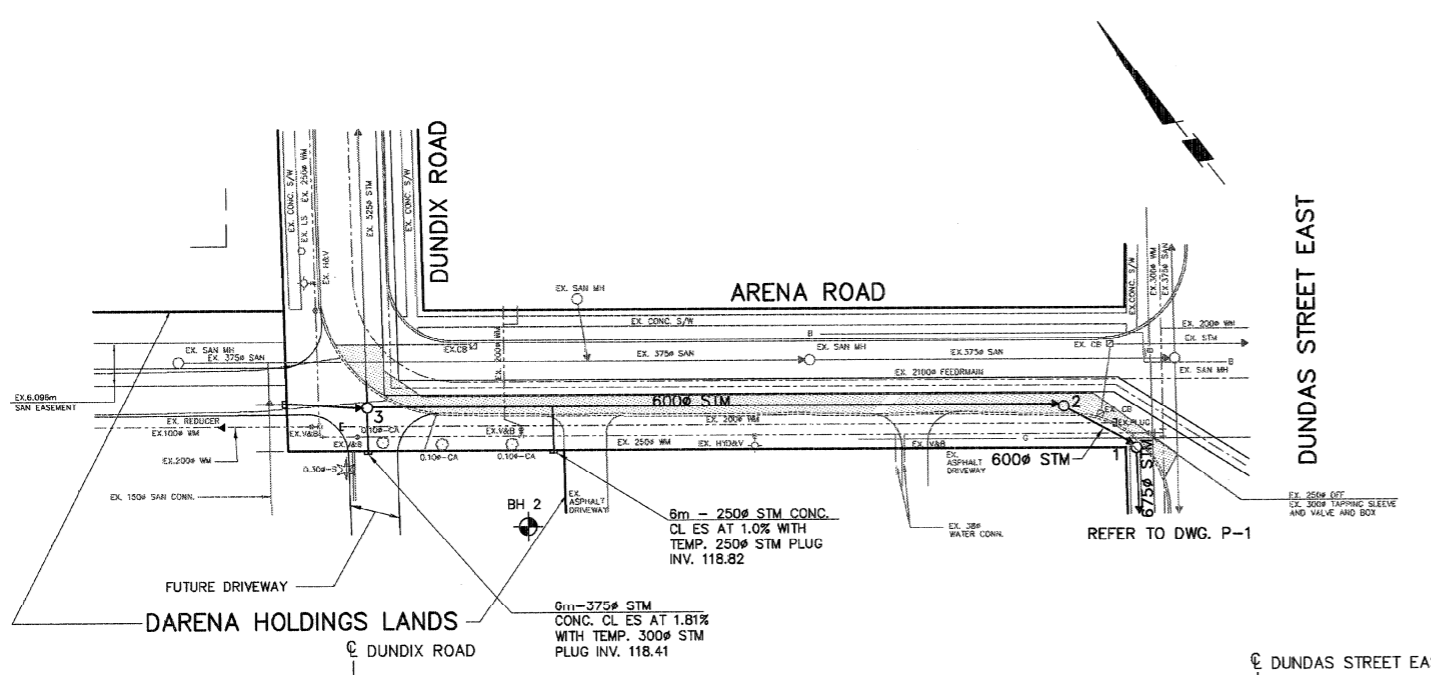
Kleinfeldt Consultants Limited
2400 MEADOWPINE BLVD. SUITE 100 MISSISSAUGA, ONTARIO L5M 6S2 (905)542-1600

DARENA HOLDINGS LIMITED
PART OF LOT 7, CON. 1 N.D.S. AND LOT 10 AND 11 R.P. 455
OZ-56/95

MISSISSAUGA Transportation and Works

PROPOSED STORM SEWER ARENA ROAD
STN. 0+000 TO STN. 0+120

PROJ. & ROAD SCALE HOR: 1:500 VERT: 1:50 AREA Z-20 PROJECT No. 1789
EXIST. GROUND DRAWN BY G.J. CHECKED BY A.J. PLAN No. P-2
CHAINAGE DATE JAN./1996 SHEET 2 OF 2



PART 1, PLAN 43R-2899
PARCEL 7-2, SECTION 43-TOR.TWP.-1(NDS)

OWNERS:
STEPHEN-MITCHELL REALTY (5%)
TOBBLE INVESTMENTS (15%)
LYNROB INVESTMENTS (15%)
RICHCO INVESTMENTS (15%)
WHITEHORN INVESTMENTS (50%)

LYNROB INVESTMENTS
ALCAZAR DEVELOPMENTS
WHITEHORN INVESTMENTS
INST. No. 272618VS
(MAJESTIC ELECTRONICS)

SEE DWG. No. 2

P.I. No. 77
STA. 0+092.43

P.I. No. 75
STA. 0+015.00

ARENA ROAD

MATCH LINE

STATION 0+100

2100Ø FEEDERMAIN

PROPOSED 250Ø WATERMAIN

INSTALL HYDRANT AND VALVE TO REMAIN OPEN AT ALL TIMES. RENSTATE DRIVEWAY IMMEDIATELY FROM STREETLINE AND VALVE TO BE AS PER DETAIL ON DWG. 16

100mm Ø SAN SERVICE CONN.

NEW 30Ø SERVICE CONNECTION TO PEEL STD. 1-7-1 WITH 30Ø V&B ON PROPERTY LINE. RECONNECT 1.0m± INSIDE PROPERTY. CONNECTION MATERIAL TO MATCH EXISTING.

INST. NO. 172867VS
ROCKAWAY CONST. LTD.
COCHILL CONST. LTD.

P.I. No. 76
STA. 0+017.84

DUNDAS STREET EAST

CONTRACT 5

CONTRACT 4

P.I. No. 74
STA. 0-036.18

P.I. No. 73
STA. 0-040.20

P.I. No. 72
STA. 0-055.00

EX. BELL CANADA FIBRE OPTIC CABLE IN PVC DUCT TO BE SUPPORTED.

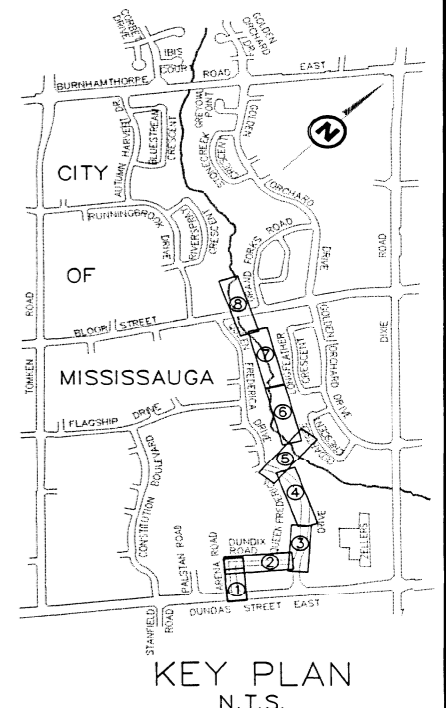
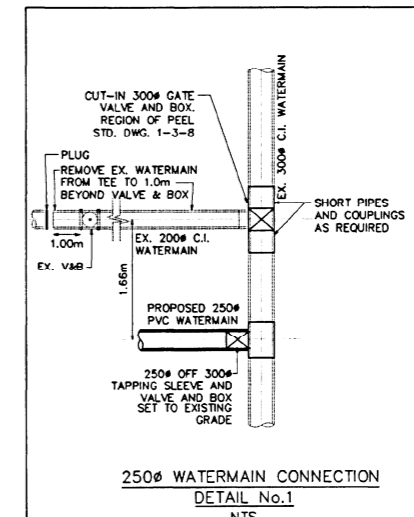
EX. 2100Ø FEEDERMAIN

PERMANENT EASEMENT

48.0m± TUNNEL

DISCLAIMER

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UTILITY EXCAVATION RECORD

EXCAVATIONS CARRIED OUT IN JULY 1993 BY WARDEN CONSTRUCTION CO. LTD.

POINT No.	UTILITY	TOP ELEVATION	COMMENTS
10	WATERMAIN	117.93	0.30m DIA. CAST IRON-LEAD JOINT
11	BELL	118.95	0.10m DIA. DUCT
12	GAS	117.65	0.20m DIA.

NOTE: POINT NUMBER ON PLAN INDICATES LOCATION OF EXCAVATION.

FEEDERMAIN P.I. LAYOUT DATA

P.I. No.	CHAINAGE	NORTHERN	EASTERN	INVERT ELEV.(m)
72	0-055.00	4828741.588	613909.570	113.25
73	0-040.20	4828753.193	613918.725	113.13
74	0-036.18	4828756.363	613921.197	111.10
75	0+015.00	4828805.788	613907.910	111.75
76	0+017.84	4828807.575	613905.706	114.30
77	0+092.43	4828854.550	613847.762	115.45
78	0+119.17	4828871.540	613826.835	115.70

- ALL DIMENSIONS AND ELEVATIONS ARE IN METRES UNLESS OTHERWISE NOTED.
- PIPE SIZES ARE IN MILLIMETRES.
- FOR BENCH MARKS, LIST OF DRAWINGS AND LEGEND, SEE INDEX SHEET

NO.	REVISIONS TO DRAWING	BY	DATE	APPR.
	AS CONSTRUCTED WATERMAIN ONLY		G.P. 01-03-97	
	ISSUED FOR TENDER		J.S. 20-06-95	A.S.
	ISSUED FOR APPROVAL: NOT FOR CONSTRUCTION		J.S. 10-04-95	A.S.

Department of Public Works
Region of Peel

REGION PROJECT No. 93-1690
ARENA ROAD WATERMAIN REPLACEMENT
REGION DRAWING No.

Ontario Clean Water Agency / Agence Ontarienne Des Eaux
SOUTH PEEL WATER SYSTEM

O.C.W.A. PROJECT No. 5-0020-53

HANLAN FEEDERMAIN
CONTRACT 5

ARENA ROAD
STA. 0-055.00 TO STA. 0+100

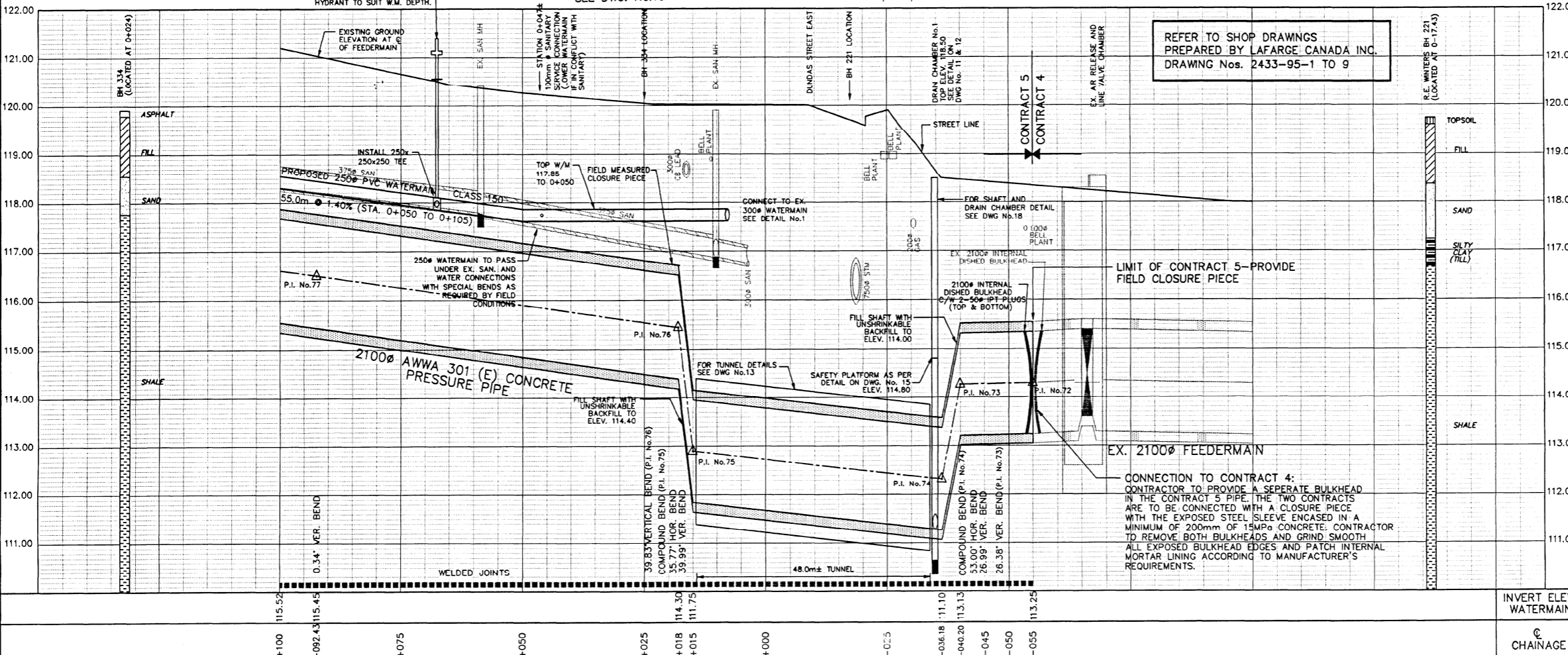
Marshall Macklin Monaghan Limited
Consulting Engineers - Surveyors - Planners

ORIGINAL STAMPED BY ALEX SLYWYNSKYJ
DATED: JUNE 19, 1995

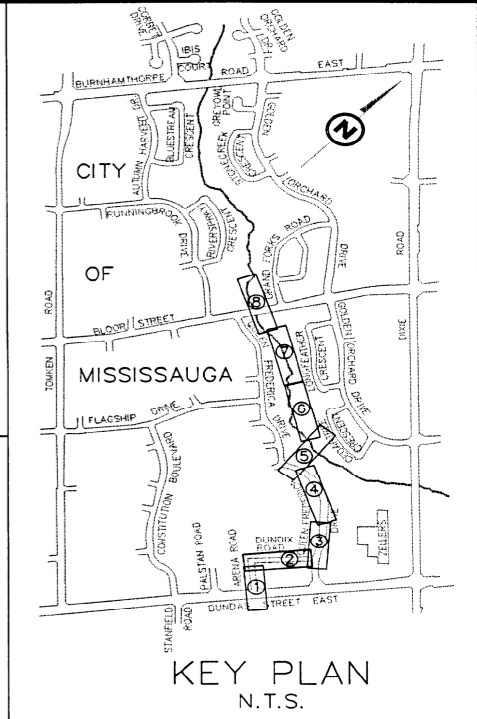
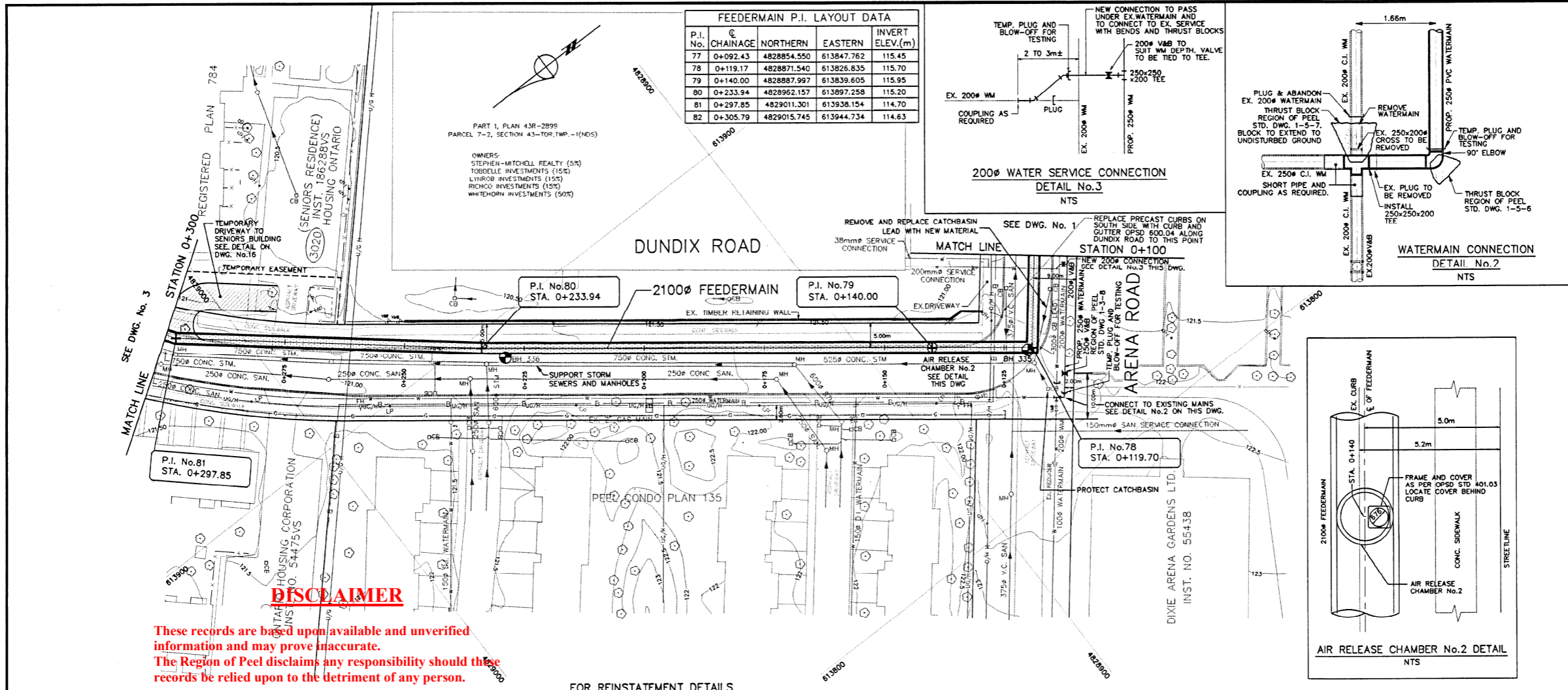
DESIGNED: J.S. DRAWN: WDG CHECKED: A.S.
DATE: APRIL 1995

SCALE: HORIZ. SCALE: 1:500 VERT. SCALE: 1:50

CONSULTANT PROJECT No. 10-90046 DRAWING No. 1



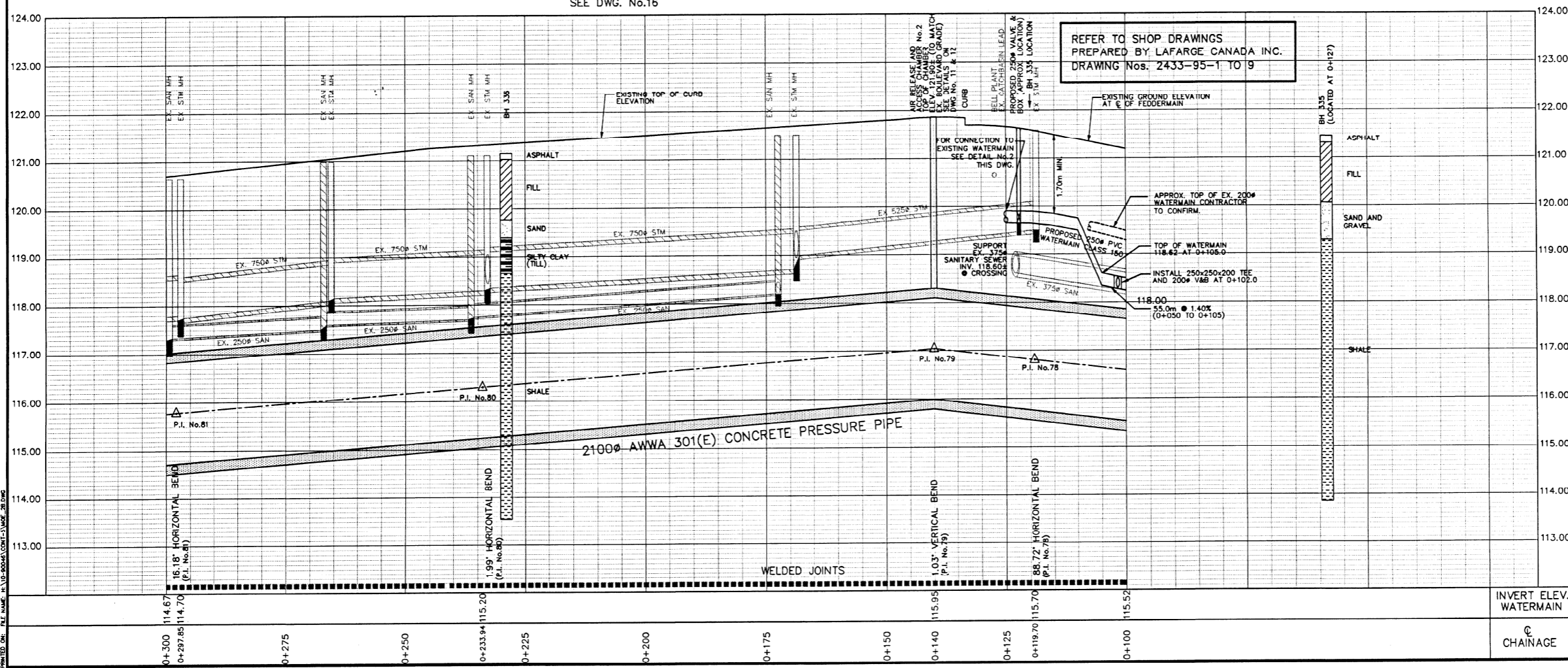
PRINTED ON: PLY NAME: IN 100-90046-CONTRACT-5-A-18-DWG



- KEY PLAN**
N.T.S.
- ALL DIMENSIONS AND ELEVATIONS ARE IN METRES UNLESS OTHERWISE NOTED.
 - PIPE SIZES ARE IN MILLIMETRES.
 - FOR BENCH MARKS, LIST OF DRAWINGS AND LEGEND, SEE INDEX SHEET

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Department of Public Works
Region of Peel

REGION PROJECT No. 93-1690
ARENA ROAD WATERMAIN REPLACEMENT
REGION DRAWING No.

South Peel Water System
O.C.W.A. PROJECT No. 5-0020-53

HANLAN FEEDERMAIN CONTRACT 5

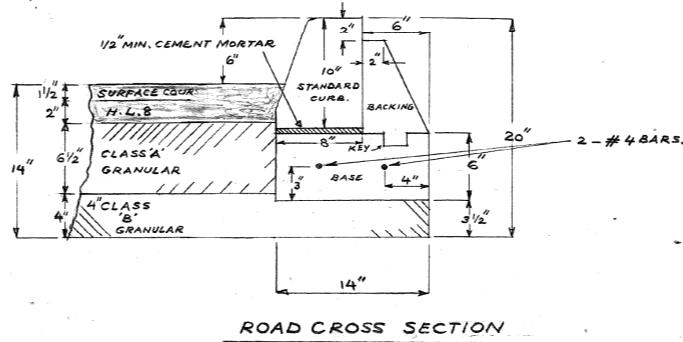
DUNDIX ROAD
STA. 0+100 TO STA. 0+300

Marshall Macklin Monaghan Limited
Consulting Engineers - Surveyors - Planners

ORIGINAL STAMPED BY
ALEX SLYWYNSKYJ

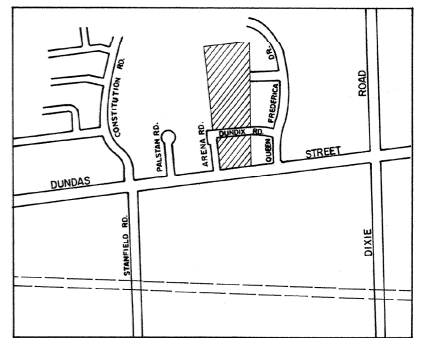
DATED: JUNE 19, 1995

DESIGNED	J.S.	DRAWN	WDG	CHECKED	A.S.
DATE: APRIL 1995					
SCALE	HORIZ. SCALE: 1:500 VERT. SCALE: 1:50				
CONSULTANT PROJECT No.	10-90046		DRAWING No.	2	

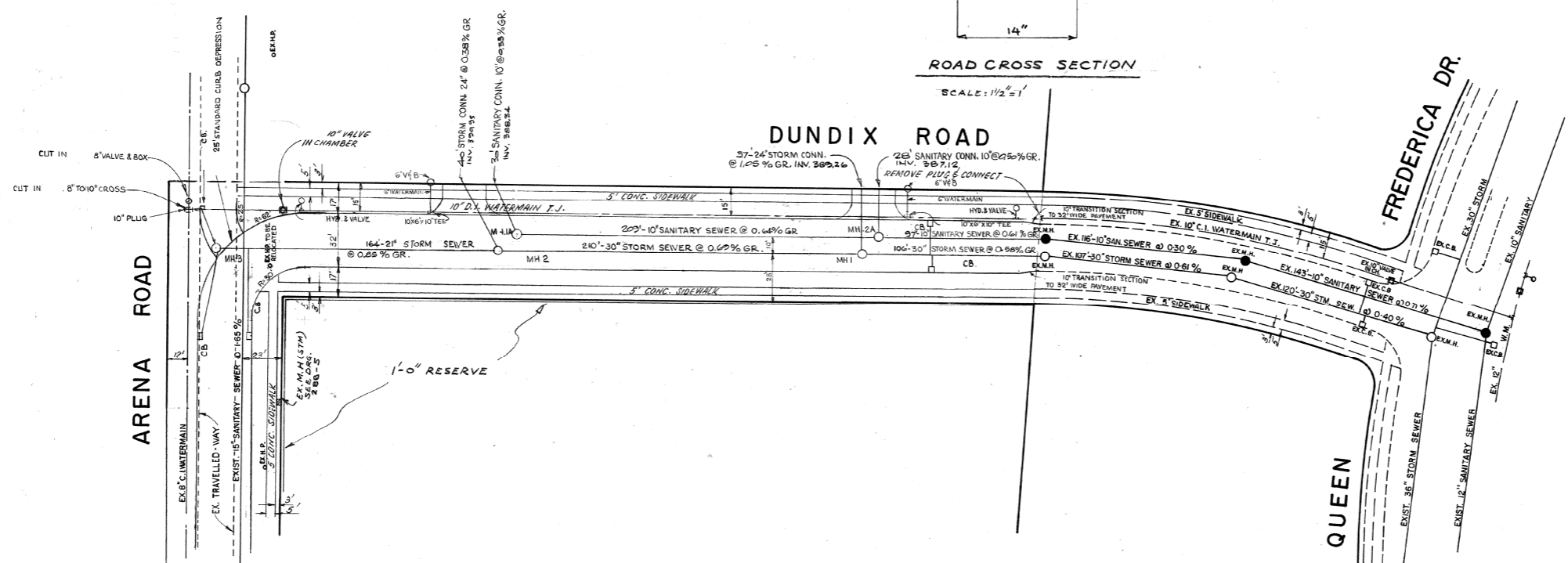


ROAD CROSS SECTION

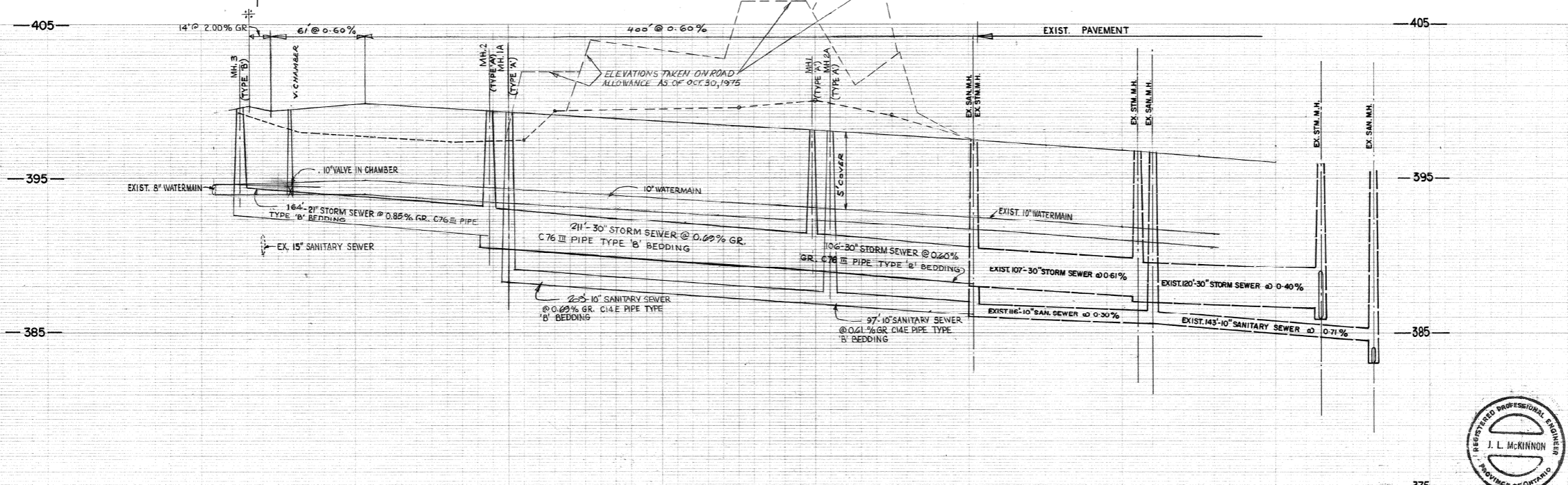
SCALE: 1/2" = 1'



KEY PLAN
SCALE: 1" = 100'



- NOTES:**
- 1) BENCHMARK No. 86 ON THE EAST FACE AT THE NORTH CORNER OF THE ROMAN CATHOLIC CHURCH AT SOUTH WEST CORNER OF DUNDAS STREET AND DIXIE ROAD. ELEVATION 391.85
 - 2) MANHOLES AS TOWN OF MISSISSAUGA STANDARD TYPE AS NOTED.
 - 3) C.B. LEADS 10" DIA. CONCRETE PIPE UNLESS NOTED OTHERWISE.
 - 4) SANITARY SEWERS TO HAVE APPROVED MECHANICAL JOINTS AND PREMIUM RUBBER GASKETS.
 - 5) SPRINGLINES OF CONNECTIONS INTO MANHOLES ARE TO MATCH THE SPRINGLINE OF THE MAIN SEWER.
 - 6) THE CONTRACTOR SHALL SUPPLY AT HIS EXPENSE ADDITIONAL BEDDING AND/OR STRONGER PIPE WHERE THE ACTUAL TRENCH WIDTH AT THE TOP OF PIPE EXCEEDS THE SPECIFIED WIDTH.
 - 7) MAX TRENCH WIDTH AT TOP OF PIPE
10" - 12" PIPE - 5'-0" TRENCH
15" - 30" PIPE - OUTSIDE DIA. + 2'-0"
 - 8) ALL WATERMAIN TO BE DUCTILE IRON ANSI CLASS II CEMENT LINED WITH TYTON JOINTS.
 - 9) THE MINIMUM LATERAL DISTANCE BETWEEN WATER FACILITIES AND OTHER UTILITIES SHALL BE 4 FEET



FIELD BOOK INFORMATION			
REVISIONS			
NO.	DATE	DETAILS	AUTH.
1.	OCT. 19, 1973	AMENDED AFTER RECEIPT OF TOWN COMMENTS	H.A.G.
2.	NOV. 23, 1973	AMENDED AFTER RECEIPT OF P.U.C. COMMENTS & SOIL REPORT.	S.S.S.
3.	NOV. 23, 1973	AMENDED AFTER RECEIPT OF TOWN COMMENTS	S.S.S.
4.	DEC. 16, 1973	AMENDED TO SHOW EX. STN. M.H. ON ARENA RD.	S.S.S.
5.	NOV. 19, 1975 DEC. 1978	SANITARY AND STORM SEWER REVISED AS CONST. SEWER.	H.A.G. J.E.

TOWN OF MISSISSAUGA
MISSISSAUGA COMMERCIAL PROPERTIES
PLAN & PROFILE OF:

DUNDIX ROAD

FROM: EAST LIMIT OF SUBDV. TO: ARENA ROAD

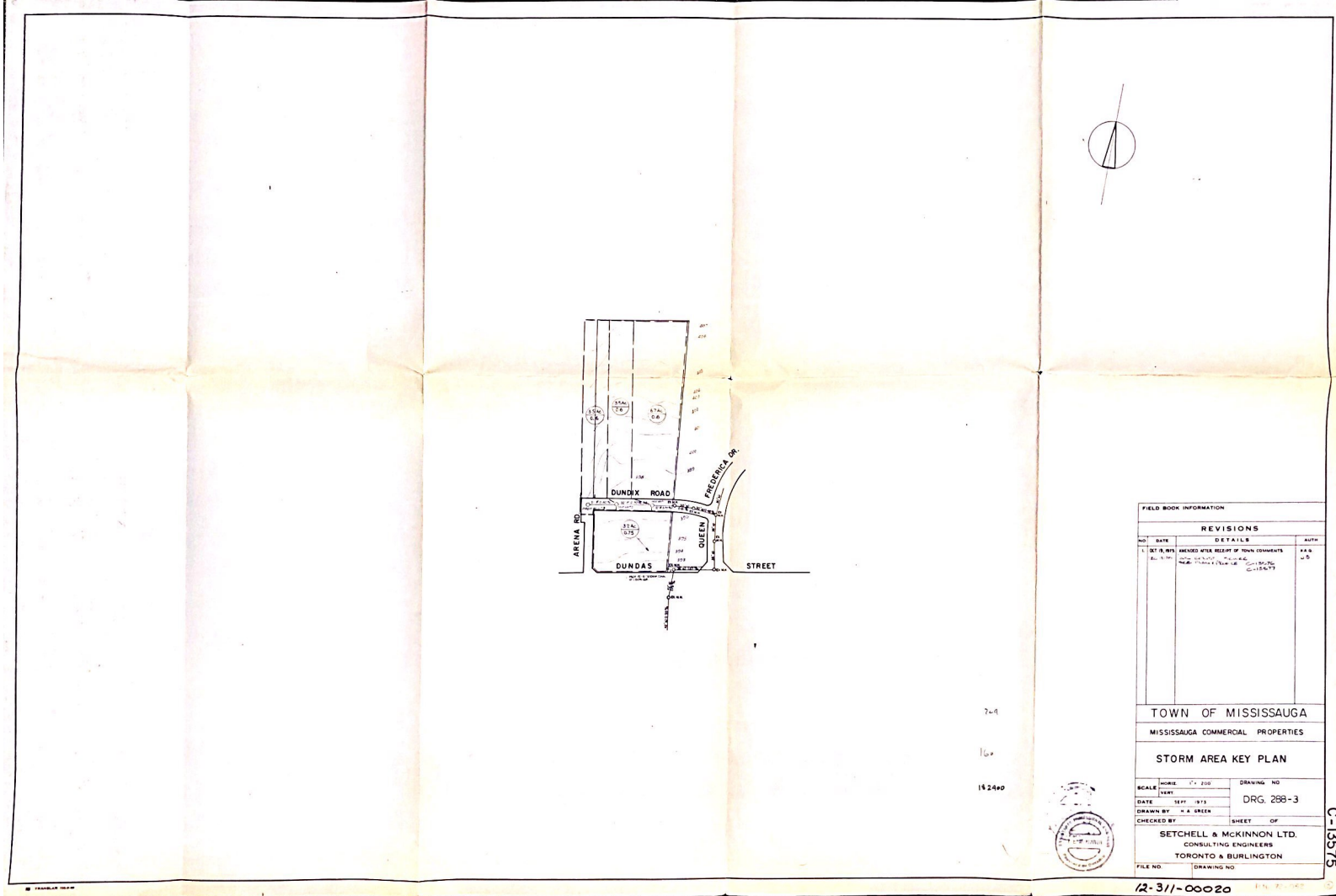
SCALE: HORIZ. 1" = 40'
VERT. 1" = 4'

DATE: SEPT. 1973
DRAWN BY: H. A. GREEN

CHECKED BY: SHEET OF
SETCHELL & MCKINNON LTD.
CONSULTING ENGINEERS
TORONTO & BURLINGTON

FILE NO. DRAWING NO. **C-13576**





FIELD BOOK INFORMATION			
REVISIONS			
NO.	DATE	DETAILS	AUTH.
1	SEP 19, 1973	REVISED AFTER RECEIPT OF YOUR COMMENTS	A.S.

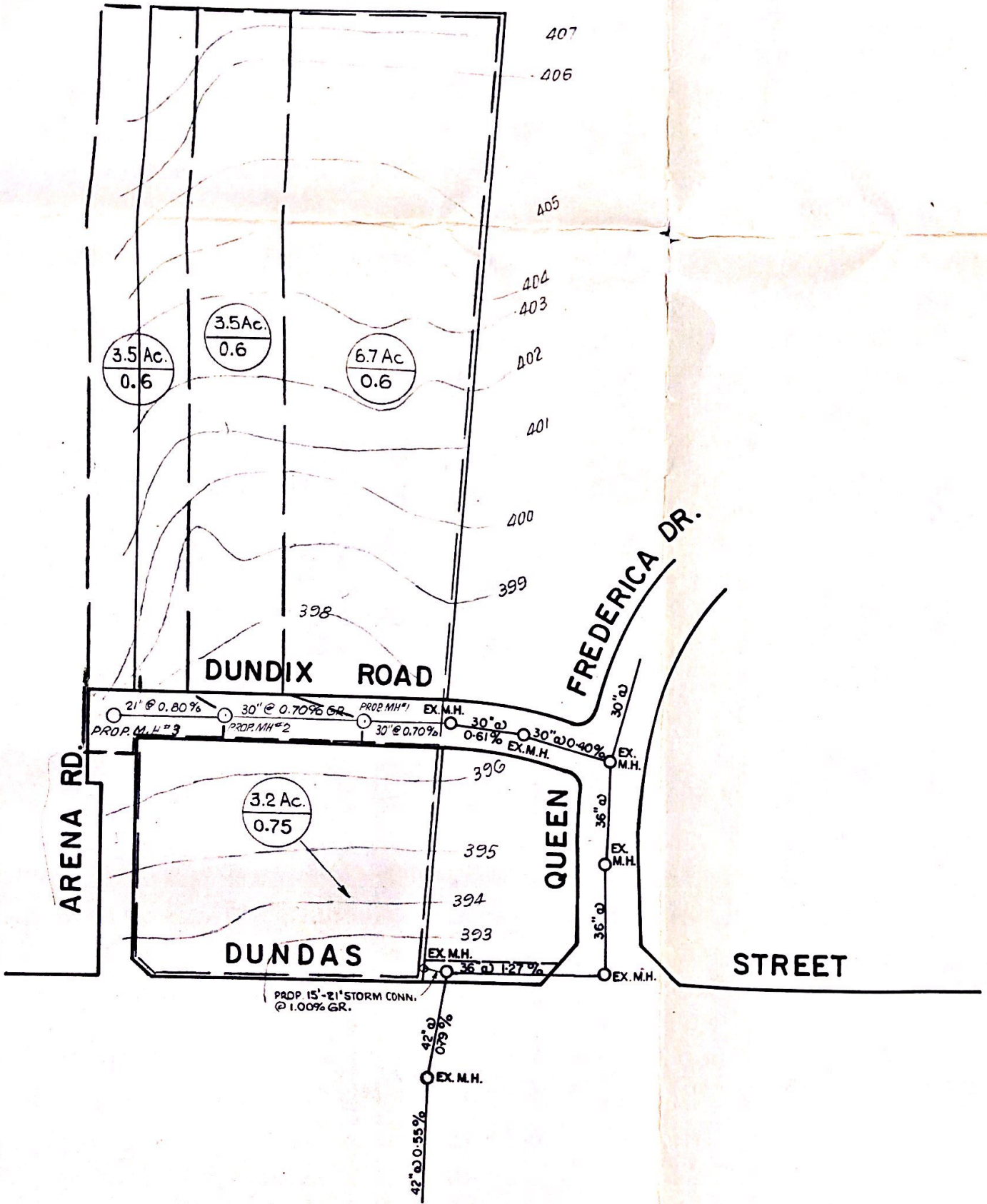
TOWN OF MISSISSAUGA
MISSISSAUGA COMMERCIAL PROPERTIES

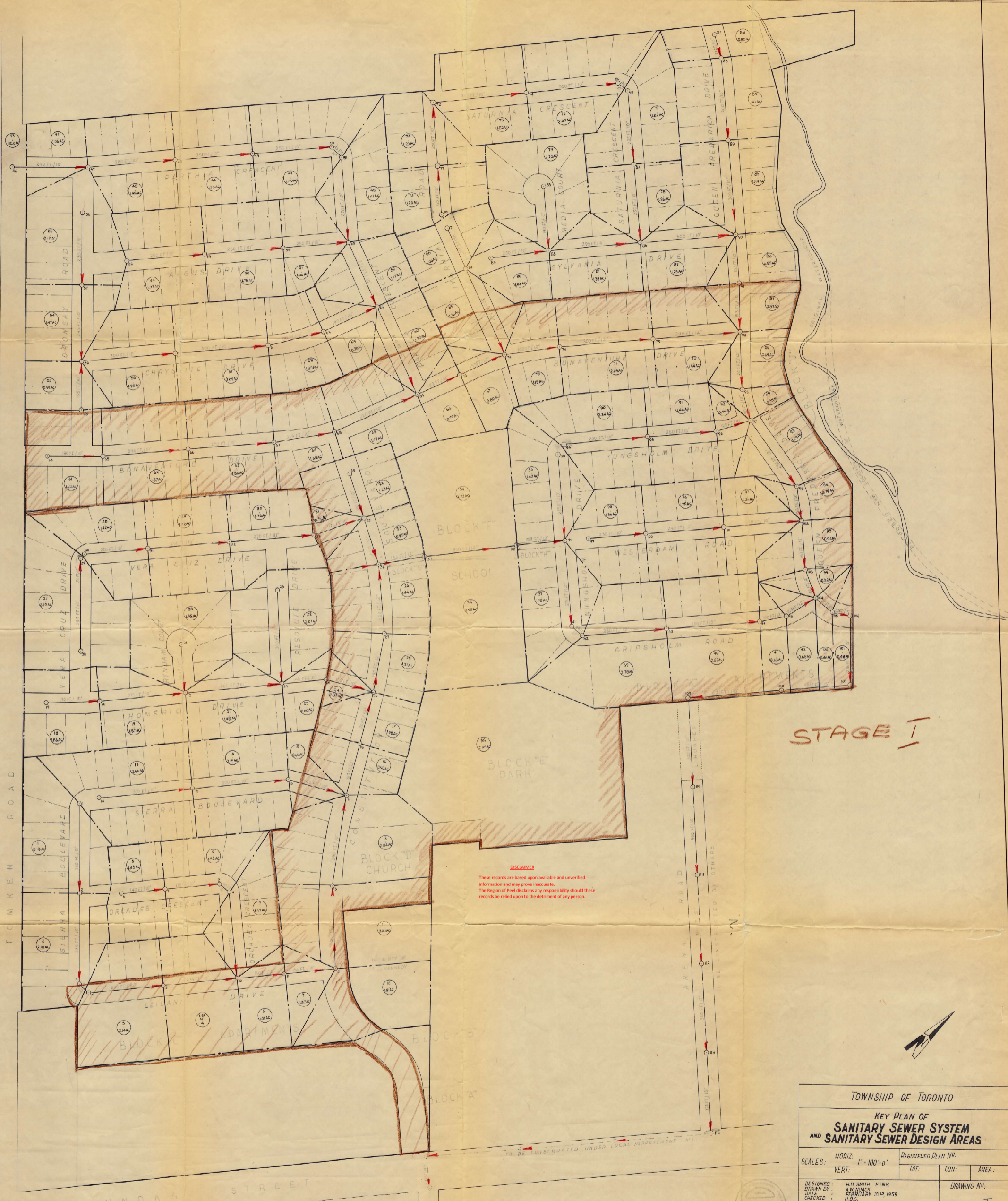
STORM AREA KEY PLAN

SCALE: HORIZONTAL 1" = 200'	DRAWING NO. DRG. 288-3
DATE: SEPT. 1973	DRAWN BY: M. A. GREEN
CHECKED BY:	SHEET OF
SETCHELL & MCKINNON LTD. CONSULTING ENGINEERS TORONTO & BURLINGTON	
FILE NO.	DRAWING NO.

12-311-00020

C 13575





STAGE I

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TOWNSHIP OF TORONTO

KEY PLAN OF
**SANITARY SEWER SYSTEM
 AND SANITARY SEWER DESIGN AREAS**

SCALES:	HORIZ: 1" = 100'-0"	REGISTERED PLAN NO.:	LOT:	CON:	AREA:
	VERT: 1" = 100'-0"				

DESIGNED BY: W.D. SMITH P.E.
 DRAWN BY: A.W. WOODCH
 DATE: FEBRUARY 18th 1959
 CHECKED: H.D.S.

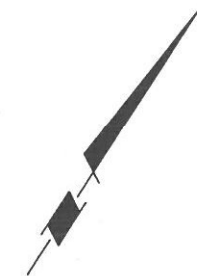
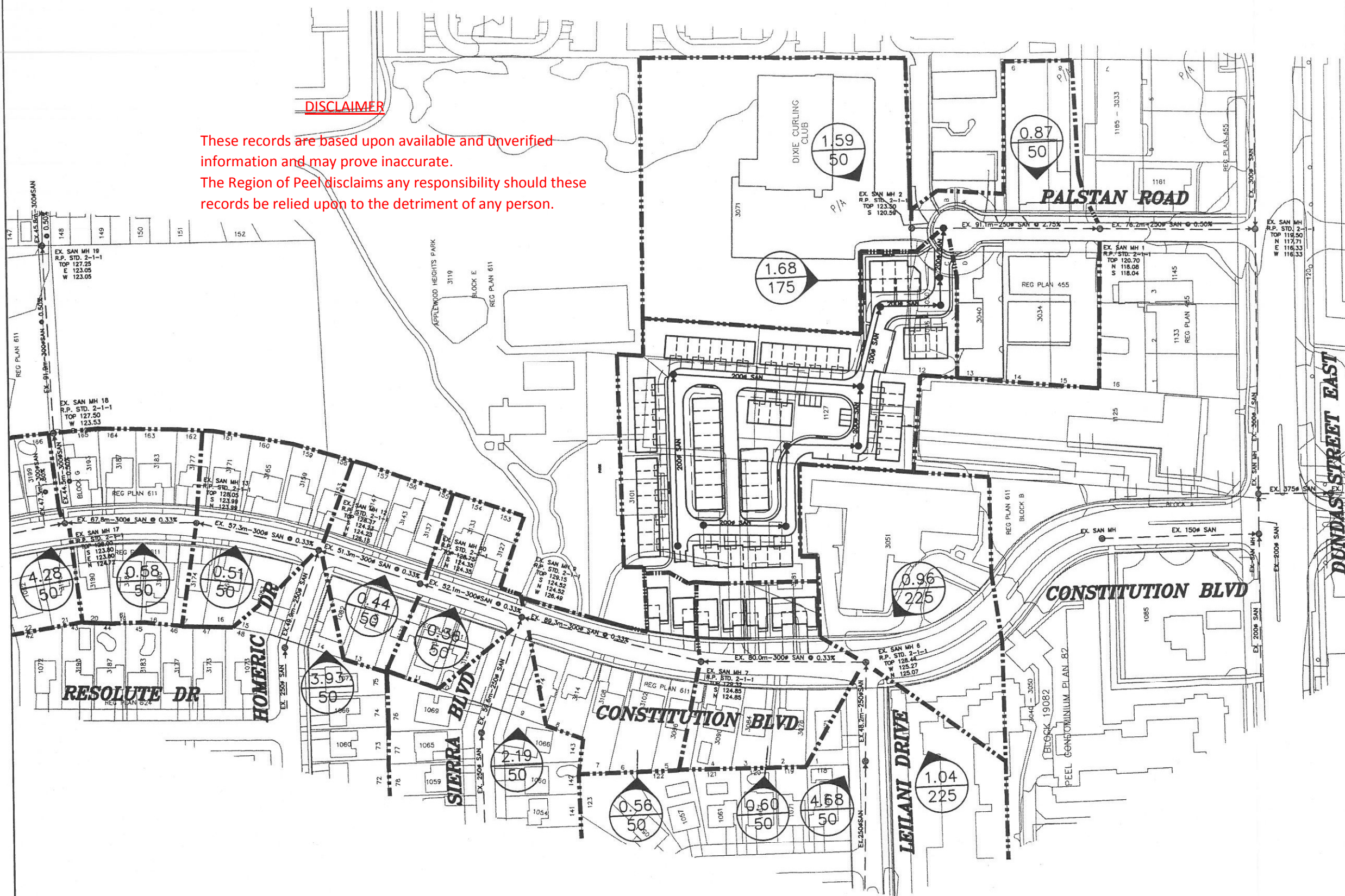
PROJECT: 59-1 SHEET NO. P-5

APPLEWOOD DEVELOPMENT LTD.

DRAWING NO.: STAGE I

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- LEGEND**
- - SANITARY MANHOLE
 - - - - - EXISTING SEWER
 - — — — — PROPOSED SANITARY SEWER
 - - - - - DRAINAGE AREA BOUNDARY
 - 1.86 / 175 - DRAINAGE AREA (HECTARES) / POPULATION (PER HECTARES)

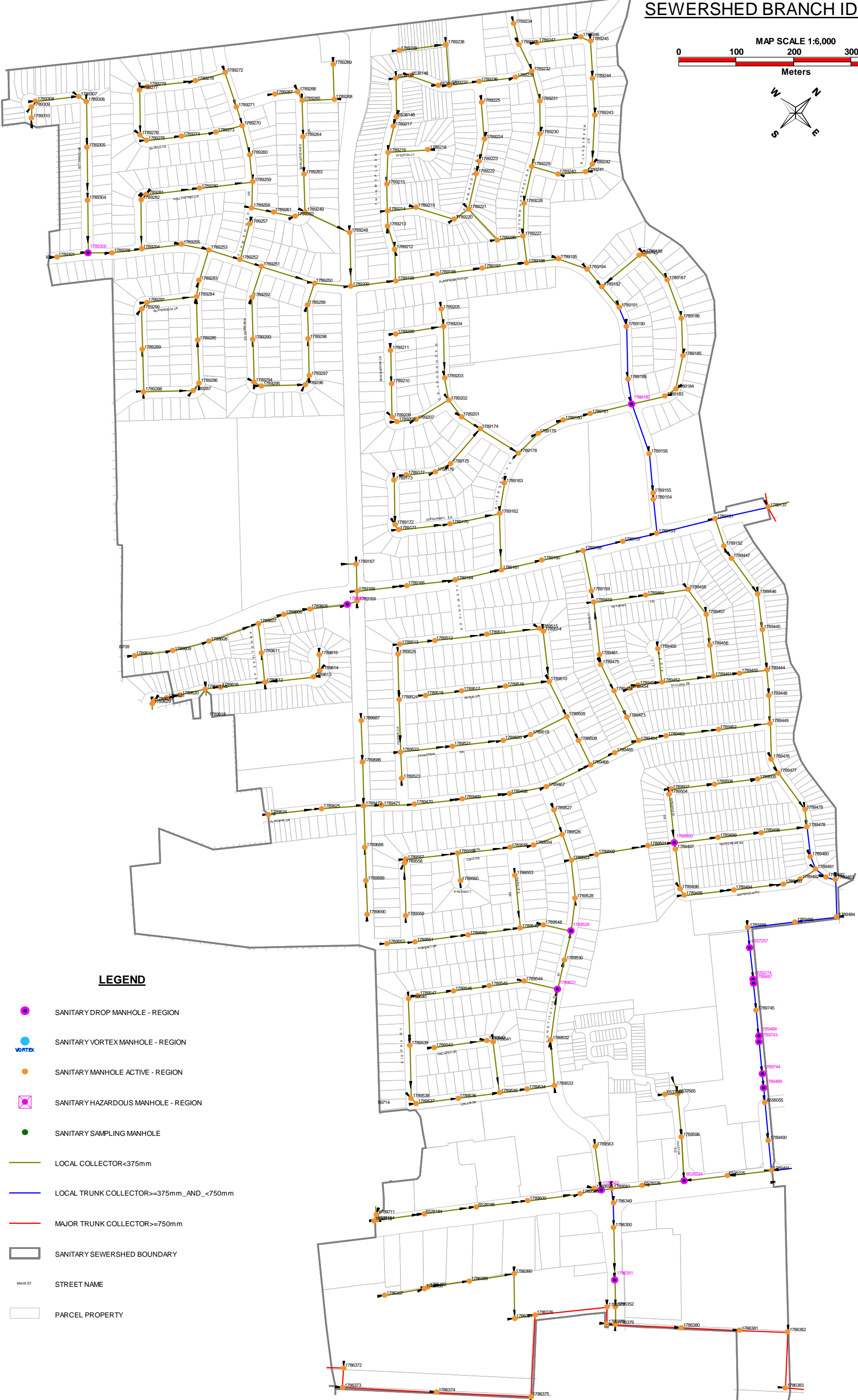
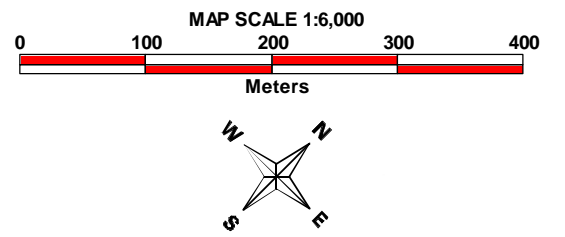
STONE MANOR DEVELOPMENTS (CONSTITUTION) INC.

REV. - FEBRUARY 2005

SKIRA & ASSOCIATES LTD.
 CONSULTING ENGINEERS
 3464 Semenyk Court, Suite 100, Mississauga, Ontario L5C 4P8
 Tel. (905) 276-5100 Fax. (905) 270-1936 Email - info@skiraconsult.ca

SANITARY DRAINAGE PLAN

PROJECT No. 204-M04	FIGURE 4
DATE - DECEMBER 2004	
SCALE - 1 : 2000	DRAWN BY - M.B.



LEGEND

- SANITARY DROP MANHOLE - REGION
- SANITARY VORTEX MANHOLE - REGION
- SANITARY MANHOLE ACTIVE - REGION
- SANITARY HAZARDOUS MANHOLE - REGION
- SANITARY SAMPLING MANHOLE
- LOCAL COLLECTOR < 375mm
- LOCAL TRUNK COLLECTOR ≥ 375mm AND < 750mm
- MAJOR TRUNK COLLECTOR ≥ 750mm
- SANITARY SEWERSHED BOUNDARY
- STREET NAME
- PARCEL PROPERTY